PROSPECT (GB)

Church Raike, Chipping

Geo-Environmental Assessment Report

March 2013

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PROJECT QUALITY CONTROL DATA SHEET

Site Name:	Church Raike, Chipping
Report Title:	Geo-Environmental Assessment Report
Report Number:	AJH/C2179/3577

Client: Prospect GB	
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CONTENTS

1.0	INTRODUCTION
2.0	METHOD OF INVESTIGATION
2.1 2.2 2.3 2.4 2.5	OBJECTIVES 2 SITE WORKS 2 SAMPLING 2 LABORATORY TESTING 3 MONITORING 3
3.0	GROUND CONDITIONS
3.1 3.2 3.3 3.4 3.5	MADE GROUND 4 NATURAL GROUND 4 BEDROCK 4 GROUNDWATER 4 OBSERVATIONS 4
4.0	TEST RESULTS
4.1 4.2	CHEMICAL TEST RESULTS - SOILS. 5 GEOTECHNICAL TESTING 5
5.0	GEOTECHNICAL ASSESSMENT
5.1 5.2 5.3 5.4 5.5 5.6 5.7	GENERAL 6 FOUNDATIONS. 6 FLOOR SLABS. 6 CONSTRUCTION 7 HIGHWAYS. 7 SOAKAWAYS 7 SLOPE STABILITY 7
6.0	ENVIRONMENTAL ASSESSMENT
6.1 6.2 6.3 6.4 6.5 6.7	CONTAMINATION8QUALITATIVE RISK ASSESSMENT8REMEDIAL MEASURES8ASBESTOS8HEALTH AND SAFETY ISSUES9WASTE DISPOSAL9
7.0	CONCLUSIONS11
7.1 7.2	SUMMARY 11 FURTHER WORK 11
8.0	REFERENCES12
	DRAWINGS
	Drawing Number Rev Title
	C2179/01 - Site Location Plan
	C2179/03 - Exploratory Hole Location Plan
	APPENDICES
	APPENDIX A Exploratory Hole Logs
	APPENDIX B Chemical Laboratory Testing Results
	APPENDIX C Geotechnical Laboratory Testing Results
	APPENDIX D Contaminated Land Screening Levels
	APPENDIX E Limitations



GEO-ENVIRONMENTAL ASSESSMENT REPORT ON A SITE OFF CHURCH RAIKE, CHIPPING

1.0 INTRODUCTION

- 1.1 This report describes a Geo-Environmental Investigation carried out for Prospect GB on a site off Church Raike, Chipping.
- 1.2 The objectives of the investigation were to:
 - Determine the near surface ground conditions through, window sampling related to the development of the site for residential development.
 - Carry out suitable testing to enable the ground to be assessed for chemical contamination.
 - Make recommendations for the foundations of both houses and associated roads.
 - Make comments and recommendations with regard to the geoenvironmental conditions encountered.
- 1.3 A Desk study has been carried out by Brownfield Solutions Limited (BSL) ref AJH/C2179/3577. The information within the desk study report details the site description and the environmental setting. This report should be read in conjunction with the Desk Study Report.

1

2.0 METHOD OF INVESTIGATION

2.1 Objectives

- 2.1.1 The aim of the fieldwork was to:
 - Investigate ground conditions on the site.
 - Assess the potential contamination on the site and obtain samples for contamination screening.
 - Assess the potential impact of any contamination on controlled waters.
 - Assess the need for detailed investigation.
 - Obtain geotechnical information on the ground conditions at the site for preliminary foundation design and preliminary pavement design purposes.
 - Install standpipes to allow future monitoring.
 - Give an assessment of the geo environmental risks associated with redevelopment of the site.

2.2 Site Works

- 2.2.1 Seven Window sample boreholes (WS01 to WS07) were drilled to depths between 1.70m and 3.80m on 14 February 2013 using a tracked window sampling rig and liners (windowless).
- 2.2.2 The approximate locations of the exploratory holes are indicated on the Exploratory Hole Location Plan, Drawing C2179/03. The exploratory hole logs are presented in Appendix A.
- 2.2.3 The exploratory holes were positioned to establish general ground conditions on the site. The exploratory holes were logged by an experienced geo-environmental engineer in general accordance with BS 5930 'Code of Practice for Site Investigations' 1999, BS EN 14688-1:2002 'Geotechnical Investigation and Testing Identification and classification of soil'.

2.3 Sampling

- 2.3.1 During the drilling of the exploratory holes, representative samples were taken at regular intervals to assist in the identification of the soils and to allow subsequent laboratory testing.
- 2.3.2 Twenty six disturbed soil samples were selected and taken during the site works. The type of sample being dependent upon the stratum and the purpose of analysis.
- 2.3.3 Disturbed samples of soil for chemical testing were placed in 1 litre plastic tubs and amber jars.



2.3.4 The distribution of samples taken across the site is recorded on the exploratory logs.

2.4 Laboratory Testing

- 2.4.1 As part of the assessment for potential contamination of the site, selected samples were taken for the purpose of chemical contamination testing.
- 2.4.2 In the absence of particularly contaminative processes on site and the lack of visual evidence of contamination impaction eight representative soil samples were screened for the following general suite of determinands:
 - Arsenic, cadmium, chromium (total and hexavalent), copper, lead, mercury, nickel, selenium, zinc, pH and Speciated PAH's.
- 2.4.3 Thee samples were screened for asbestos fibres. Two samples have also been scheduled for organo-phosphorus pesticides.
- 2.4.4 The Chemical Laboratory Testing Results are presented in Appendix B.
- 2.4.5 Representative disturbed samples were obtained for all soil types encountered. Selected samples were scheduled for testing at an approved laboratory in accordance with BS 1377 'Method of Test for Soils for Civil Engineering Purposes' 1990. The following tests were scheduled:

BS Test Number	Description	No of Samples
Part 2:	Natural Moisture Content	6
Part 2:	Plasticity Index Analysis	6
Part 3:	pH Value	6
Part 3:	Water Soluble Sulphate Content	6

2.4.6 The Geotechnical Laboratory Testing results are presented in Appendix C.

2.5 Monitoring

- 2.5.1 Gas standpipes were installed in the four of the boreholes on the site. The standpipes consisted of plain PVC pipe from ground level to 1.0m bgl, with slotted PVC pipe from 1.0m to the base of the borehole. A bentonite seal was made around the plain pipe. A clean gravel pack was placed around the slotted pipe
- 2.5.2 A program of ground gas monitoring will be instigated and will be reported in an addendum report.



3.0 GROUND CONDITIONS

3.1 Made Ground

3.1.1 Made Ground was not encountered in any of the window sample locations.

3.2 Natural Ground

- 3.2.1 The natural strata underlying the topsoil was generally a firm to stiff yellow brown sandy clay, overlying a firm to stiff and very stiff dark brown grey sandy clay.
- 3.2.2 The upper yellow brown clay contained some angular sandstone gravel and was present to depths of between 0.60m and 0.80m. The stiff dark grey sandy clay contained much fine to coarse gravel and occasional cobbles.
- 3.2.3 In WS01 a soft to firm dark brown sandy clay was present between 0.95m and 1.50m. In WS05 in the centre of the site the drill string refused at 1.70m on an assumed cobble. WS04 also refused at 3.80m probably on a cobble.
- 3.2.4 In WS07 there was a very thin band of coarse black sand at 1.50m.

3.3 Bedrock

3.3.1 Bedrock was not encountered in this investigation.

3.4 Groundwater

3.4.1 Groundwater was not generally encountered during the investigation, although in WS04 the clay was saturated below 1.20m.

3.5 Observations

- 3.5.1 During the works undertaken by BSL observations for both visual and olfactory evidence of contamination were made.
- 3.5.2 There was not visual or olfactory evidence of contamination noted during the investigation.



4.0 TEST RESULTS

4.1 Chemical Test Results - Soils

- 4.1.1 The samples were tested for an assessment of the chemical contamination and results were examined with reference to a selection of guidance documents as detailed in Appendix D.
- 4.1.2 The apparent exceedence of the quoted Screening value is taken as indicating further detailed assessment or remedial action is required.
- 4.1.3 None of the chemical test results exceeded their respective screening concentrations for residential end use.
- 4.1.4 Asbestos fibres were not detected in the samples tested.
- 4.1.5 The results of the pesticide testing is outstanding and will be reported on receipt.

4.2 Geotechnical Testing

- 4.2.1 Water soluble sulphate testing was undertaken on six of the natural strata. The results revealed soluble sulphate (SO_4) contents of <0.01 g/l to 0.13g/l. Associated pH values were obtained which ranged between 5.2 and 8.2 and indicating slightly acid to slightly alkaline conditions.
- 4.2.2 Plasticity index results which ranged between 10% and 22%, moisture contents were in the range 11% to 30%.
- 4.2.3 After modification of particle size in accordance with NHBC Chapter 4.2 the modified plasticity indices are in the range 7.5% to 16.5% indicating the soils to be of low volume change potential.



5.0 GEOTECHNICAL ASSESSMENT

5.1 General

- 5.1.1 The site is currently a cricket pitch with a small pavilion. Made ground has not been found across the site.
- 5.1.2 The eastern part of the site has many semi-mature trees on and these will need to be removed to facilitate the development.

5.2 Foundations

- 5.2.1 The most suitable foundations for houses on this site are likely to be unreinforced strip foundations. The clay on the site is of low volume change potential, therefore the foundations should be at a minimum depth of 750mm, deeper near trees and hedges in accordance with NHBC Chapter 4.2.
- 5.2.2 A nett allowable bearing pressure not exceeding 90kN/m² should be assumed at 750mm, the shear strength increases with depth and foundations can be deepened if higher loads are required to be supported.
- 5.2.3 On the eastern part of the site there is an area of soft soil that extends to 1.50m in WS01. Foundations should be extended below this to suitable firm clays. It is likely given the trees in this area that this depth will be exceeded due to the area of influence in cohesive soils.
- 5.2.4 The bearing stratum should be inspected for 'soft spots' within the natural clay strata, resulting for instance from localised groundwater perched within the overlying fill materials. Any such soft spots should be dealt with in accordance with good site practice.
- 5.2.5 A survey of all trees and hedges on the site and within influencing distance of the site boundary should be undertaken to identify tree species and heights. This information will be required in order to assess the effects of trees on the cohesive strata.
- 5.2.6 Where foundation depths due to trees already present or recently removed exceeds 1.50m there is a possibility for heave to occur on removal of the tree. NHBC Guidance states that compressible material or void former is required against the inside face of all external wall foundations.

5.3 Floor Slabs

5.3.1 If required ground bearing floor slabs may generally be adopted at the site provided that once finished levels have been established, less than 600mm of suitable, appropriately compacted granular material exists beneath the slab.

5.3.2 Where foundation depths due to trees already present exceeds 1.50m there is a possibility for heave to occur on removal of the tree. NHBC Guidance states that either a precast concrete floor, a suspended timber or in-situ concrete floor must be used. We recommend the former, the required void size for beneath floor slabs on this site is 125mm low.

5.4 Construction

- 5.4.1 Instability of excavations through natural soils is not anticipated provided they are not exposed to adverse weather conditions for any substantial period of time. All excavations should be carried out in accordance with CIRIA Report 97 'Trenching Practice'.
- 5.4.2 Excavation depths should generally be readily achieved using conventional plant (JCB or similar) although high specification plant (tracked 360° or similar) and possibly breaking equipment may be required locally to penetrate old foundations associated with pavillion.
- 5.4.3 The results of laboratory pH and sulphate content testing indicates that ACEC Class AC-1 and sulphate class DS-1 conditions prevail in accordance with BRE Special Digest 1 "Concrete in aggressive ground" 2005. The specific concrete mixes (the Design Concrete Class) to be used on site will be determined by the site specific concrete requirements in terms of the durability and structural performance. These are assessed in terms of the Structural Performance Level (SPL) and any need for Additional Protective Measures (APM) detailed in Part D of BRE Special Digest 1 with further guidance in Pt E and F.

5.5 Highways

5.5.1 Cohesive soils will be encountered at road formation levels, therefore CBR values of 2% to 5% are likely to be achieved in undisturbed natural soils for pavement design purposes. However unless proven otherwise by in-situ testing at sub-base level by a specialist geotechnical engineer, a design CBR value not exceeding 2% should be assumed.

5.6 Soakaways

5.6.1 The use of soakaways within the natural ground is not feasible at the site due to the presence of relatively impermeable strata underlying the site.

5.7 Slope Stability

5.7.1 The site is elevated above the road by approximately 2m and care will be required to ensure that foundation loads do not induce instability in this bank. It is recommended that houses are set back and foundations are set below a line of 45 degrees drawn up from the base of the bank.



6.0 ENVIRONMENTAL ASSESSMENT

6.1 Contamination

6.1.1 On the basis of the testing undertaken to date it would appear that there is no made ground on the site. The chemical testing indicates that the natural ground is uncontaminated.

6.2 Qualitative Risk Assessment

6.2.1 The risk assessment methodology used in this instance is based on **Source**- Pathway - Receptor (SPR) philosophy. The **source** is the presence of contamination, or substance/event likely to cause harm. The **receptor** is the target that may be detrimentally affected by the source. The **pathway** is the means of the contamination to move from the source to the receptor. Where any of these three factors are removed there is deemed to be no risk.

Source-Pathway-Receptor Analysis

Potential Contaminative Source / Location	Contaminant	Receptor	Likelihood	Justification
Pesticides from farming	Pesticides.	Site end users, workers	Low	Unlikely that significant contamination from pesticides is present. The results for the pesticide testing is outstanding however the risk is still considered to be low .
Landfill site	Ground gases	Confined spaces in	Low	Former landfill site situated 110m south west of the site and received difficult waste. Therefore
		buildings		the risk is considered to be moderate .
Natural Rock	Radon Gas	Confined spaces in buildings	High	Full Radon precautions are required. The risk is considered to be high .

- 6.2.2 It is considered that there is no source of soil contamination on the site.
- 6.2.3 The site requires full radon protection, other ground gas monitoring is ongoing.

6.3 Remedial Measures

- 6.3.1 Full radon precautions are required within the properties, this would mitigate any other ground gases as well.
- 6.3.2 No other remedial measures are considered unnecessary.

6.4 Asbestos

6.4.1 The investigation of asbestos issues within buildings was beyond the scope of this report. However, guidance from UK Government indicates that



asbestos should be assumed to be present in buildings unless proven otherwise.

6.4.2 Any asbestos will require removal prior to re-development. This will need to be done by a suitably qualified experienced and licensed contractor, who ensures that adequate PPE is provided to operatives, and that all the relevant legislation is adhered to.

6.5 Health and Safety Issues

- 6.5.1 No sources of contamination were recorded on the site, although the site is not contaminated it is good practice to prevent site workers from coming into contact with soils General guidance on these matters is given in the Health and Safety Executive (HSE) document "Protection of Workers and the General Public during the Redevelopment of Contaminated Land". In summary, the following measures are suggested to provide a minimum level of protection:
 - All ground workers should be issued with the relevant protective clothing, footwear and gloves. These protective items should not be removed from the site and personnel should be instructed as to why and how they are to be used.
 - Hand-washing and boot-washing facilities should be provided.
 - Care should be taken to minimise the potential for off-site migration of contamination by the provision of dust suppression control and wheel cleaning equipment during the construction works.
 - Good practices relating to personal hygiene should be adopted on the site
 - The contractor shall satisfy the Health and Safety Executive with regard to any other matters concerning the health, safety and welfare of persons on the site.

6.7 Waste Disposal

6.7.1 The Landfill Directive (Directive 1999/31/EC on the landfilling of waste) led to the establishment of a methodology for classifying wastes. Wastes can only be accepted at a landfill if they meet the relevant Waste Acceptance Criteria (WAC) for that type of landfill.

There are three different WAC, these are for:

- Inert waste
- Non –hazardous waste
- Hazardous waste
- 6.7.2 Wastes should be first classified based on their total concentrations. WAC testing is then required if the end disposal route is a hazardous waste landfill, Stable non-reactive waste cell or inert landfill.
- 6.7.3 The implementation of the landfill directive has had the effect of increasing significantly disposal costs. Dependent on the proposed disposal route

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- waste may need to be classified on the basis of detailed Waste Acceptance Criteria (WAC) testing, which is outside the scope of this work.
- 6.7.4 The total testing results indicate that generally the soils are inert (below the relevant SGV or GAC criteria). It is unlikely that the soils encountered would be classified as hazardous waste.
- 6.7.5 However as the material is generally clean it is entirely possible that the soils could be recycled at a suitable local waste treatment plant or transfer station or re-used on another site.
- 6.7.6 Where it is necessary to dispose material off site it is recommended that materials are segregated and where necessary sufficient time is allowed to further classify the material properly, including discussion with landfill sites and waste transfer stations to find the best disposal route.



7.0 CONCLUSIONS

7.1 Summary

- 7.1.1 The site has previously been used for agriculture and from the 1960's has been used as a cricket pitch.
- 7.1.2 The site does not contain any made ground and the soils on the site are indicated to be uncontaminated.
- 7.1.3 Strip foundations with a safe bearing capacity of 90kN/m² are considered suitable on the site. Locally some deepening may be required to found below soft spots. Deepening wil also be required due to trees
- 7.1.4 Care should be taken not to load the bank adjacent to the road and it is recommended that buildings are set back from this.
- 7.1.5 The site requires full radon precautions, a full gas assessment will be made on completion of the gas monitoring..

7.2 Further Work

- 7.2.1 The gas monitoring program should be completed.
- 7.2.2 The results of the pesticide testing is outstanding and will be reported on receipt.



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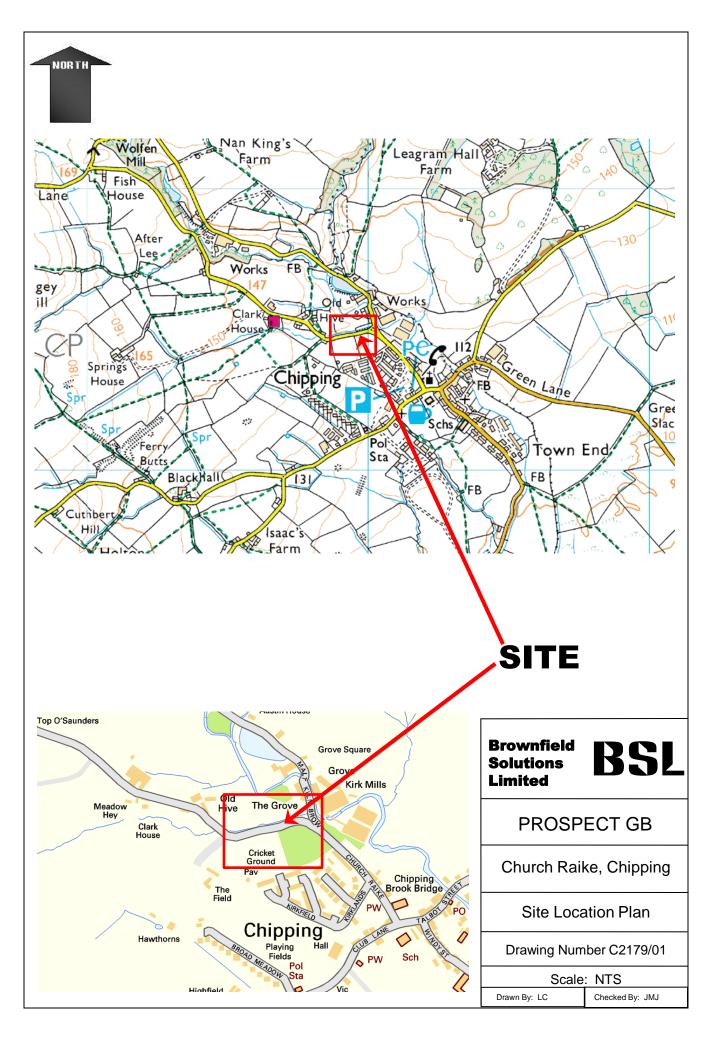
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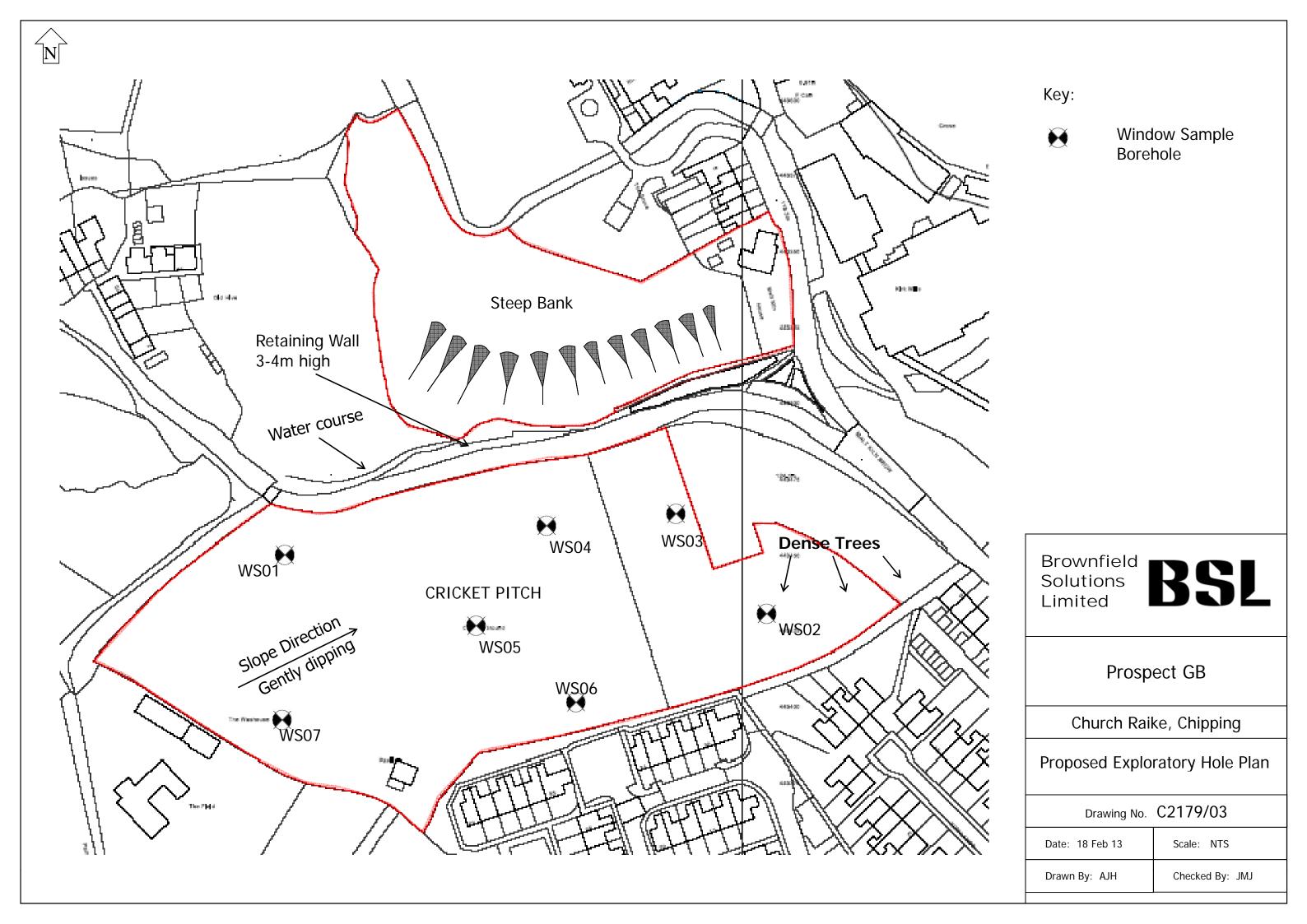
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13

DRAWINGS







APPENDIX A Exploratory Hole Logs

R	35					Wychwood 1 Queen Str Northwich Cheshire				Borehole No WS01			
L						CW9 5JL Tel: 01606 3	334844			Sheet 1 of 1			
Proj	ect Na	ame				oject N	ield-solutions.			Hole Type			
Chu	ırch Ra	aike, Chip	ping		C	2179		Co-ords: -		WLS			
Loca	ation:	Chippir	ng					Level: -		Scale 1:25			
Clie	nt:	Prospe	ect (GB	3) Ltd					ked by RW	Logged By AJH			
Well	Water	Sampl		Situ Testing	Depth	Level	Legend	Stratum Description					
	Strikes	Depth (m)	Туре	Results	(m)	(m AOD)	X X X X	Dark brown sandy clayey TOPSOIL	Puon				
		0.20	D		0.30			Firm yellow brown mottled grey sandy CLA gravel of sandstone	Y with some a	angular			
		0.90	D							-			
		1.00	SPT	N=5 (2,1,0,2,1,2)	0.95			Soft to firm dark brown sandy CLAY with so	ome angular g	ravel -1			
		1.50	D		1.50			Firm to stiff dark brown grey sandy CLAY v Gravel is fine to coarse angular sandstone	vith much grav	vel.			
		2.00	SPT	N=21 (3,4,4,5,5,7)				Becoming friable and very stiff below 2.	00	-2			
		2.50	D										
		3.00	SPT	N=31 (6,5,5,10,8,8)	3.00			End of Borehole at 3.0	 00 m	-3			
										-4			
_	\sqcup												

Remarks: 1. Groundwater not encountered.



DC					Wychwood 1 Queen Str Northwich	House eet			Borehole	
BS					Cheshire CW9 5JL Tel: 01606 3	334844			WS0 Sheet 1	
Project N	ame			Pr	oject N	ield-solutions.	com		Hole T	
	aike, Chip	ping			2179		Co-ords: -		WLS	
Location:	Chippin						Level: -		Scal 1:25	
Client:	Prospe	ct (GE	3) Ltd		Dates: 14/02/2013 Checked by RW			Checked by RW	Logged AJH	Ву
Well Water Strikes	Sample Depth (m)	es & Ir Type	Results	Depth (m)	Level (m AOD)	Legend	Stratui	!		
	Deptil (III)	(iii) Type Results ()			(7.02)		Dark brown sandy clayey TOPSO			
	0.20	D								
				0.30			Firm yellow brown mottled grey sa gravel of sandstone	andy CLAY with some	angular	
							gravel of sandstone			
										-
	0.80	D		0.80						-
	0.60			0.60			Firm to stiff to becoming very stiff with much gravel and occasional	dark brown grey sandy cobbles. Gravel is fine	/ CLAY to	-
;:∃::::	1.00	SPT	N=6 (2,2,2,1,1,2)				coarse angular sandstone			-1
			(2,2,2,1,1,2)							
										-
		_								-
	1.50	D								-
										-
										-
	2.00	SPT	N=21							-2
			(1,2,6,4,4,7)							-
										-
										-
										-
	3.00	SPT	N=42	3.00						3
	0.00		(5,5,5,12,15,10)	0.00			End of Bor	ehole at 3.00 m		-
										-
										-
										-
										-4
										-
										-
										-
										-
Remarks:	1. Ground	lwater i	not encountered.							

AGS

B	3					Wychwood 1 Queen Str Northwich Cheshire CW9 5.JL				WS03 Sheet 1 of 1	
Proj	ect Na	ame				www.brown				Hole Type	
		aike, Chip	ping			2179		Co-ords: -		WLS	
Loc	ation:	Chippin	ıg			Level: -			Scale 1:25		
Clie	nt:	Prospe	ct (GE	3) Ltd					Checked by RW	Logged By AJH	
Well	Water Strikes	Sample Depth (m)	es & In	Situ Testing Results	Depth (m)	th Level			7,011		
	Ounco	Depth (m)	Туре	Results	(111)	(III / IOD)		Dark brown sandy clayey TOPSO			-
		0.20	D		0.30			Firm to stiff yellow brown mottled angular gravel of sandstone	grey sandy CLAY with	some	
		0.60	D								
		1.00	SPT	N=11	0.80			Firm to stiff becoming very stiff da with much gravel and occasional coarse angular sandstone	ork brown grey sandy C cobbles. Gravel is fine	LAY to -1	ı
		1.20	D	(1,1,1,2,4,4)							
		2.00	SPT	N=19 (3,4,4,5,4,6)						-2	2
		2.50	D							-	
		3.00	SPT	N=24 (6,5,5,5,6,8)	3.00			End of Bon	ehole at 3.00 m	-3	\$
										-4	1
D.	arke:	1 Cround								-	

Remarks: 1. Groundwater not encountered.



	. =				Wychwood I				Borehole N	О
BS					1 Queen Str Northwich Cheshire	eet			WS04	
					CW9 5JL Tel: 01606 3				Sheet 1 of	1
Project N	lame				oject N	ield-solutions.c			Hole Type	
	aike, Chip	ping			2179		Co-ords: -		WLS	
Location:	Chippir	ng		!			l		Scale	
							Level: -		1:25	
Client:	Draana	at (CE)) 4d				Dates: 14/02/2013	Checked by	Logged By	/
Client:	Prospe	-			•			RW	AJH	
Well Water Strikes	Sampl Depth (m)	es & In Type	Situ Testing Results	Depth (m)	Level (m AOD)	Legend	Stratur	n Description		
	2 op ()	. , , , ,	riodano		, ,		Dark brown sandy clayey TOPSO	IL		
	0.20	D								-
				0.30			Firm light grey mottled brown slig	htly sandy CLAY with		-
							occasional angular gravel of sand	stone		-
	0.50	D								-
				0.70			Firm dark brown grey sandy CLA	/ with much gravel and	Loccasional	+
							cobbles. Gravel is fine to coarse a	angular sandstone	loccasional	-
	1.00	SPT	N=9							- -1
	1.00	JF I	(1,0,1,1,3,4)							
	1.25	D					Dark grey coarse sand band 1	.20m to 1.30m		-
							becoming stiff to very stiff with cobbles below 1.30m		asional	-
							coddies below 1.30m			
										-
	4.00									•
	1.80	D								
	2.00	SPT	N=23							-2
			(5,4,5,6,6,6)							
										-
										-
										-
										-
	3.00	SPT	N=25 (6,5,5,6,7,7)							-3
			(=,=,=,=,=,=,=,=,=,=,=,=,=,=,=,=,=,=,=,							
										•
										-
· H·	3.80	SPT	N=27 (4,5,5,7,7,8)	3.80			End of Bor	ehole at 3.80 m		-
			(4,0,0,1,1,0)							- -4
										-
										-
										-
										-
										-

Brownfield Solutions Ltd

Remarks: 1. Clay saturated below 1.20m. 2. Sampler bouncing at at 3.80m



		_	,			Wychwood I	rownfield Solutions Ltd Vychwood House					О
R	35					1 Queen Str Northwich Cheshire	eet				WS05	
						CW9 5JL Tel: 01606 3	334844 ield-solutions.c	om.			Sheet 1 of	1
Proj	ect Na	ame			Pr	oject N					Hole Type	•
Chu	rch Ra	aike, Chip	ping		C	C2179		Co-oras:	Co-ords: -			WLS
Loca	ation:	Chippin	ıg					Level:	-		Scale 1:25	
Clie	nt·	Prospe	ct (GF	3) I td				Dates:	14/02/2013	Checked by	Logged By	/
			•	•	B #	l				RW	AJH	
Well	Water Strikes	Depth (m)	Type	Results	Depth (m)	(m AOD)	Legend			n Description		
Well	Water Strikes	Sample Depth (m) 0.20 0.50 1.00 1.60 1.70		N=9 (1,2,2,2,2,3) 50/45mm 45mm (25,50)	Depth (m) 0.30 1.70	Level (m AOD)	Legend	Firm yellow b gravel of san	orown mottled grey sa dstone Decoming very stiff da avel and occasional			-1 -2 -344
												-

Remarks: 1. Groundwater not encountered. 2. Refusal at 1.70m on cobble.



DC	•				Brownfield S Wychwood 1 Queen Str Northwich	Solutions Ltd House reet			Borehole N	lo
BS					Cheshire CW9 5JL	204044			WS06	
Droinet Na	200.0			l D.		ield-solutions.co	om		Sheet 1 of Hole Type	
Project Na Church Ra		nina			oject N 2179	10.	Co-ords: -		WLS	3
Location:	Chippir				2175				Scale	
	- 11	3					Level: -		1:25	
Client:	Prospe	ot (CE	D/ I +d				Dates: 14/02/2013	Checked by	Logged By	y
				_				RW	AJH	
Well Water Strikes	Depth (m)	Samples & In Situ Testing th (m) Type Results		Depth (m)	Level (m AOD)	Legend	Stratur	n Description		
							Dark brown sandy clayey TOPSO	IL		-
	0.20	D								-
				0.30			Firm to stiff yellow brown mottled angular gravel of sandstone	grey sandy CLAY with	some	<u> </u>
							angular graver or sandstone			-
				0.60			Stiff dark brown grey sandy CLAY cobbles. Gravel is fine to coarse a	with much gravel and	occasional	†
							copples. Gravei is fine to coarse a	angular sandstone		_
	0.90	D	N 47							-
	1.00	SPT	N=17 (4,5,5,5,4,3)							-1 -
										-
										-
										-
										-
	1.80	D								-
		0.07								-
	2.00	SPT	N=17 (2,3,3,4,5,5)							-2
										-
										-
										-
										-
	2.00	SPT	N. 20	2.00						-
, ,	3.00	581	N=29 (6,5,5,6,9,9)	3.00			End of Bor	ehole at 3.00 m		-3
										-
										-
										-
										-
										-
										-4
										-
										-
										-
										-
										-
_ _										<u></u>
Remarks:	1. Ground	dwater r	not encountered.							

R	Q	21				Wychwood 1 Queen Str Northwich					Borehole WS07		
BSL 1 Queen Street Northwich Cheshire CW9 5JL Tel: 01606 334844 www.brownfield-solutions.com										Sheet 1 c			
Proje	ect N	ame			Pr	oject N						oe	
Church Raike, Chipping C2179								Co-ords:	Co-ords: -				
Location: Chipping								Level:	-		Scale 1:25		
Clier	nt:	Prospe	ct (GE	3) Ltd				Dates:	14/02/2013	Checked by RW	Logged I AJH	Зу	
Well	Water Strikes	Sample	es & In	Situ Testing	Depth (m)	Level (m AOD)	Legend		Stratur	n Description	7.011		
	Strikes	Depth (m)	Туре	Results	(m)	(m AOD)) Logona	Dark brown sa	andy clayey TOPSO				
		0.20	D		0.30			Firm to stiff ve	ellow brown mottled	grey sandy CLAY with	some		
		0.50	D					angular grave	l of sandstone				
		1.00	SPT	N=11 (2,1,1,4,3,3)	0.60			Firm to stiff da occasional co	ark brown grey sand bbbles. Gravel is fine	y CLAY with much gra to coarse angular san	vel and dstone	-1	
		1.50	D									[
		1.60	D					very tnin b	and of black coarse	sand at 1.50m		-	
		2.00	SPT	N=28								-2	
		2.00		(13,4,6,5,5,12)				becoming	very stiff below 2.0n	า		-	
		2.00-2.45	D										
::: ::::		3.00	SPT	N=36 (9,12,8,9,8,11)	3.00				End of Bore	ehole at 3.00 m		3	
				(-, -, -) 5 (5) (1)								-4	
												-	
												-	
												-	
Dam	arke:	1 Cround				ļ							

Remarks: 1. Groundwater not encountered.





APPENDIX B Chemical Laboratory Testing Results



Depot Road Newmarket CB8 0AL Tel: 01638 606070

Brownfield Solutions Limited Wychwood House 1 Queen Street Northwich, Cheshire CW9 5JL

FAO Tony Hewitt 22 February 2013

Dear Tony Hewitt

Test Report Number 224002

Your Project Reference C2179 - Church Raike, Chipping

Please find enclosed the results of analysis for the samples received 19 February 2013.

All soil samples will be retained for a period of one month and all water samples will be retained for 7 days following the date of the test report. Should you require an extended retention period then please detail your requirements in an email to customerservices@chemtest.co.uk. Please be aware that charges may be applicable for extended sample storage.

If you require any further assistance, please do not hesitate to contact the Customer Services team.

Yours sincerely





2183



- Notes to accompany report:
- The sign < means 'less than'
- Tests marked 'U' hold UKAS accreditation
- Tests marked 'M' hold MCertS (and UKAS) accreditation
- Tests marked 'N' do not currently hold UKAS accreditation
- · Tests marked 'S' were subcontracted to an approved laboratory
- n/e means 'not evaluated'
- i/s means 'insufficient sample
- u/s means 'unsuitable sample'
- Comments or interpretations are beyond the scope of UKAS accreditation.
- The results relate only to the items tested
- All results are expressed on a dry weight basis
- The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH. BTEX. VOCs, SVOCs, PCBs, phenols
- For all other tests the samples were dried at < 37°C prior to analysis
- Uncertainties of measurement for the determinands tested are available upon request
- None of the test results included in this report have been recovery corrected.

Test Report 224002 Cover Sheet

FAO Tony Hewitt

LABORATORY TEST REPORT

Chemtest
The right chemistry to deliver results

Results of analysis of 13 samples received 18 February 2013

Report Date 22 February 2013

C2179 - Church Raike, Chipping

Login I	Batch No						224	1002		
Chemte	est LIMS ID				Al31467	Al31468	Al31469	Al31470	Al31472	Al31473
Sample	ID				WS1	WS1	WS2	WS2	WS3	WS3
Sample	· No									
Sampli	ng Date				14/2/2013	14/2/2013	14/2/2013	14/2/2013	14/2/2013	14/2/2013
epth					0.20m	0.90m	0.20m	0.80m	0.60m	2.50m
Лatrix					SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
SOP↓	Determinand↓	CAS No↓ U	nits↓ *							
2010	рН			М	6.3	6.9	5.5	7.4	6.3	8.0
2120	Sulfate (2:1 water soluble) as SO4	14808798	g l-¹	М		<0.01		<0.01		0.13
2490	Chromium (hexavalent)	18540299	mg kg-1	N	<0.5	<0.5	<0.5		<0.5	
2450	Arsenic	7440382	mg kg-1	М	12	13	7.7		19	
	Cadmium	7440439	mg kg-1	М	0.83	1.3	0.59		5.7	
	Chromium	7440473	mg kg-1	М	12	16	7.6		12	
	Copper	7440508	mg kg-1	М	43	33	28		36	
	Mercury	7439976	mg kg-1	М	0.19	<0.10	0.10		<0.10	
	Nickel	7440020	mg kg-1	М	12	29	9.9		65	
	Selenium	7782492	mg kg-1	М	1.4	1.1	0.80		0.90	
	Zinc	7440666	mg kg-1	М	78	110	59		160	
2800	Naphthalene	91203	mg kg-1	М	<0.10	<0.10	<0.10		<0.10	
	Acenaphthylene	208968	mg kg-1	N	<0.10	<0.10	<0.10		<0.10	
	Acenaphthene	83329	mg kg-1	M	<0.10	<0.10	<0.10		<0.10	
	Fluorene	86737	mg kg-1	М	<0.10	<0.10	<0.10		<0.10	
	Phenanthrene	85018	mg kg-1	M	<0.10	<0.10	<0.10		<0.10	
	Anthracene	120127	mg kg-1	М	<0.10	<0.10	<0.10		<0.10	
	Fluoranthene	206440	mg kg-1	М	<0.10	<0.10	<0.10		<0.10	
	Pyrene	129000	mg kg-1	М	<0.10	<0.10	<0.10		<0.10	
	Benzo[a]anthracene	56553	mg kg-1	М	<0.10	<0.10	<0.10		<0.10	
	Chrysene	218019	mg kg-1	М	<0.10	<0.10	<0.10		<0.10	
	Benzo[b]fluoranthene	205992	mg kg-1	М	<0.10	<0.10	<0.10		<0.10	
	Benzo[k]fluoranthene	207089	mg kg-1	N	<0.10	<0.10	<0.10		<0.10	
	Benzo[a]pyrene	50328	mg kg-1	M	<0.10	<0.10	<0.10		<0.10	

All tests undertaken between 19/02/2013 and 22/02/2013

^{*} Accreditation status

FAO Tony Hewitt

LABORATORY TEST REPORT

Chemtest
The right chemistry to deliver results

Results of analysis of 13 samples received 18 February 2013

Report Date 22 February 2013

C2179 - Church Raike, Chipping

Logi	n Batch No						224	002		
Cher	mtest LIMS ID				Al31474	Al31475	Al31476	Al31477	Al31478	Al31479
Sam	ple ID				WS4	WS5	WS5	WS6	WS6	WS7
Sam	ple No									
Sam	pling Date				14/2/2013	14/2/2013	14/2/2013	14/2/2013	14/2/2013	14/2/2013
Dept	th				0.50m	0.20m	0.50m	0.20m	1.80m	1.60m
Matr	İX				SOIL	SOIL	SOIL	SOIL	SOIL	SOIL
SOF	² ↓ Determinand↓	CAS No↓	Units↓	*						
2010	pH			M	6.7	4.7	5.8	5.2	7.9	8.2
2120	Sulfate (2:1 water soluble) as SO4	14808798	g l-¹	М			<0.01		<0.01	
2490	Chromium (hexavalent)	18540299	mg kg-1	N	<0.5	<0.5		<0.5		<0.5
2450	Arsenic	7440382	mg kg-1	M	3.6	9.4		12		15
	Cadmium	7440439	mg kg-1	M	0.35	0.57		0.71		2.2
	Chromium	7440473	mg kg-1	M	22	8.4		8.6		13
	Copper	7440508	mg kg-1	M	9.3	32		33		31
	Mercury	7439976	mg kg-1	M	<0.10	0.20		0.11		<0.10
	Nickel	7440020	mg kg-1	M	17	9.4		13		48
	Selenium	7782492	mg kg-1	M	0.42	0.89		1.3		11
	Zinc	7440666	mg kg-1	M	56	53		62		110
2800	Naphthalene	91203	mg kg-1	M	<0.10	<0.10		<0.10		<0.10
	Acenaphthylene	208968	mg kg-1	N	<0.10	<0.10		<0.10		<0.10
	Acenaphthene	83329	mg kg-1	M	<0.10	<0.10		<0.10		<0.10
	Fluorene	86737	mg kg-1	M	<0.10	<0.10		<0.10		<0.10
	Phenanthrene	85018	mg kg-1	M	<0.10	<0.10		<0.10		<0.10
	Anthracene	120127	mg kg-1	M	<0.10	<0.10		<0.10		<0.10
	Fluoranthene	206440	mg kg-1	M	<0.10	<0.10		<0.10		<0.10
	Pyrene	129000	mg kg-1	M	<0.10	<0.10		<0.10		<0.10
	Benzo[a]anthracene	56553	mg kg-1	M	<0.10	<0.10		<0.10		<0.10
	Chrysene	218019	mg kg-1	M	<0.10	<0.10		<0.10		<0.10
	Benzo[b]fluoranthene	205992	mg kg-1	M	<0.10	<0.10		<0.10		<0.10
	Benzo[k]fluoranthene	207089	mg kg-1	N	<0.10	<0.10		<0.10		<0.10

< 0.10

<0.10

Benzo[a]pyrene

<0.10

<0.10

mg kg-1

M

50328

^{*} Accreditation status

FAO Tony Hewitt

LABORATORY TEST REPORT

Chemtest
The right chemistry to deliver results

Results of analysis of 13 samples received 18 February 2013

Report Date
22 February 2013

C2179 - Church Raike, Chipping

Login Batch No				224002
Chemtest LIMS ID				Al31480
Sample ID				WS7
Sample No				
Sampling Date				14/2/2013
Depth				2.00m - 2.45m
Matrix				SOIL
SOP↓ Determinand↓	CAS No↓	Units↓	*	
2010 pH			М	7.8
2120 Sulfate (2:1 water soluble) as SO4	14808798	g l-1	M	0.11
2490 Chromium (hexavalent)	18540299	mg kg-1	N	
2450 Arsenic	7440382	mg kg-1	M	
Cadmium	7440439	mg kg-1	M	
Chromium	7440473	mg kg-1	М	
Copper	7440508	mg kg-1	M	
Mercury	7439976	mg kg-1	М	
Nickel	7440020	mg kg-1	M	
Selenium	7782492	mg kg-1	M	
Zinc	7440666	mg kg-1	M	
2800 Naphthalene	91203	mg kg-1	М	
Acenaphthylene	208968	mg kg-1	N	
Acenaphthene	83329	mg kg-1	M	
Fluorene	86737	mg kg-1	M	
Phenanthrene	85018	mg kg-1	М	
Anthracene	120127	mg kg-1	M	
Fluoranthene	206440	mg kg-1	М	
Pyrene	129000	mg kg-1	M	
Benzo[a]anthracene	56553	mg kg-1	М	
Chrysene	218019	mg kg-1	М	
Benzo[b]fluoranthene	205992	mg kg-1	М	
Benzo[k]fluoranthene	207089	mg kg-1	N	
Benzo[a]pyrene	50328	mg kg-1	М	

^{*} Accreditation status

LABORATORY TEST REPORT

Chemtest
The right chemistry to deliver results

Results of analysis of 13 samples received 18 February 2013

Report Date 22 February 2013

C2179 - Church Raike, Chipping

FAO Tony Hewitt

					224002							
					Al31467	Al31468	Al31469	Al31470	Al31472	Al31473		
					WS1	WS1	WS2	WS2	WS3	WS3		
					14/2/2013	14/2/2013	14/2/2013	14/2/2013	14/2/2013	14/2/2013		
					0.20m	0.90m	0.20m	0.80m	0.60m	2.50m		
					SOIL	SOIL	SOIL	SOIL	SOIL	SOIL		
2800	Dibenzo[a,h]anthracene	53703	mg kg-1	N	<0.10	<0.10	<0.10		<0.10			
	Indeno[1,2,3-cd]pyrene	193395	mg kg-1	M	<0.10	<0.10	<0.10		<0.10			
	Benzo[g,h,i]perylene	191242	mg kg-1	M	<0.10	<0.10	<0.10		<0.10			
	Total (of 16) PAHs		mg kg-1	N	<2.0	<2.0	<2.0		<2.0			

FAO Tony Hewitt

LABORATORY TEST REPORT

Chemtest
The right chemistry to deliver results

Results of analysis of 13 samples received 18 February 2013

Report Date 22 February 2013

C2179 - Church Raike, Chipping

					224002							
					Al31474	Al31475	Al31476	Al31477	Al31478	Al31479		
					WS4	WS5	WS5	WS6	WS6	WS7		
					14/2/2013	14/2/2013	14/2/2013	14/2/2013	14/2/2013	14/2/2013		
					0.50m	0.20m	0.50m	0.20m	1.80m	1.60m		
					SOIL	SOIL	SOIL	SOIL	SOIL	SOIL		
2800	Dibenzo[a,h]anthracene	53703	mg kg-1	N	<0.10	<0.10		<0.10		<0.10		
	Indeno[1,2,3-cd]pyrene	193395	mg kg-1	M	<0.10	<0.10		<0.10		<0.10		
	Benzo[g,h,i]perylene	191242	mg kg-1	M	<0.10	<0.10		<0.10		<0.10		
	Total (of 16) PAHs		mg kg-1	N	<2.0	<2.0		<2.0		<2.0		

^{*} Accreditation status

LABORATORY TEST REPORT

Chemtest
The right chemistry to deliver results

Results of analysis of 13 samples received 18 February 2013

Report Date 22 February 2013

FAO Tony Hewitt

C2179 - Church Raike, Chipping

					224002
					Al31480
					WS7
					14/2/2013
					2.00m - 2.45m
					SOIL
2800	Dibenzo[a,h]anthracene	53703	mg kg-1	N	
	Indeno[1,2,3-cd]pyrene	193395	mg kg-1	М	
	Benzo[g,h,i]perylene	191242	mg kg-1	М	
	Total (of 16) PAHs		mg kg-1	N	



Depot Road Newmarket CB8 0AL Tel: 01638 606070

Brownfield Solutions Limited Wychwood House 1 Queen Street Northwich, Cheshire CW9 5JL

FAO Tony Hewitt 26 February 2013

Dear Tony Hewitt

Test Report Number 224002

Your Project Reference C2179 - Church Raike, Chipping

Please find enclosed the results of analysis for the samples received 19 February 2013.

If you require any further assistance, please do not hesitate to contact the Customer Services team.

Yours sincerely

Darrell Hall, Director



2183



Notes to accompany report.

- The in-house procedure is employed to identify materials and fibres in soils
- The sample is examined by stereo-binocular and polarised light microscopy
- · Sample size is reduced by coning and quartering to obtain a representative sub-sample if necessary
- The bulk identification is in accordance with the requirements of the analyst guide (HSG 248)
- Samples associated with asbestos are retained for six months
- The results relate only to the items tested as supplied by the client
- Comments or interpretations are beyond the scope of UKAS accreditation

Brownfield Solutions Limited Wychwood House 1 Queen Street Northwich, Cheshire CW9 5JL

LABORATORY TEST REPORT Asbestos in Soils



Results of analysis of 3 samples received 18 February 2013 C2179 - Church Raike, Chipping

Report Date 26 February 2013

FAO Tony Hewitt

Login Batch No: 224002

Qualitative Results

				SOP 2190	
				ACM Type	Asbestos Identification
Chemtest ID	Sample ID	Sample Desc	Depth (m)		
Al31467		WS1	0.20	-	No Asbestos Detected
Al31471		WS3	0.20	-	No Asbestos Detected
Al31477		WS6	0.20	-	No Asbestos Detected

The detection limit for this method is 0.001%

Signed

Albert Vella

Senior Environmental Surveyor

All tests undertaken between 22-Feb-2013 and 22-Feb-2013



APPENDIX C Geotechnical Laboratory Testing Certificates



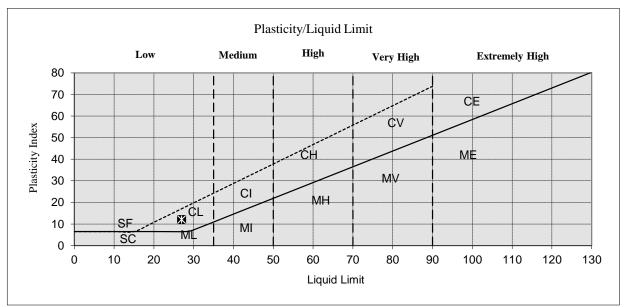
Ruby House, 40A Hardwick Grange, Warrington WA1 4RF Tel (01925) 286880 Fax (01925) 286881



LABORATORY TEST REPORT LIQUID & PLASTIC LIMIT TESTS BS 1377: Part 2: 1990 Cl 4.4,5.3

Site:	Church Raike, Chipping	Job No.:	-
Client:	Brownfield Solutions Ltd	Lab Ref No.:	SA13268/01
	Wychwood House	Sample Ref.:	WS1 @ 1.50m
	1 Queen Street	Date Received:	20/02/2013
	Northwich CW9 5JL	Date Tested:	27/02/2013
Originator:	Anthony Hewitt	Date Reported:	27/02/2013

Sampling Certificate	No	
Sampled By	Client	
Sample Type	Disturbed	
Sample Preparation Method	Washed	
MATERIAL	Brown Gravelly Clay	
Retained 425 micron (%)	30	
Natural Moisture Content (%)	13	
Liquid Limit (single point)(%)	27	
Plastic Limit (%)	15	
Plasticity Index	12	



Approved Signature TESTCONSULT LIMITED ULBaker



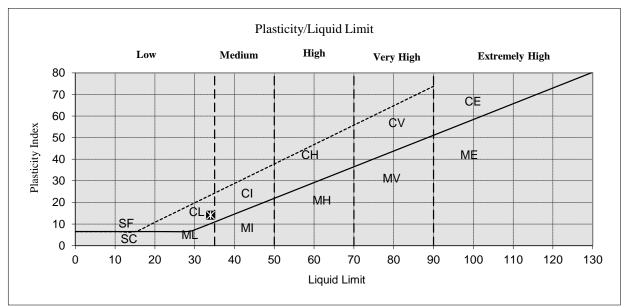
Ruby House, 40A Hardwick Grange, Warrington WA1 4RF Tel (01925) 286880 Fax (01925) 286881



LABORATORY TEST REPORT LIQUID & PLASTIC LIMIT TESTS BS 1377: Part 2: 1990 Cl 4.4,5.3

Site:	Church Raike, Chipping	Job No.:	-
Client:	Brownfield Solutions Ltd	Lab Ref No.:	SA13268/05
	Wychwood House	Sample Ref.:	WS5 @ 1.60m
	1 Queen Street	Date Received:	20/02/2013
	Northwich CW9 5JL	Date Tested:	27/02/2013
Originator:	Anthony Hewitt	Date Reported:	27/02/2013

Sampling Certificate	No	
Sampled By	Client	
Sample Type	Disturbed	
Sample Preparation Method	Washed	
MATERIAL	Brown Gravelly Clay	
Retained 425 micron (%)	25	
Natural Moisture Content (%)	15	
Liquid Limit (single point)(%)	34	
Plastic Limit (%)	20	
Plasticity Index	14	



Approved Signature TESTCONSULT LIMITED

Matibales



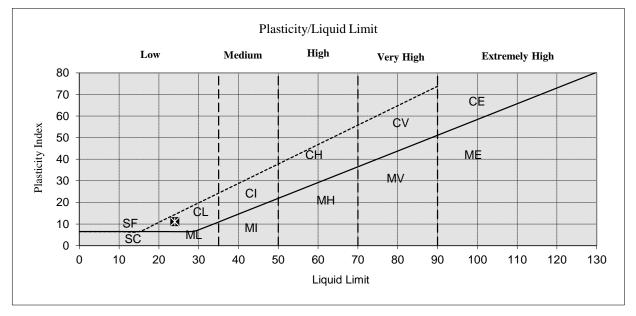
Ruby House, 40A Hardwick Grange, Warrington WA1 4RF Tel (01925) 286880 Fax (01925) 286881



LABORATORY TEST REPORT LIQUID & PLASTIC LIMIT TESTS BS 1377: Part 2: 1990 Cl 4.4,5.3

Site:	Church Raike, Chipping	Job No.:	-
Client:	Brownfield Solutions Ltd	Lab Ref No.:	SA13268/04
	Wychwood House	Sample Ref.:	WS4 @ 1.80m
	1 Queen Street	Date Received:	20/02/2013
	Northwich CW9 5JL	Date Tested:	27/02/2013
Originator:	Anthony Hewitt	Date Reported:	27/02/2013

Sampling Certificate	No	
Sampled By	Client	
Sample Type	Disturbed	
Sample Preparation Method	Washed	
MATERIAL	Brown Gravelly Clay	
Retained 425 micron (%)	25	
Natural Moisture Content (%)	11	
Liquid Limit (single point)(%)	24	
Plastic Limit (%)	13	
Plasticity Index	11	



Approved Signature TESTCONSULT LIMITED UtBaker



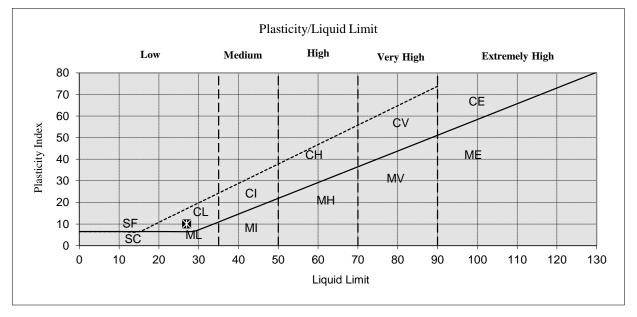
Ruby House, 40A Hardwick Grange, Warrington WA1 4RF Tel (01925) 286880 Fax (01925) 286881



LABORATORY TEST REPORT LIQUID & PLASTIC LIMIT TESTS BS 1377: Part 2: 1990 Cl 4.4,5.3

Site:	Church Raike, Chipping	Job No.:	-
Client:	Brownfield Solutions Ltd	Lab Ref No.:	SA13268/03
	Wychwood House	Sample Ref.:	WS3 @ 1.20m
	1 Queen Street	Date Received:	20/02/2013
	Northwich CW9 5JL	Date Tested:	27/02/2013
Originator:	Anthony Hewitt	Date Reported:	27/02/2013

Sampling Certificate	No	
Sampled By	Client	
Sample Type	Disturbed	
Sample Preparation Method	Washed	
MATERIAL	Brown Gravelly Clay	
Retained 425 micron (%)	25	
Natural Moisture Content (%)	13	
Liquid Limit (single point)(%)	27	
Plastic Limit (%)	17	
Plasticity Index	10	



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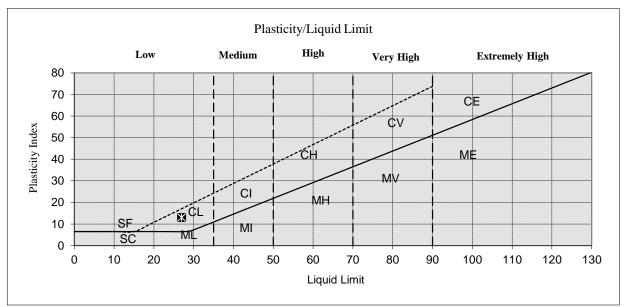
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LABORATORY TEST REPORT LIQUID & PLASTIC LIMIT TESTS BS 1377: Part 2: 1990 Cl 4.4,5.3

Site:	Church Raike, Chipping	Job No.:	-
Client:	Brownfield Solutions Ltd	Lab Ref No.:	SA13268/02
	Wychwood House	Sample Ref.:	WS2 @ 1.50m
	1 Queen Street	Date Received:	20/02/2013
	Northwich CW9 5JL	Date Tested:	27/02/2013
Originator:	Anthony Hewitt	Date Reported:	27/02/2013

Sampling Certificate	No	
Sampled By	Client	
Sample Type	Disturbed	
Sample Preparation Method	Washed	
MATERIAL	Brown Gravelly Clay	
Retained 425 micron (%)	30	
Natural Moisture Content (%)	12	
Liquid Limit (single point)(%)	27	
Plastic Limit (%)	14	
Plasticity Index	13	



Approved Signature TESTCONSULT LIMITED

Utbaker



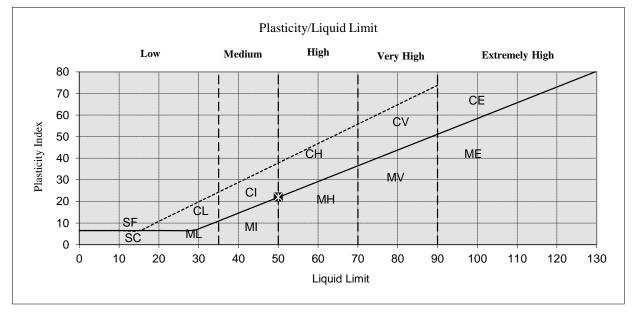
Ruby House, 40A Hardwick Grange, Warrington WA1 4RF Tel (01925) 286880 Fax (01925) 286881



LABORATORY TEST REPORT LIQUID & PLASTIC LIMIT TESTS BS 1377: Part 2: 1990 Cl 4.4,5.3

Site:	Church Raike, Chipping	Job No.:	-
Client:	Brownfield Solutions Ltd	Lab Ref No.:	SA13268/06
	Wychwood House	Sample Ref.:	WS7 @ 0.50m
	1 Queen Street	Date Received:	20/02/2013
	Northwich CW9 5JL	Date Tested:	27/02/2013
Originator:	Anthony Hewitt	Date Reported:	27/02/2013

Sampling Certificate	No	
Sampled By	Client	
Sample Type	Disturbed	
Sample Preparation Method	Washed	
MATERIAL	Brown Sandy Clay	
Retained 425 micron (%)	25	
Natural Moisture Content (%)	30	
Liquid Limit (single point)(%)	50	
Plastic Limit (%)	28	
Plasticity Index	22	



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AJH/C2179/3577



APPENDIX D Contaminated Land Screening Levels



Contaminated Land Screening Values

In assessing the potential for contamination Brownfield Solutions Limited (BSL) follows UK guidance and current best practice.

General

The current recommended method for assessing contamination is on the basis of:

Source-Pathway-Receptor

Where any one of these "pollution linkages" is absent there is deemed to be no risk.

Fundamentally receptors can be considered as humans and controlled waters (surface and ground waters).

The purpose of using screening levels is to have a simple means of assessing the potential contamination of a site and to inform decisions on whether further investigation is warranted or whether an option to undertake clean up based on the data to hand is cost effective.

Human Health

Current UK guidance is provided by DEFRA and the Environment Agency(EA). Publications forming part of the guidance include; CLEA Model, toxicological reports and soil guideline values (SGV), collectively referred to as the CLEA Guidance. The CLEA Guidance has included a number of publications which have provided initial screening values for soil contamination based on standard land uses and soil assumptions.

CLEA guidance has gone through a number of revisions, all of the original SGV's that were published have been withdrawn and publication of new SGV's started in 2009. The preference from the EA is that site specific screening levels are used wherever possible. Due to numerous factors it is not always possible to utilise site specific values. In these instances BSL uses the following data sources in the order of preference given below:

- Current UK SGV's
- CIEH GAC values (derived by LQM)
- Withdrawn UK SGV's
- Guidance from other European countries
- Guidance from the rest of the World.

Controlled Waters

The impaction of contamination on controlled waters is assessed by the comparison with Environmental Quality Standards (EQS). The EQS's cover a large number of compounds.

Where certain compounds are not covered by the EQS these are commonly compared to the UK Drinking Water Standards (DWS).

Further Assessment

When screening values are exceeded then further consideration is required. This could include the use of simple measures to break the pollution pathway and mitigate the risk, further more detailed investigation, including the deriving of site specific values to better define the risk and to design appropriate remedial measures.

APPENDIX E Limitations



Limitations

This ground investigation was conducted and has been prepared for the sole internal use and reliance of the Client, Prospect (GB) and cannot not be relied upon or transferred to any other parties without the express written authorisation of BSL. If an unauthorised third party comes into possession of this report they rely on it at their risk and the authors owe them no duty of care or skill.

The findings and opinions conveyed via the desk study within this report are based on information obtained from a variety of sources as detailed within this report, which BSL believes are reliable. In addition if information has been used from third parties and in particular other investigations and reports this information has been used in good faith. BSL cannot and does not guarantee the authenticity or reliability of third party information it has relied upon.

The investigation carried out on the site has been conducted to provide the best information and assessment on the ground conditions within site access and budgetary constraints. Exploratory holes only investigate a small area in relation to the overall site area and can therefore only provide a general indication of overall site conditions. Therefore the findings, opinions, geotechnical and environmental recommendations within this report are based on the ground conditions encountered at each location. It should be noted that different ground conditions may exist that have not been identified within this investigation.

The occurrence of and depths to groundwater may vary seasonally due to changes in weather, it should be noted that any observations and recommendations made on groundwater within this report are based on a select number of site visits over a limited period of time and may not be fully representative of groundwater conditions on the site.

Current UK guidance and legislation has been used in the geotechnical and environmental assessment of the site, BSL is not liable for any subsequent changes in the guidance and legislation.

The recommendations within this report are based upon the proposed site end use provided to BSL at the time of the investigation. If the end use or development layout changes from the proposal then the recommendations may change or become invalid.

Although every effort has been made to position exploratory holes in the least sensitive areas of the site, exploratory hole positions were located approximately as part of this investigation and no guarantee can be given as to their accuracy. Consideration should be given to the possibility that exploratory holes excavated as part of this investigation and indeed any previous ground investigation work by others may be encountered beneath or within the influence of individual foundations. BSL cannot be held responsible for structural failures caused by the location of foundations of any form of structure within the influence of exploratory holes.

No existing manhole covers were lifted or drainage runs inspected during the course of this ground investigation. The site plans enclosed in this report should not be scaled off.