

ENVIRONMENTAL & GEOTECHNICAL **ENGINEERING**

















Phase II **Intrusive** Ground Investigation www.hspconsulting.c

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Land off Chatburn Road, **Clitheroe**

C2099



PROPOSED RESIDENTIAL DEVELOPMENT, CHATBURN ROAD, CLITHEROE

Phase II Geo-Environmental Assessment Report

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Executive Summary

HSP Consulting has been commissioned by Oakmere Homes NW Ltd to provide a Phase II Geoenvironmental Assessment report providing information on likely constraints to the development of the site, parameters for design and recommendations for any mitigation measures should they be required.

The site comprises two open fields to the north west of Chatburn Road, 1.5km north east of Clitheroe town centre at approximate National Grid reference (NGR) 375200,442990. The site address is Chatburn Road, Clitheroe, BB7 2EQ. It is proposed to develop the site to provide twenty eight residential houses.

The ground investigation comprised ten window sample boreholes to a maximum depth of 2.80m, twelve machine excavated trial pits to a maximum depth of 2.60m and three cable percussive boreholes to a maximum depth of 3.00m to provide information for foundation design and obtain representative disturbed soil samples to forward for geotechnical and geo-environmental analysis. The geology of the site comprises topsoil to a maximum depth of 0.30m begl overlying firm to stiff clay, gravel and limestone of the Clitheroe Limestone Formation and Hodder Mudstone Formation, the full depth of which was not proven.

The natural cohesive deposits belonging to the Clitheroe Limestone Formation and Hodder Mudstone Formation are considered as suitable a formation layer for the proposed houses where they have been encountered in a medium strength condition from a minimum depth of 0.50m (i.e. at least 200mm into the natural weathered bedrock deposits). At the above depth HSP would recommend that an allowable bearing pressure of 126kNm² should be readily achievable when utilising a 0.60m wide strip trench footing.

It is considered appropriate to adopt a basic Design Sulphate Class of DS-1 together with an Aggressive Chemical Environment for Concrete (ACEC) of AC-1.

Elevated concentrations of Arsenic have been identified in one location at the site at 0.50m depth. Contamination at this depth is unlikely to pose a risk to end users and remediation is unlikely to be required unless ground levels are to be reduced in the area. Elevated levels of Benzo[b]fluoranthene and Dibenz(a,h)Anthracene have been identified in topsoil from WS3A and WS5. This material will need to be removed from site and is not suitable for re-use in gardens on the site.

Analysis of the ground gas monitoring undertaken to date indicates the site falls into a Characteristic Situation 1 / Green. Therefore gas protection measures are not necessary within any new developments upon the site.

The executive summary contains an overview of key findings and conclusions. However no reliance should be placed on the executive summary until the whole of the report has been read. Other sections of the report may contain information which puts into context the findings noted within the executive summary.



1. Introduction

1.1 Background

Oakmere Homes NW Ltd propose develop the site with twenty eight residential properties and associated gardens, soft landscaping and access roads. Our Client intends to submit this report to support the discharge of planning conditions.

1.2 Client Brief & Scope

HSP Consulting has been commissioned by Oakmere Homes NW Ltd to undertake an intrusive ground investigation at the site to investigate the existing ground conditions and provide information on likely constraints to the development, parameters for design and recommendations for any mitigation measures should they be required.

The report presents the following information:

- a summary of the previous Geo-environmental Reports (Section 1.4 below),
- details of the ground investigation undertaken and the ground conditions encountered,
- details and results of the geotechnical testing and contamination analysis,
- recommendations for mitigating constraints to the proposed development where appropriate and providing parameters for foundation design.

Where applicable, the fieldwork was undertaken in accordance with BS5930:1999 Code of Practice for Site Investigations and BS10175:2001 Investigation of Potentially Contaminated Sites.

1.2 Report Objectives

The objectives of this report are to:

- establish the geological and hydrogeological conditions using existing available/published information;
- summarise available information and identify site specific geotechnical and environmental hazards which may place a constraint upon the proposed siteuse;
- produce an updated Conceptual Site Model identifying potential pollution linkages between sources of contamination, pathways and receptors;

1.3 Limitations

The recommendations made in this report are based on the findings of the intrusive ground investigation undertaken by HSP Consulting Ltd between 16th and 18th March 2015.

1.4 Previous Reports

A previous Phase I report has been made available to HSP Consulting Engineers Ltd by the Client.

• Thomas Consulting, Preliminary Risk Assessment Report, Land at Chatburn Road, Clitheroe, Ref: P4559-01-R1, October2013.



2. Review of Existing Information & Geoenvironmental Setting

2.1 The Site

2.1.1 Location

The site comprises two open fields to the north west of Chatburn Road, 1.5km north east of Clitheroe town centre at approximate National Grid reference (NGR) 375200,442990. The site address is Chatburn Road, Clitheroe, BB72EQ.

2.1.2 Description

The site rises from approximately 82.7m by the watercourse in the north west to approximately 92.7m along the south eastern boundary with Chatburn Road. A wall defines the south eastern boundary while to the north west and north east the site is bound by wooden fencing. The site boundary to the south west is marked by a line of trees and hedges.

Two stone walled pens are present, one in the southern corner of the site and another along the centre of the south eastern boundary. An unnamed watercourse flows north east to south west in the north of the site.

2.1.3 Surrounding Land Use

The main features of interest identified from the Phase I report and site walkover are:

North: Railway line with residential properties and the former Coplow Hill Lime Quarry

beyond.

East: Open fields and the watercourse which flows throughthe site. South: Chatburn Road with further fields and a hospitalbeyond.

West: Residential Housing.

2.1.4 Site Access

Vehicle access is off Chatburn Road in the southern corner of the site.

2.1.5 Proposed End Use

It is proposed to redevelop the site to provide twenty eight houses. The proposed development plan is provided in Appendix I.



2.2 Geology

2.2.1 Made Ground

The BGS mapping (Ref 2) indicates that Made Ground should not be encountered upon the site.

2.2.2 Superficial Deposits

The BGS mapping indicates that the site is underlain by glacial till of Devensian age. This is likely to comprise sandy gravelly clay with boulders.

2.2.3 Bedrock Geology

The BGS mapping indicates the site is underlain by the Clitheroe Limestone Formation and Hodder Mudstone Formation, described as 'Predominantly pale grey and commonly coarsely crinoidal, packstones, wackestones and subordinate grainstones and mudstones with Waulsortian mudmound reef limestones present at two levels' and 'redominantly grey to dark grey mudstone, with subordinate and variable detrital limestone, siltstone and sandstone. Mudmound reef (Waulsortian) limestones, limestone boulder conglomerates and breccias locally, near the base. Soft sediment deformation, slumps, debris flows and gravity slides are widespread.'

2.2.4 Structural Geology

One structural fault has been identified on BGS mapping 800m south west of the site. The fault trends north west to south east downthrown to the south west.

2.3 Pertinent Site SensitivityInformation

This information is provided in the Preliminary Risk Assessment Report by Thomas Consulting (Ref. 1) and summarised here for completeness.

2.3.1 Mining

The site is not in an area with potential for coal mining. One BGS mineral extraction site has been identified within 250m of the site relating to Coplow Limestone Quarry 145m north of the site. Extraction at the site is recorded as being ceased.

2.3.2 Hydrogeology

Aquifer Units

The Clitheroe Limestone Formation and Hodder Mudstone Formation at the site are designated as a Secondary A Aquifer strata described as permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers. These are generally aquifers formerly classified as minor aquifers.

Groundwater Vulnerability

The site is not located within a Source Protection Zone.



2.3.3 Hydrology

Nearest Surface Water Course

The closest recorded surface water course is the unnamed watercourse in the north of the site. The nearest off site water feature is a pond off Deanfield Way 280m east of the site.

2.3.4 Flood Risk

The site is recorded to be located within the influence of an Environment Agency Zone 2 and 3 floodplain in the north and west of the site.

Although the report provides information on flood risk this does not constitute a flood risk assessment for the site. The flood risk information provided only relates to flooding from Rivers or Seas and does not account for flooding from other sources such as groundwater, blockages in drainage systems, artificial water features and overlandflow.

2.3.5 Radon

The property is in a higher probability radon area, as between 10 and 30% of homes are above the action level. Full radon protective measures are necessary in the construction of new dwellings or extensions at the site.

2.3.6 Sensitive Land Uses, Ecological and Statutory Designations

The site is not located within a Nitrate Vulnerable Zone.

A Site of Special Scientific Interest (SSSI) is located 19m north of the site at Coplow Quarry. A second SSSI and local nature reserve are present 260m south east of the site at Salthill Quarry. A local nature reserve is also present 415m north of the site at Cross Hill Quarry. No other records of sensitive land use (SSSI, SAC, Environmentally Sensitive Areas, Local Nature Reserves etc) have been identified within 1km radius of the site.



3. Fieldwork & Factual Information

Site work was carried out between the 16th and 18th of March 2015. Where applicable, the fieldwork was undertaken in accordance with BS5930:1999 + A2:2010 Code of Practice for Site Investigations (Ref. 6) and BS 10175:2011 + A1:2013 Investigation of Potentially Contaminated Sites (Ref. 8).

The exploratory holes were positioned as close to the proposed building footprints as possible to provide information for foundation design and obtain representative soil samples for geotechnical and geo-environmental analysis. A number of trial pits were also excavated under the footprint of proposed roads in order to obtain samples for California Bearing Ratio (CBR) testing.

3.1 Exploratory Methods

The physical methods of investigation employed were 10No window sample boreholes to a maximum depth of 2.80m begl, 3No cable percussive boreholes to a maximum depth of 7.00m begl and 12No machine excavated trial pits to a maximum depth of 2.60m begl. The exploratory holes were logged and sampled by an Engineer from HSP Consulting Ltd and the logs are presented in Appendix II. The exploratory hole locations are shown on the Ground Investigation Layout Plan presented in AppendixIII.

Fragmentary bulk, disturbed and undisturbed samples were recovered from materials revealed within all of the exploratory holes. Geo-environmental samples, placed in plastic tubs and glass jars supplied by the laboratory, were also obtained specifically for chemical analysis. The samples were taken to UKAS accredited laboratories for further examination and testing.

3.2 In-situ Testing

3.2.1 Standard Penetration Tests

Standard Penetration Tests (SPT's) were carried out at 1.00m intervals to refusal within all of the window sample boreholes. In the cable percussive boreholes alternate SPTs and U100s at 1.00m intervals to 5.00m and at 1.50m intervals thereafter. The SPT's were undertaken in accordance with BS 1377:1990 and the results are included on the appended borehole logs (Appendix II).

3.3 Laboratory Testing

The laboratory testing schedules were prepared by HSP Consulting Ltd.

3.3.1 Geotechnical Testing

Geotechnical testing has been undertaken by a UKAS accredited laboratory as part of the works at the site:

- Particle Size Distribution
- Atterburg Limits
- One Dimensional Consolidation
- Triaxial Tests
- Natural Moisture Content
- Dry Density / Optimum Moisture Content relationshiptests
- Laboratory California Bearing Ratio (CBR) undisturbedtests



Sulphate Analysis

The laboratory testing has been carried out by Geolabs Limited (UKAS accredited, laboratory No.1982) in accordance with BS1377:1990 using calibrated equipment specifically for the British Standard.

3.3.2 Chemical Analysis

The geo-environmental samples retained specifically for chemical analysis were stored in cooled containers until delivery to the laboratory by courier.

Chemical analysis was scheduled on nine soil samples for the presence of a selected suite of potential contaminants as outlined in the tables below:

Exploratory Hole Location & Depth	Sample Description
WS1 0.50m	Clay'
WS2 0.50m	Clay ¹
WS3A 0.10m	Topsoil ¹
WS4 0.10m	Topsoil ¹
WS5 0.10m	Topsoil ¹
WS5 0.50m	Clay ¹
WS6 0.50m	Clay ¹
WS7 0.50m	Clay ¹
WS8 0.50m	Clay ¹

¹Geo-environmental Analysis Only

Metals	Cadmium	Chromium (III & VI)	Copper
	Lead	Mercury	Nickel
	Zinc		
Semi Metals and Non-metals	Arsenic	Boron	Selenium
Others	рН	Asbestos	
Inorganic Chemicals	Cyanide	Sulphate	Sulphide
Organic Chemicals	PAH (US EPA 16)	TPH (CWG)	Phenol

The contamination analysis was carried out by Chemtest Environmental Ltd (UKAS accredited, laboratory No. 2183) during the period 9th to 14th April 2015. The results are presented in Appendix V.

3.4 Ground Conditions

3.4.1 Published Geology

The published geology indicates the site is underlain by Till overlying the Clitheroe Limestone Formation and Hodder Mudstone formation as described in section 2.2.3above.

3.4.2 Ground Conditions on site or General Geology & Revealed Strata

The exploratory hole confirms the published information, the strata generally comprises:



Table 1 – Encountered Gro	ound Conditions
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Strata		Depth (mbegl)	Thickness (m)	Description
Anthropogenic and Topsoil	TOPSOIL	G.L - 0.30	0.30m	TOPSOIL comprising turf over sandy gravelly CLAY
	Strata	Depth (mbegl)	Thickness (m)	Description
		0.15 – 2.80	2.65m	Firm to stiff sandy gravelly CLAY with occasional cobbles and boulders
	Clitheroe Limestone Formation and Hodder Mudstone Formation	0.20 - 1.60	1.40m	Firm to stiff silty sandy CLAY
		0.25 – 1.30	1.05m	Clayey sandy GRAVEL & COBBLES of sandstone
Bedrock		1.00 - 2.50	1.50m	Clayey sandy GRAVEL of sandstone
Deditock		1.60 – 6.70	5.10m	Firm to stiff brown and grey CLAY with occasional cobbles and boulders
		3.40 - 5.30	1.90m	Stiff gravelly CLAY with cobbles and boulders
		5.30 – 5.80	0.50m	Stiff sandy CLAY
		4.10 – 7.00	2.90m	LIMESTONE

3.5 Groundwater Levels

Groundwater was encountered from a minimum depth of 1.00m during the drilling works.

Monitoring of the groundwater has been undertaken as part of this investigation. WS2 was proven to be dry on three occasions. Groundwater was proven in the remaining borehole installations at depths between 0.52m begl within CP1 and 4.16m begl in CP1.

3.6 Ground Gas Monitoring

Sources of potential ground gas were identified with the Preliminary Risk Assessment Report (Ref. 1). Gas monitoring installations were constructed within six of the boreholes at the site (CP1, CP2, CP3, WS2, WS6 and WS8). Each well has been constructed using 50mm diameter HDPE pipe with the top one metre being plain and the remainder slotted. All of the borehole installations have a 6mm pea gravel surround to the slotted pipe with a bentonite seal above and a gas tap. The covers are cemented flush with ground level and are either a round or square lockable stopcock cover.

HSP Consulting uses a GFM 430 Gas Analyser. Prior to its use a calibration check can be performed against gas readings in air. It is recommended that this check is undertaken once on each day the analyser is used. Annual calibration is undertaken on the unit and a copy of this certificate has been included within Appendix VI.

The results of the ground gas monitoring undertaken to date are discussed in Section 5.5 below.



4. Geotechnical Assessment

4.1 Detailed Ground Model

For the purposes of this assessment the trial pit logs, cable percussive and window sample borehole information has been utilised. The borehole logs are presented in Appendix II.

4.1.1 Topsoil

Topsoil was encountered in all exploratory locations across the site and generally comprised turf over sandy gravelly CLAY to a maximum depth of 0.30m begl.

4.1.2 Made Ground

Made Ground was not encountered at the site.

4.1.3 Clitheroe Limestone Formation and Hodder Mudstone Formation

Cohesive deposits of the Clitheroe Limestone Formation and Hodder Mudstone Formation was encountered in all exploratory hole locations from 0.15m to 6.70m begl. This generally comprised firm to soft silty sandy CLAY with cobbles and occasional boulders overlying stiff gravelly CLAY with occasional boulders to a maximum proven depth of 6.70m begl. Sandstone GRAVEL was encountered between 1.00m and 2.50m begl in CP3, TP5 and TP6.

The bedrock strata was generally recorded to comprise limestone and sandstone recovered as gravel and cobbles from a minimum depth of 0.25m begl in the south west of the site to a maximum proven depth of 7.00m in the east of the site. The base of the Clitheroe Limestone Formation and Hodder Mudstone Formation was not proven.

4.1.4 In-situ Testing and Assessment

A series of Standard Penetration Tests (SPT's) undertaken within the window sample boreholes have returned a SPT 'N' values in the range of 5 to 50 at 1.00m depth. The following table summarises the N values at depth across the site within the natural strata. The range of N values is provided for the site as awhole.

Table 2 - SPT N Values

Depth (m)	Range of 'N' Values	Mean 'N' Value	Description
1.00	5 - 50	13	Clay
2.00	7 - 50	18	Clay
3.00	42 - 50	37.5	Clay
3.80	50	50	Clay

Thirteen Plasticity Index Tests have been undertaken to confirm the visual description and engineering behaviour of the soils. The results are included in Appendix IV.

The plasticity index of the cohesive till deposits is in the range 11 to 24% indicating clays of low to high plasticity. The modified plasticity index of the cohesive soils are in the range 3% to 19% indicating soils of Low Volume Change Potential (VCP) in accordance with the NHBC guidance on building near trees (Ref. 9). The natural moisture content of the samples was in the range 10% to 26%.



Seven Particle Size Distribution and Natural Moisture Content tests have been undertaken to confirm the visual description and engineering behaviour of the soils. The results are included in Appendix IV.

4.2 Earthworks

Earthworks operations are expected at the site to allow vehicle access. Significant earthworks are not expected for the individual residential housing plots as the finished floor levels can be tailored to suit the contours of the site. The site levels drop from the road at the south eastern boundary to the watercourse in the north west of the site and levels will need to be altered along the alignment of the access road to allow the highway gradients to conform to adoptable standards.

The grading results indicate the Glacial Till, broadly classifies as general cohesive fill Class 2 in accordance with Highways Specification for Highways Works, Series 600, however the soils do not wholly conform to further subdivision due to the gravel and cobble sized fractions. These materials are unlikely to be suitable for use as engineered fill without the use of a ground improvement technique and possibly some sorting to reduce the number of oversized particles (i.e the cobbles and boulders observed during the site work. Consideration should be given to a scheme of lime stabilisation to reduce the moisture content and to aid in achieving high compaction criteria with respect to controlling long term settlements. Specific details should be discussed with a specialist contractor.

Stringent groundwater and surface water control will be imperative during excavation, as the cohesive deposits will rapidly soften upon contact with water. In addition these materials will be susceptible to softening during periods of wet weather and will easily be damaged by site traffic and deterioration at times of heavy rainfall. It can be seen from the appended compaction test results that the samples tested, in many cases had natural moisture contents above (by a maximum of 17%) the optimum moisture content. End product compaction criteria of 100% of the maximum dry density obtained in the 2.5kg tests will need to be achieved if these materials are to be used as fill to structures including the highway.

Given the alteration of the site levels it is recommended that in-situ CBR testing is carried out to determine the deign CBR values once the formation levels are exposed. CBR values in the order of 2% may be anticipated at formation providing any anomalously very soft or soft pockets are over excavated.

It is recommended that the following should be carried out with respect to hardstanding in areas of cut:

- The exposed formation should be carefully inspected and any undesirable materials such as topsoil, obstructions and hard spots should be removed and replaced with suitable granular hardcore.
- Any very soft / soft spots should be similarly over excavated and replaced.
- The formation should be proof rolled prior to construction which, in view of the cohesive nature of the soils encountered, should be undertaken as soon as possible to reduce the exposure time and the risk of softening, particularly in wet or frosty weather conditions. Where it is unavoidable, the softened upper layer should be scraped back prior to rolling and emplacement of the sub-base and /or capping.
- Consideration should be given to provision of a geofabric and / or geogrid stiffener between the formation soils and road, car park or hard landscaping construction



materials in order to prevent either punching of materials into any soft underlying soils, or the squeezing of the soils into the road construction during rolling. This would have the added benefit of providing a small degree of long term reinforcement and will lessen the effect of any differential settlement across areas of hardstanding spanning a variable subgrade.

It should be appreciated that the comments above are based upon a limited number of samples and should be treated as a preliminary basic guide only and not for detailed design work. A full earthworks appraisal will be required prior to the earthworks.

4.3 Excavations

Excavations to proposed formation level for new foundations and infrastructure should generally be readily achievable adopting standard excavation plant. However, random and potentially severe falls should be anticipated from the faces of near vertically sided unsupported excavations carried out at the site. Where personnel are required to enter near vertically sided excavations, it is considered that full support should be provided to the full depth of all excavations.

It is recommended that all support systems are continually assessed by fully trained or experienced personnel.

Groundwater was encountered from a minimum depth of 1.00m begl during the fieldwork, there is a possibility that groundwater entries may be encountered at shallow depths during construction. It should be noted that groundwater levels may vary due to seasonal variations or other effects. Should shallow groundwater entries be encountered at the site during groundwork operations traditional sump and pump dewatering should be sufficient if required.

4.4 Foundations

The development proposals for the site indicate twenty eight residential properties. The proposed development plans for the site can be seen in Appendix I. Should development plans alter a geotechnical engineer from HSP must be consulted to review the foundation options.

For the purpose of this foundation assessment the information gained from all window sample and cable percussive boreholes has been included.

The table below indicates the indicative allowable bearing pressure (ABP) that could be achieved using strip foundations across the building footprint. An ABP has been calculated using the mean of the corrected SPT $(N_1)_{60}$ values for the borehole group at 1m intervals from the existing ground level.

Table 3 – Indicative Allowable Bearing Pressures

Depth (m)	Mean SPT 'N160 ' Value	Eurocode 7 Soil Strength Description	Consistency (BS5930) Description	Approximate ABP (kN/m²) – 0.60m wide strip footing	Approximate ABP (kN/m²) – 2x2m pad footing
1.0	15.25	Medium Strength	-	125	130



2.0	21.11	High Strength	-	195	200
3.0	37.5	Very High Strength	-	415	450

The natural cohesive deposits belonging to the Clitheroe Limestone Formation and Hodder Mudstone Formation are considered as suitable a formation layer for the proposed houses where they have been encountered in a medium strength condition from a minimum depth of 0.50m (i.e. at least 200mm into the natural weathered bedrock deposits).

At the above depth HSP would recommend that an allowable bearing pressure of 125kNm² should be readily achievable when utilising a 0.60m wide strip trench footing.

The allowable bearing capacity value incorporates a factor of safety of 3 and total settlements are not expected to exceed approximately 25mm, thereby keeping differential settlements within acceptable limits.

Should higher loadings be required, consideration should be given to deepening the foundations to bear onto the very high strength cohesive deposits encountered from 3.00m begl.

4.5 Ground Floor Slab

Ground bearing floor slabs are considered to be a feasible option for the proposed development. In order to meet the requirements for radon protection these will need to be reinforced.

4.6 Concrete Classification

The results of sulphate and pH testing carried out on selected soil samples taken during this investigation have been compared with the recommendations outlined in BRE Special Digest 1, Part 1: 2005.

The guidelines given in BRE Special Digest 1 are based upon a site classification relating to its previous usage. It is considered appropriate to define this site as a 'greenfield site' location for the purposes of concrete classification.

On the basis of the above, it is considered appropriate to adopt a basic Design Sulphate Class of DS-1 together with and Aggressive Chemical Environment for Concrete (ACEC) of AC-1.

4.7 Pavement Design

At this stage the external proposals indicate a dedicated vehicular access from Chatburn Road heading to a turn head in the north of the site with smaller side roads for access to the residential dwellings.



Given the cohesive nature of the shallow soils encountered a CBR value of 3% to 3.5% is recommended for design purposes at this stage.

Consideration should be given to proof rolling the proposed building footprint and external areas once the formation level has been achieved as good practice. This is to target a CBR value of at least 5% throughout. Should any soft spots be encountered across the area they should be removed and replaced with suitably compacted stone or sub-base material.

Exposed subgrades will likely deteriorate rapidly on exposure to wet weather and should be shaped to shed water. Sub-base should be placed as soon as possible to minimise the exposure of the subgrade to adverse weather conditions.

4.8 Drainage

No soakaway or permeability testing was carried out on the underlying soils at the site at the time of this ground investigation.

The exploratory holes encountered cohesive deposits of low permeability across the site. The use of soakaway drainage is not considered feasible at thesite.



5. Environmental Assessment

5.1 Introduction

The approach to the human health risk assessment reported here follows the principals given in CRL 11, i.e. application of the following assessment hierarchy:

- Tier 1 risk screening by establishment of potential pollutant linkages, i.e. the preliminary conceptual site model (PCSM), or
- Tier 2 generic quantitative assessment using generic assessment criteria (GACs) that represent 'minimal' or 'tolerable' risk,or
- Tier 3 quantitative risk assessment using site specific assessment criteria (SSACs) that represent 'unacceptable risk', or where generic assessment criteria are not available or they are not applicable to the CSM.

The results of laboratory analysis have been screened against GACs including the Defra Category 4 Screening Levels (C4SL) and LQM and CIEH S4ULs for Human Health Risk Assessment (Copyright Land Quality Management Limited reproduced with permission; Publication Number S4UL3180. All rights reserved). (Refs 11 & 10 respectively).

The potential sources of contamination based on historical and current land uses were identified within the Preliminary Risk Assessment Report (Ref. 1). The standard exposure scenario of residential with plant uptake has been used to identify potential exposure pathways for human health receptors as the proposed development plan indicates rear gardens to the two. Controlled water, flora and fauna and property receptors have also been included within the CSM.

5.2 Assessment of Soil Analysis Results

Nine samples, as detailed in section 3.3.2, were scheduled for analysis from the development area. These provide a basis for characterising the soils to outline the potential impacts on human health and any environmental receptors from any contamination found.

The screening process for on-site human health receptors show that the GAC, representative of minimal risk for a residential setting was marginally exceeded for Arsenic and Benzo[b]fluoranthene in one location and Dibenz(a,h)Anthracene in two locations. The results for the remaining potential contaminants of concern were below the screening criteria for individual contaminant concentrations.

Table 4 – GAC Exceedances

Contaminant	GAC (mg/kg)	No. of exceedances	Concentration (mg/kg), sampling location and depth (m)
Arsenic	37 ¹	1	54 WS1, 0.50m
Benzo[b]fluoranthene	2.6 ²	1	3.5 WS5, 0.10m
Dibenz(a,h)Anthracene	0.24 ²	2	0.44 @ WS3A, 0.10m 0.43 @ WS5, 0.10m

¹C4SL, ²LQM & CIEH GAC

This investigation has identified the ground conditions to comprise a layer of Topsoil to a maximum depth of 0.20m (WS5) underlain by Clay to a maximum depth of 1.75m (WS1).



We therefore consider that there is the potential risk associated with the interaction between the near surface soils and end users of the site including construction workers. Mitigation measures and recommendations in relation to the contamination identified are made in Section 5.3 below.

5.3 Human Health Mitigation

Results from the nine geo-environmental samples have been screened for on-site human health receptors and none of the GACs (C4SLs or S4ULs) have been exceeded for any of the potential contaminants of concern with the exception of one elevated level of Arsenic in WS1 at 0.50m begl, one elevated level of Benzo[b]fluoranthene in WS5 at 0.10m begl and elevated levels of Dibenz(a,h)Anthracene in WS3A at 0.10m begl and WS5 at 0.10mbegl.

The exceedance of the C4SL for Arsenic in WS1 is a single exceedance at 0.50m depth within undisturbed natural ground it is unlikely to pose a significant possibility of significant harm to the proposed end users of the site unless ground levels in this area are reduced during development. Should the lowering of levels in this area be considered a geoenvironmental engineer from HSP must be consulted to review the mitigation options.

The elevated levels of Benzo[b]fluoranthene and Dibenz(a,h)Anthracene at 0.10m begl in WS3A and WS5 are within topsoil at the site. It is recommended that the topsoil in the vicinity of WS3A and WS5 is removed from site. This material will not be suitable for re-use in gardens. The concentrations of Benzo[b]fluoranthene and Dibenz(a,h)Anthracene recorded in these locations are not considered to pose a significant possibility of significant harm to the proposed end use of the site provided they are removed from site. If the material passes a 3AA2 test it may be suitable for reuse on a commercial site. Should any obvious evidence of unexpected contamination be encountered during the redevelopment works it should be reported to HSP so that an inspection can be made and appropriate sampling and assessment work be carried out

Appropriate health and safety precautions should be adopted during any excavation works to avoid exposure to contaminated soils and dust. Reference to the HSE document HSG 66 'Protection of workers and the General Public during Redevelopment of Contaminated Land'.

The approval of the local Environmental Health Officer should be sought with respect to the soil contamination assessment and mitigation proposals.

5.4 Water Supply

The environmental testing for the site has been compared to the following document in order to assess the most appropriate pipe material that should be used upon the site for mains water supply:

'Guidance for the selection of water supply pipes to be used in Brownfield sites – UK Water Industry Research – Ref: 10/WM/03/21.'

Based on the chemical analysis report it is considered that specialist materials are unlikely to be required for water supply pipes at the site. However confirmation of supply pipes should be sought from utility providers.



5.5 Ground Gas Risk Assessment

Sources of potential ground gas were identified within the Preliminary Risk Assessment Report. Ground gas concentrations have been monitored on six occasions over a three month period in order to obtain an indication of the ground gas regime at the site.

The results indicate that methane has not been recorded above the limits of detection of the gas monitor. Carbon dioxide has been recorded at concentrations up to a maximum 4.4% by volume in air. Positive gas flows have been recorded at levels up to 1.2l/hr.

The results have been assessed in line with the guidance provided in NHBC Guidance on Methane and Carbon Dioxide (Ref 14) and CIRIA Document C665 'Assessing Risks Posed by Hazardous Ground Gases to Buildings' (Ref 15.). Comparison of these results with Table 8.5 of the CIRIA document indicates that the site falls into a Characteristic Situation 1 and NHBC Green. Therefore gas protection measures are not necessary within any new developments upon the site with regards to methane and carbon dioxide, however full radon protection measures are required for the site.

The certificates and summary for the gas monitoring are included as Appendix VI.

5.6 Updated Conceptual Site Model

The PCSM and Summary of plausible pollutant linkages was produced by undertaking a Source-Pathway-Receptor analysis of the site and is present in the Preliminary Risk Assessment (Ref. 1). Based on the findings of this and the previous investigation the updated conceptual site model has been updated and is presented in the table below.



Table 5 - U	Ipdated	Conceptual	Site Mode	el.
-------------	---------	------------	-----------	-----

Source	Pathway	Receptor	Comments
On site	P1: Horizontal and vertical migration of contaminants through potentially permeable soils and rocks.	R1: Property, services and substructures	The glacial till and Clitheroe Limestone Formation and Hodder Mudstone Formation may contain sulphates that present a risk to buried concrete. Testing indicates the soils are unlikely to be aggressive to concrete and it is considered appropriate to adopt a basic Design Sulphate Class of DS-1 together with and Aggressive Chemical Environment for Concrete (ACEC) of AC-1. The chemical analysis of the soils indicate specialist materials are unlikely to be required for water supply pipes at the site.
\$1:Agricultural Land. On site	P2: Human uptake pathways	R2: Construction and maintenance workers	Elevated concentrations of Arsenic, Benzo[b]fluoranthene and Dibenz(a,h)Anthracene have been identified within the natural cohesive material and topsoil on site at 0.10 - 0.50m depth. Site workers should be advised of the potential for contact with the Made Ground materials. Appropriate health and safety precautions should be adopted during any excavation works to avoid exposure to contaminated soils and dust as per Section 5.3.
S2:Made Ground.	P2: Human uptake pathways	R3: End Users	Elevated concentrations of Arsenic have been identified within the natural cohesive material on site at 0.50m depth. Due to the depth of the contamination it is unlikely that the end users will come into contact with soils through leisure/sporting activities. Benzo[b]fluoranthene and Dibenz(a,h)Anthracene have been identified within the topsoil on site at 0.10m depth. This material is not suitable for reuse in gardens and should be removed from site. As a result it is unlikely that the end users will come into contact with soils through leisure/sporting activities. Therefore the risk is considered to be very low.



6. References

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- 2. BRITISH GEOLOGICAL SURVEY. 1975. Clitheroe. England and Wales Sheet 68. Bedrock and Superficial Deposits. 1:50 000 (Keyworth, Nottingham: British geological Survey).
- 3. British Geological Survey Lexicon Search http://www.bgs.ac.uk/lexicon/
- 4. Department of the Environment Industry Profiles.
- 5. Site Investigation in Construction, Volume 3, Specification for Ground Investigation 2nd Edition.
- 6. BS 5930:1999 + A2:2010 Code of Practice for Site Investigations.
- 7. BS 8576:2013 Guidance on investigations for ground gas. Permanent gases and Volatile Organic Compounds (VOCs)
- 8. BS 10175:2011 + A1:2013 Investigation of Potentially Contaminated Sites Code of Practice.
- 9. NHBC Standards, Chapter 4.2, Building near trees.
- 10. Nathanail, C.P., McCaffrey, C., Gillett, A.G., Ogden, R.C. and Nathanail, J.F. 2015. The LQM/CIEH S4ULs for Human Health Risk Assessment. Land Quality Press, Nottingham.
- 11. Department for Environment, Food and Rural Affairs and Contaminated Land: Applications in Real Environments (CL:AIRE) (December 2013). SP1010: Development of Category 4 Screening Levels for Assessment of Land Affected by Contamination.
- 12. BRE Special Digest 1:Concrete in Aggressive Ground, 2005, Building Research Establishment.
- 13. CL:AIRE The definition of Waste: Development Industry Code of Practice, 2008.
- 14. NHBC & RSK Group Plc, March 2007. Guidance on evaluation of development proposals on sites where methane and carbon dioxide are present. Ed 4.
- 15. CIRIA C665 'Assessing Risks Posed by Hazardous Ground Gases to Buildings'



Appendix I





Appendix II

		10							Borehole N	lo.
	5	þ				Bo	reho	ole Log	CP1	
con	sult	ing						J	Sheet 1 of	
Projec	t Name	Chatburn I	Road,		Project No.		Co-ords:	-	Hole Type	е
					C2099				CP	
Locati	on:	Clitheroe					Level:		Scale	
									1:50 Logged B	
Client:		Oakmere	Homes	;			Dates:	17/03/2015 -17/03/2015	Driller	y
		Sample	s and I	n Situ Testing	Depth	Level				
Well	Water Strikes	Depth (m)	Туре	Results	(m)	(m)	Legend	Stratum Description		
V (N)		0.00 - 0.20	В	results		, ,	.\{\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	Grass overyling brown clayey TOPS	OIL	
		0.10 0.20	D D		0.20		× × × × × × × × × × × × × × × × × × ×	Soft to firm orange brown mottled sil		
		0.20	В				<u>x</u> <u>x</u>	CLAY.		_
							×_×]
		1.00		50 (7,16/50 for						1 -
· 🗆 :		1.00 1.50	_B	270mm)				with a few large boulder and cobbles and	a little coarse	=
		1.00 - 1.50	В					gravel.]
		1.60 1.70 - 1.80	D U		1.60			Stiff dark grey CLAY.		
		1.80		50 (25 for 85mm/5	0					
		1.80 - 2.30	В	for 290mm)						2 —
: H:]		1.00 2.00								_
										=
		2.80		50 (9,12/50 for]
:		2.80 - 3.30	В	265mm)						3 —
							====			-
										=
		3.80		50 (25 for 95mm/5	n]
				for 275mm)						4 —
: H: :		3.80 - 4.10	В		4.10 4.20			LIMESTONE boulder. End of borehole at 4.20 m		
								End of borenote at 4.20 m]
										_
										_ =
										5 —
										=
										_
										-
										6 —
										_
										=
										7 _
										=
]
										_
										=
										8 —
										=
										=
]
										9 —
										=
]
										۱,, ۱

Remarks

. No groundwater was encountered during the drilling proces

Borehole was terminated at 4.20m due to refusal.

Gas and water monitoring standpipe installed to 4.20m depth.



		1							Borehole N	Ю.
M	5	þ				Во	reho	ole Log	CP2	
con	sult	ing						J	Sheet 1 of	
Projec	t Name:	Chatburn I	Road,		Project No. C2099		Co-ords:	-	Hole Type CP	е
Locati	on.	Clitheroe			02000		Level:		Scale	
Locali	011.	Ontrioroc					Lovoi.		1:50	
Client		Oakmere	Homes	5			Dates:	18/03/2015 -19/03/2015	Logged B Driller	У
Well	Water Strikes		1	In Situ Testing	Depth	Level	Legend	StratumDescription		
11 IN	Strikes	Depth (m)	Туре	Results	(m)	(m)	V////////			
		0.00 - 0.25 0.10	B D		0.25			Grass overlying silty TOPSOIL.		_ =
(2)		0.30 0.50 - 1.00	D B				×— ×	Ornage brown mottled silty sandy C	LAY.] =
		0.00								-
		1.00		N=19 (1,1/3,4,6,6)			×			1 -
		1.00 - 1.50	В				×x-			1 -
							×— -x-			-
		1.80	D		1.80		x	with sandstone gravel and occasional col	bbles.	=
		1.90 - 2.35	Ū				×	Firm to stiff brown grey mottled CLA	Υ.	-
										2 —
		2.40 2.60	D D		2.40 2.60			Firm to stiff brown grey mottled CLA	V	-
		2.70 2.70 - 3.20	В	N=42 (4,9/14,11,8,9				Grey brown very sandy CLAY with n		1 -
		2.70 - 3.20						and cobbles.	3	-
		3.40	D		3.40					3 -
		3.50 - 3.65	U		0.40] =
		3.50 - 3.80 3.80 - 4.20	B U					Stiff grey gravelly CLAY. Gravel is o	f limestone.	-
			_							
		4.20	D							4 -
										-
		4.80		50 (25 for 90mm/50)					-
		4.80 - 5.20	В	for 295mm)						-
		5.30 5.30	D	50 (25 for 85mm/50	5.30					5 —
. F:		0.00		for 245mm)	,			Grey sandy CLAY.		-
								with limestone gravels, boulder and cobb	loo	-
		5.30 - 5.50 5.30 - 5.80	D B		5.80			LIMESTONE	163.	-
		5.80	D		6.00			End of borehole at 6.00 m		6 _
		5.80		50 (25 for 90mm/50 for 250mm))					-
		6.00		50 (25 for 80mm/50)					-
				for 225mm)						-
										7 -
										-
										-
										-
										8 –
										=
										-
										=
										9 —
										=
										-
										10 —

Remarks

. No groundwater was encountered during the drilling proces

2. Borehole was terminated at 6.00m due to refusi

3. Gas and water monitoring standpipe installed to 4.20m depth



h		n							Borehole N	lo.
Щ	2	P				Bo	reho	ole Log	CP3	
con	sulti	ing						9	Sheet 1 of	
Projec	t Name:	Chatburn F	Road,		Project No.		Co-ords	: -	Hole Type	е
					C2099				CP Scale	
Locati	on:	Clitheroe					Level:		1:50	
Client:	:	Oakmere l	Homes	3			Dates:	18/03/2015 -18/03/2015	Logged B Driller	у
	14/-4	Samples	s and	In Situ Testing	Donath	Lavial			Dilliel	
	Water Strikes	Depth (m)	Туре	Results	Depth (m)	Level (m)	Legend	Stratum Description		
		0.00 - 0.20 0.10	B D		0.20			Grass overyling brown clayey TOPS		-
		0.20 0.20 - 0.70	D B				xx	Orange brown mottled silty sandy C	LAY.	_
		0.70	D		0.70		X————X			=
		0.70 - 1.00 0.80	B W		1.00		\times	Grey brown mottled silty sandy CLA gravel and cobbles.	Y with some	_
		1.00 1.10	D	N=41 (4,5/8,11,14,8	3]			Brown clayey GRAVEL.		1 -
		1.10 - 1.60	В							_
		1.70 1.80 - 2.20	D U		1.70] =
		2.20	_					Firm to stiff brown grey mottled CLA	Υ.	
		2.20 2.40	D D		2.40					2 —
		2.50 - 2.70 2.70	В	N=33 (4,7/6,8,8,11)			Stiff grey CLAY with many limestone	cobbles and	-
		2.70 - 3.15 2.70 - 3.20	D B					boulders.	, 00000100 and	=
\Box										3 —
Η.										3 –
							-2			=
		3.70 - 4.15	U				~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~			=
							- <u>-</u>			4 —
		4.20	D							-
							<u></u>]
		4.70		50 (25 for 80mm/50			<u> </u>			=
\Box		4.70 - 5.20	В	for 250mm)			~			5 —
\exists							~~~~			=
										3
		5.70 - 5.85	U							=
		5.70 - 6.20	В				<u> </u>			6 —
							<u> </u>			=
							~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~			]
		6.70		50 (25 for 135mm/5	0 6.70			LIMESTONE		
		6.70 - 6.80	D	for 228mm)	7.00			End of borehote at 7:00 m		7 _
		7.00		50 (25 for 85mm/50 for 225mm)	0					=
				,						-
										]
										8 —
										]
										-
										9 —
										10 —
Rema	rks			1		1	ı	ı		

undwater was encountered during the drilling process.

Borehole was terminated at 7.00m due to refusal.

Gas and water monitoring standpipe installed to 4.20m depth.

	C							Trialpit I	No
	5 P					Tri	ial Pit Log	TP1	I
con	sulting						_	Sheet 1	
Projec	t Chatburr	n Road,		Projec			Co-ords: -	Date	
Name	:			C2099	9		Level:	18/03/20	
Locati	on: Clitheroe	)					Dimensions (m):	Scale 1:25	
Client	: Oakmer	e Home	es				Depth 1.30	Logge LEB	d
	Sample	s and I	n Situ Testing	D (1	Lavial		1.50	LED	
Water Strike				Depth (m)	Level (m)	Legend	d Stratum Description		
Wat Strik	0.80 1.20	В	Results	(m) 0.25	(m)	Legell (1)	Grass overlying brown sandy clayey TOPSOIL plasticity.  Grey slightly clayey sandy GRAVEL & COBBLE and cobbles is fine to coarse angular to sub an sandstone.  End of pit af 1:30 m	S. Gravel	1   3   4   1   1   1   1   1   1   1   1   1
Remo	rks: 1 No.	groundw	ater was encountered du	ring the e	excavation	nroces			5—

2. Trial pit was terminated at 1.30m depth and backfilled with arisings.

								Trialpit N	VO
n	SD					Tri	al Pit Log	TP1E	
011	sulting						_	Sheet 1 c	of 1
Projec Name	t Chatburr	า Road,		Project C209			Co-ords: - Level:	Date 18/03/20	1.5
				C209	9		Dimensions	Scale	
Locati	on: Clitheroe	)					(m):	1:25	
Client	: Oakmere	Home	s				Depth 0.50	Logged LB	t
ter ke	Sample	s and I	n Situ Testing	Depth	Level	Legend	StratumDescription		
Water Strike	Depth	Туре	Results	(m)	(m)				
				0.15			Grass overlying blackish brown slightly gravelly sandy clayey TOPSOIL. High plasticity. Gravel medium angular to sub angular of sandstone.  Firm orange brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angular angular of sandstone and mudstone.  ———————————————————————————————————	is fine to CLAY.	2
Poma	rke: 1 No.	arounds	vater was encountered du	uring the c	drilling pr	2000			5— ——

2. Trial pit was terminated at 0.50m depth due to bedrock and backfilled with  $\,$  arisings.



								Trialpit N	10
n	SD					Tri	al Pit Log	TP1	
011	sulting						_	Sheet 1 c	of 1
Projec Name	ct Chatburr	n Road,		Projec			Co-ords: -	Date	
Name				C209	9		Level: Dimensions	18/03/201 Scale	
Locati	on: Clitheroe	)					(m):	1:25	
Client	: Oakmere	e Home	S				Depth	Logged	i
							0.50	LB	
Water Strike			n Situ Testing	Depth	Level	Legend	StratumDescription		
Str	Depth	Туре	Results	(m)	(m)				
				0.15			Grass overlying blackish brown slightly gravelly sandy clayey TOPSOIL. High plasticity. Gravel medium angular to sub angular of sandstone.  Firm orange brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angular angular of sandstone and mudstone.  End of pit at 0.50 m	is fine to CLAY.	1   3   4   1   1   1   1   1   1   1   1   1
Pema	rko: 4 No.	around.	ater seenade was encol	intered d	uring the	yeavetic	n process		5—

2. Trial pit was terminated at 0.50 m depth due to bedrock and backfilled with arisings.



	6 5							Trialpit I	No
	5 P					Tr	ial Pit Log	TP2	2
con	sulting						<b>O</b>	Sheet 1	of 1
Projec		n Road,		Projec			Co-ords: -	Date	
Name	: 			C2099	9		Level:	18/03/20	
Locati	on: Clitheroe	)					Dimensions (m):	Scale 1:25	
Client	: Oakmere	Home	e				Depth	Logge	
Cilerii							2.00	LEB	
Water Strike			n Situ Testing	Depth	Level	Legend	Stratum Description		
Wa	Depth	Туре	Results	(m)	(m)	N//AN///A	Cross quarking blooking brown and alouan	ODCOII	
N	0.80	В		2.00			Grass overlying blackish brown sandy clayey Low plasticity.  Firm orange brown slightly gravelly sandy CLAY plasticity. Gravel is fine to coarse angular to su of sandstone and mudstone.  HSV - 24 kPa at 0.50m depth.  HSV - 32 kPa at 1.50m depth.  End of pif at 2.00 m	7. High ab rounded	1   3   1   1   1   1   1   1   1   1
Remo	irke: 1 No.	aroundw	ater was encountered du	ring the d	rilling pr	oress.			5—

2. Trial pit was terminated at 2.00m depth and backfilled with arisings.

								Trialpit N	Ю
n	<b>5 D</b>					al Pit Log	TP3		
con	sulting						_	Sheet 1 c	of 1
Projec	ct Chatburr	n Road,		Projec			Co-ords: -	Date	
Name	): 			C2099	9		Level:	18/03/20	
Locat	ion: Clitheroe	)					Dimensions (m):	Scale 1:25	
Client	t: Oakmere	. Homo					Depth	Logged	i
Cilein							1.70	LEB	
ite ke	Sample	s and I	n Situ Testing	Depth	Level	Legend	StratumDescription		
Water Strike	Depth	Туре	Results	(m)	(m)				
							Grass overlying blackish brown sandy clayey T High plasticity.		=
				0.15		V//AV//A	Firm orangish brown sandy gravelly CLAY. High	)	=
							plasticity. Gravel is fine to coarse angular to sub of sandstone and mudstone.	rounded	=
	0.50	В							-
	0.50	Ь							3
							HSV - 48 kPa at 0.60m depth.		_
									=
									=
									1-
									-
									-
							with low sub rounded boulders of sandstone at 1.40m dep	th.	
	1.50	В					HSV - 14 kPa at 1.50m depth.		=
				1.70			End of pit at 1.70 m		-
							End of picac 1.70 III		-
									=
									2—
									=
									Ξ
									3
									-
									=
									=
									=
									3-
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									=
									=
									-
									=
									4-
									-
									=
									=
									=
									=
									-
									5-
Rema	rks: 1. Slia	ht aroun	dwater seepage was en	countered	during th	e excava	tion process at 1.30m depth.	<u> </u>	

2. Trial pit was terminated at 1.70m depth and backfilled with arisings.

	6 6							Trialpit I	Vο
	5 P					Tr	ial Pit Log	TP4	
con	sulting						•	Sheet 1	of 1
Projec Name:		n Road,		Projec			Co-ords: -	Date	
Name.				C2099	9		Level:	18/03/20	
Location	on: Clitheroe	:					Dimensions (m):	Scale 1:25	
Client	: Oakmere	Home	S				Depth 1.90	Logge	
<b>-</b>	Sample	s and li	n Situ Testing	Dth	11		1.90	LB	
Water Strike	Depth	Туре	Results	Depth (m)	Level (m)	Legend	d Stratum Description		
ĭS ×	0.60	В	Results	1.90			Grass overlying blackish brown sandy gravelly TOPSOIL. High plasticity. Gravel is fine to med angular to sub angular of sandstone.  Firm orangish brown sandy gravelly CLAY. High plasticity. Gravel is fine to coarse angular to sub of sandstone and mudstone.  HSV - 38 kPa at 0.50m depth. with a medium sub rounded boulder of sandstone.  HSV - 22 kPa at 1.40m.  End of pit at 1.90 m	lium n	2
Rema	rks: 1. No g	groundwa	ater was encountered d	uring the d	rilling pro	ocess.			5—

2. Trial pit was terminated at 2.00m depth and backfilled with arisings.

	c n							Trialpit I	No
	5 P					Tr	ial Pit Log	TP5	;
con	sulting						•	Sheet 1	of 1
Projec Name		n Road,		Projec			Co-ords: -	Date	
INAIIIE	•			C2099	9		Level: Dimensions	18/03/20 Scale	
Locati	on: Clitheroe	)					(m):	1:25	
Client	: Oakmere	Home	S				Depth	Logge	
							2.10	LB	
ater			n Situ Testing	Depth (m)	Level (m)	Legend	Stratum Description		
Water	0.50  1.60 1.90	B B	Results	1.90 2.10	(m)	Legence	Grass overlying blackish brown sandy gravelly TOPSOIL. High plasticity. Gravel is fine to med angular to sub angular of sandstone.  Firm orange brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angular rounded of sandstone and mudstone.  ### ### ### ### ### ### ### ### ### #	CLAY. to sub	1   1   1   1   1   1   1   1   1   1
Rema	rks: 1. No g	groundw	ater was encountered o	during the e	excavatio	n process	<b>5</b> .		5—

2. Trial pit was terminated at 2.10m depth and backfilled with arisings.

6	C							Trialpit N	10
	<b>S D</b>					Tri	ial Pit Log	TP6	
con	sulting						•	Sheet 1 o	of 1
Projec Name		n Road,		Projec			Co-ords: -	Date	
varrie	•			C2099	)		Level: Dimensions	18/03/201 Scale	
_ocati	on: Clitheroe	)					(m):	1:25	
Client	: Oakmere	Home	S				Depth	Logged	1
			n Situ Testing				2.50	LB	
Water Strike	Depth	Туре	Results	Depth (m)	Level (m)	Legend			
•	0.50	В		1.00			Grass overlying blackish brown sandy gravelly TOPSOIL. High plasticity. Gravel is fine to med angular to sub angular of sandstone.  Firm orange brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angular angular ofsandstone.  ### ### ### ########################	CLAY. to sub	1
	2.20	В		2.10		Ž	Stiff dark grey silty sandy gravelly CLAY. Low possible to coarse angular to sub rounded	plasticity.	2—
Remaak	rks: 1. No c	groundw	ater was encountered d	2.30 2.50	xcavatio	n process	Sandstone.  Dark grey sandy very clayey GRAVEL. Gravel is coarse angular to sub rounded of sandstone.  End of pit at 2.50 m	s fine to	3

2. Trial pit was terminated at 2.50m depth and backfilled with arisings.

								Trialpit N	No
n	SD					al Pit Log	TP7		
	sulting						_	Sheet 1 o	of 1
Proje Name		n Road,		Projec			Co-ords: -	Date	
INAIIIE	;. 			C209	9		Level: Dimensions	18/03/20 ⁻ Scale	
Locat	ion: Clitheroe	)					(m):	1:25	
Clien	t: Oakmere	e Home	s				Depth	Logged	
011011	T			ı	1		1.10	LB	
Water Strike	Sample		n Situ Testing	Depth (m)	Level (m)	Legend	Stratum Description		
Wat	Depth 0.50	B	Results	0.30 1.10	(m)	Legellu	Grass overlying blackish brown slightly gravelly sandy clayey TOPSOIL. High plasticity. Gravel coarse sub angular to sub rounded of sandston  Firm light grey to orangish brown slightly grave sandy CLAY. High plasticity. Gravel is fine to me angular to sub angular of sandstone and mudstems.  HSV - 32 kPa at 0.60m depth. becoming softer and very garvelly from 0.70m depth.  End of pit at 1.10 m  End of pit at 1.10 m	is fine to ne. Ily very edium	1   3   4   1   1   1   1   1   1   1   1   1
Rema	arks: 1. Slial	ht groun	dwater seepage was en	countered	during th	e excava	tion process at 0.70m depth.		5—

2. Trial pit was terminated at 1.10m depth due to collapsing and backfilled with arisings.

	CD							Trialpit I	No
Ш	5 P					Tri	al Pit Log	TP8	
con	sulting						<u> </u>	Sheet 1 o	of 1
Projec Name		n Road,		Project C209			Co-ords: - Level:	Date 18/03/20	
_ocati	on: Clitheroe			10200			Dimensions	Scale	
_UCall	on. Cittieroe	<del>,</del>					(m): Depth	1:25	
Client	: Oakmere	e Homes					2.50	Logged LB	a
er ke	Sample	s and In S	itu Testing	Depth	Level	Legend	Stratum Description		
Water Strike	Depth	Туре	Results	(m)	(m)	Legend			
•	0.50	В		0.25			Grass overlying blackish brown slightly gravel sandy clayey TOPSOIL. High plasticity. Grave coarse sub angular to sub rounded of sandsto Firm greyish brown slightly gravelly very sand High plasticity. Gravel is fine to coarse angula rounded of sandstone and mudstone.  HSV - 48 kPa at 0.50m depth. with medium angular to sub rounded sandstone gravel a depth.	el is fine to one. y CLAY. r to sub	
	1.50	В					HSV - 38 kPa at 1.30m depth. becoming softer with increased depth.		
	2.40	В		2.50			End of pit at 2:50 m		2_
									3
									4
									5_
Rema	rks: 1. Slig	ht groundw	ater seepage was e	encountered	during th	ne excava	tion process at 0.80m depth.		_

2. Trial pit was terminated at 2.50m depth and backfilled with arisings.



	CD							Trialpit	No
Ш	5 P				TP9				
con	sulting						al Pit Log	Sheet 1	of 1
Projec Name		n Road,		Projec			Co-ords: -	Date	
				C2099	9		Level: Dimensions	18/03/20 Scale	
Locati	on: Clitheroe	Э					(m):	1:25	
Client	: Oakmer	e Homes					Depth 2.50	Logge LB	d
<u>-</u> 0	Sample	es and In S	Situ Testing	Depth	Level				
Water Strike	Depth	Туре	Results	(m)	(m)	Legend	StratumDescription		
•	0.60	В		0.20			Grass overlying blackish brown slightly gravel sandy clayey TOPSOIL. High plasticity. Grave medium angular to sub angular of sandstone.  Firm greyish brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angula angular of sandstone and mudstone.  HSV - 61 kPa at 0.40m depth. becoming very gravelly at 0.70m depth.	el is fine to CLAY.	1 —
	1.50	В		1.20			Firm dark grey slightly sandy gravelly CLAY. I plasticity. Gravel is fine to coarse angular to st of sandstone.  HSV - 28 kPa at 1.40m depthwith a low sub rounded boulder content of sandstone at depth.	ub angular	2_
	2.20	В		2.50			End of pit at 2:50 m		
									3-
									4-
									]
Rema	rko: 1 Clia	ht ground	ater seeness was a	ncountared	during #	00 0V00V0	tion process at 1.00m depth.		5-
CIIId	ino. I. oliy	, groundw	ator ocepage was e		adming ti	io unuava	aon process at 1.00m acptil.		- 10

2. Trial pit was terminated at 2.50m depth and backfilled with arisings.

6	2							Trialpit N	٧o
	5 P					Tr	ial Pit Log	TP10	
con	sulting						9	Sheet 1 o	of 1
Projec	ct Chatburr	n Road,		Projec			Co-ords: -	Date	
Name	): 			C2099	9		Level:	18/03/20	
Locati	ion: Clitheroe	)					Dimensions (m):	Scale 1:25	
Client	: Oakmere	e Home	S				Depth	Logged	
							2.60	LB	
Water Strike	Depth	Type	n Situ Testing Results	Depth (m)	Level (m)	Legend	Stratum Description		
<u> </u>				0.25			Grass overlying blackish brown slightly gravelly sandy clayey TOPSOIL. High plasticity. Gravel medium angular to sub angular of sandstone.  Firm orangish brown slightly gravelly very sand High plasticity. Gravel is fine to coarse angular rounded of sandstone.  HSV - 52 kPa at 0.50m depth.	is fine to by CLAY.	
	0.60	В					with medium angular to sub rounded content of sandstone	∋ at 0.80m	-
							with low angular to sub rounded content of sandstone at a depth.		1—
	1.50	В					HSV - 40 kPa at 1.40m depth.		2
	2.40	В		2.60			End of pit at 2:60 m		
									3_
									4
Rema	irks: 1. No i	groundw	ater was encountered du	ring the e	excavation	n process	5.		5—

2. Trial pit was terminated at 2.60m depth and backfilled with arisings.

	n				_	_		Borehole No.
11 2	P				Boı	reho	ole Log	WS1
consult	ing						0	Sheet 1 of 1
Project Name	: Chatburn	Road,		Project No. C2099		Co-ords:	-	Hole Type WS
Location:	Clitheroe					Level:		Scale 1:50
Client:	Oakmere	 Homes	 3			Dates:	17/04/2015 - 17/04/2015	Logged By LEB
Water Water	Sample	s and	In Situ Testing	Depth	Level			
Well Strikes		Туре		(m)	(m)	Legend	StratumDescription	
	0.15	D		0.15			MADE GROUND - blackish brown v clayey TOPSOIL. Low plasticity.	· / -
	0.50	D					Firm orangish brown slightly gravelly CLAY. High plasticity. Gravel is fine	to coarse –
							subangular to angular of sandstone  HSV - 46 kPa at 0.70m depth.	.   =
	1.00		N=12 (0,0/1,2,3,6)	)				1 _
	1.50	D						=
	1.75	ES		1.75 1.80			Grey SANDSTONE. Recovered as	a coarse -
							angular gravel.  End of borehole at 1.80 m	2
								-
								- - -
								3 —
								=
								4_
								• =
								-
								5 —
								=
								=
								6
								=
								7
								-
								- - -
								8 _
								=
								9
								-
								-
Remarks								10 —
· ·omanio								1

1. No groundwater was encountered during the drilling process.

2. Borehole was terminated at 1.80m depth due to refusal and backfilled with arisings.



		<b>1</b> 0							Borehole N	lo.
	5	p				ole Log	WS2			
con	sult	ing							Sheet 1 of	
Projec	t Name	Chatburn I	Road,		Project No.		Co-ords:	: <del>-</del>	Hole Type	е
					C2099				WS Scale	
Locati	ion:	Clitheroe					Level:		1:50	
01:		0-1					Datas	47/04/0045 47/04/0045	Logged B	у
Client		Oakmere I	Homes	3			Dates:	17/04/2015 -17/04/2015	LEB	
<b>147-11</b>	Water	Samples	and I	n Situ Testing	Depth	Level	Legend	StratumDescription		
Well	Strikes	Depth (m)	Туре	Results	(m)	(m)	Logona	·		
12		0.10	D		0.20			Grass overlying blackish brown very clayey TOPSOIL. Lowplasticity.	y sandy	-
		0.50	_		0.20			Firm orangish brown slightly gravelly	very sandy	=
		0.50	D					CLAY. High plasticity. Gravel is fine angular to angular of sandstone.	to coarse sub	]
		4.00		N 40/4 4/0 0 0 0				HSV - 42 kPa at 0.80m depth.		
		1.00		N=10 (1,1/2,2,3,3)	'					1 —
		4 = 0								=
		1.50	D							-
								HSV - 62 kPa at 1.80m depth		
		2.00		N=16 (2,3/4,4,4,4)	2.10			Firm greyish brown sandy gravelly (	CLAY. High	2 —
								plasticity. Gravel is fine to coarse su angular of sandstone and mudstone	ub rounded to	]
		2.50 2.50	D	50 (25 for 105mm/5	50			HSV - 52 kPa at 2.40m depth.		-
				`for 255mm)	2.80		, 1 <del>- 1   1 - 2</del>   1 - 2	End of borehole at 2.80 m		
										3 —
										]
										=
										=
										4 _
										]
										_
										=
										5 —
										=
										=
										_
										6 —
										_
										=
										=
										7 –
										=
										_
										8 –
										=
										=
										]
										9 _
										=
										10 _

. No groundwater was encountered during the drilling process.

Borehole was terminated at 2.50m depth due to refusal.
 Gas and water monitoring standpipe installed to 2.50m depth.



0	C	n							Borehole N	lo.
- T	2	P				Boi	reho	ole Log	WS3	
con	sult	ing			Project No.		1		Sheet 1 of Hole Type	
Projec	ct Name:	Chatburn I	Road,		C2099		Co-ords:	: <del>-</del>	WS	5
Locat	ion:	Clitheroe					Level:		Scale	
Client	:	Oakmere	Homes	S			Dates:	17/04/2015 -17/04/2015	1:50 Logged B	у
\A/~!I	Water	Sample	s and	In Situ Testing	Depth	Level	1	Ctuatura Dan aviation		
vveii	Strikes	Depth (m)	Туре	Results	(m)	(m)	Legena			
					(m) 0.20		Legend	Grass overlying blackish brown ver TOPSOIL. Lowplasticity.  Firm light grey and oranish brown sa CLAY. High plasticity. Gravel is fine angular to rounded of sandstone.  HSV - 44 kPa at 0.50m depth.  becoming soft from 1.10m depth. with a cobble of sandstone.  End of borehole at 1.80 m	indy gravelly	3
										7
										10 —

- Remarks

  1. No groundwater was encountered during the drilling process.
- 2. Borehole was terminated at 1.80m depth due to refusal and backfilled with arisings.



	C	n							Borehole No.
	2	Ρ				Boi	reho	ole Log	WS3A
	sult et Name		Road,		Project No.		Co-ords		Sheet 1 of 1 Hole Type
		Olivi.			C2099				WS Scale
Locat	ion:	Clitheroe					Level:		1:50
Client	:	Oakmere I				T	Dates:	17/04/2015 - 17/04/2015	Logged By LEB
Well	Water Strikes	-	1	In Situ Testing	Depth (m)	Level (m)	Legend	StratumDescription	
	Strikes	Doptii (iii)	Туре	Results		(111)	XXXXXXXXX	Grass overlying blackish brown san	dy clayey
		0.10 0.50 1.00 1.40 1.60	D D	N=16 (1,1/3,4,5,4 50 (25 for 135mm/ for 235mm)				TOPSOIL. Highplasticity.  Firm light grey and oragnish brown gravelly sandy CLAY. High plasticity to medium sub rounded to angular and mudstone. with a cobble of sandstone at 1.50m depth.  Extremely weak grey medium grain	slightly y. Gravel is fine of sandstone
				,				SANDSTONE recovered as angula gravel.  End of borehole at 1.70 m	coarse
									3-
									4-
									5-
									6-
									7-
									8-
									9-

- 1. No groundwater was encountered during the drilling process.
- 2. Borehole was terminated at 1.70m depth due to refusal and backfilled with arisings.



	C	n				_	_	_	Borehole N	lo.
	2	P				Boi	rehe	ole Log	WS4	
con	sulti	ing			D				Sheet 1 of	
Projec	ct Name:	Chatburn I	Road,		Project No. C2099		Co-ords	: -	Hole Type WS	е
Locat	ion:	Clitheroe					Level:		Scale	
Locat									1:50 Logged	
Client	:	Oakmere I	Homes	<b>.</b>			Dates:	17/04/2015 - 17/04/2015	LEB	
Well	Water Strikes			In Situ Testing	Depth	Level (m)	Legend	StratumDescription	ı	
V/834/	Strikes	Depth (m)	Туре	Results	(m)	(111)	XVXXXXXXX	Grass overlying blackish brown slig	ihtly gravelly	
		0.10	D		0.20			sandy clay TOSPOIL. High plasticit fine to medium angular to sub angu	ty. Gravel is	
		0.50	D					sandstone Firm orangish brown slightly gravelly	y sandy	-
		1.00		N=5 (1,1/1,2,1,1)				CLAY. High plasticity. Gravel is fine angular to sub angular of sandstone	e to medium	1 _
				(1, 1, 1, 2, 1, 1, 1)				mudstone.  HSV - 44 kPa at 0.80m depth.		_
		1.50	D							_
										=
		2.00		N=31 (4,6/5,7,8,11)						2 —
		2.50	D		2.30		×_×_	Stiff dark grey silty sandy gravelly C plasticity. Gravel is fine to coarse a	CLAY. Low	- -
D.C.		2.70		35 (25 for 115mm/3 for 245mm)	2.70		-×-	rounded of sandstone and mudstor	ne.	=
				101 245(11111)				End of borehole at 2.70 m	<i>-</i>	3 -
										_ _ _
										_ _
										4_
										-
										_
										5_
										) — -
										_
										-
										6 _
										_
										7_
										-
										_
										8 _
										_
										-
										_
										9
										10 _

- 1. No groundwater was encountered during the drilling process.
- 2. Borehole was terminated at 2.70m depth due to refusal and backfilled with arisings.



	C	<b>n</b>							Borehole No.
	2	Ρ				Boi	reho	ole Log	WS5
con	sult	ing						0	Sheet 1 of 1
Project	t Name:	: Chatburn I	Road,		Project No. C2099		Co-ords:	-	Hole Type WS
Location	on:	Clitheroe					Level:		Scale 1:50
Client:		Oakmere I	Home	<del></del>			Dates:	17/04/2015 - 17/04/2015	Logged By LB
Well	Water Strikes	Samples		In Situ Testing	Depth	Level	Legend	StratumDescription	
vveii	Strikes	Depth (m)	Туре	Results	(m)	(m)		Grass overlying brown slightly sand	
		0.10	D		0.20			very clayey TOPSOIL. High plastici fine to medium angular to sub angu	ty. Gravel is
		0.50	D					sandstone. Firm yellowish brown and grey slight very sandy CLAY. High plasticity. Gr	tly gravelly
		1.00		N=5 (3,2/2,1,1,1)				coarse angular to sub rounded ofsa	andstone and 1 _
		1.50	D					HSV - 19 kPa at 0.90m depthbecoming soft from 1.20m depth.	
					1.70			End of borehole at 1.70 m	
		2.00		50 (25 for 135mm/5 for 275mm)	50				2 –
									-
									3 –
									-
									4 –
									5 –
									-
									6 -
									-
									7 –
									-
									8 –
									9 -
									-
Remar	ks		<u> </u>						10 –

- 1. Groundwater was encountered during the drilling process at 1.10m depth.
- 2. Borehole was terminated at 1.70m depth due to refusal and backfilled with arisings.



hsp					Bo	 reh	ole Log	Borehole N		
con	sult	ing				יטט	CII	JIE LUG	Sheet 1 of	
	ct Name		Road,		Project No.		Co-ords:	: -	Hole Type	
					C2099		<del> </del>		WS Scale	
Locat	ion:	Clitheroe					Level:		1:50	
Client	ï	Oakmere I	Homes	;			Dates:	17/04/2015 -17/04/2015	Logged B LB	У
Well	Water	-	and I	In Situ Testing	Depth	Level	Legend	StratumDescription		
VVEII	Strikes	Boptii (iii)	Туре	Results	(m)	(m)	-09	Grass overlying blackish brown slig		
		0.10 0.50	D D		0.15			sandy clayey TOPSOIL. High plasti fine to medium sub angular of sand mudstone.	city. Gravel is	
		1.00		N=14 (1,2/2,3,4,5)				Soft light grey to orangish brown sli very sandy CLAY. High plasticity. G coarse angular to sub angular ofsa	Gravel is fine to	1 —
		1.50	D		1.20			mudstone.  HSV - 32 kPa at 0.60m depth.  Dark grey gravelly clayey SAND. So coarse. Gravel is fine to coarse and	jular to sub	
	6	1.90		50 (25 for 85mm/24,12,14,)	1.90			angular of sandstone and mudstone	e. 	2_
										3 4 5 6 7 8 9
										10 —

. Groundwater was encountered during the drilling process at 1.00m dept

2. Borehole was terminated at 1.90m depth due to refusa

3. Gas and water monitoring standpipe installed to 1.90m depth.



0	C	n				_	_		Borehole N	10.
76. Th	onsulting					Bo	reho	ole Log	WS7	
					Project No.				Sheet 1 of 1 Hole Type	
Projec	t Name	: Chatburn	Road,		C2099		Co-ords:	: -	WS	C
_ocat	ion:	Clitheroe					Level:		Scale 1:50	
Client	:	Oakmere	Homes	3			Dates:	17/04/2015 -17/04/2015	Logged B	У
	Water	Samples	s and I	In Situ Testing	Depth	Level	Logond	Ctratum Depariation		
Well	Strikes	Depth (m)	Туре	Results	(m)	(m)	Legend	-		
		0.10	D		0.25			Grass overlying blackish brown slig sandy clayey TOPSOIL. High plast	icity. Gravel is	] =
		0.50	D					fine to medium sub angular of sand mudstone.	1	-
								Firm light brown and orangish brown gravelly CLAY. High plasticity. Grave	el is fine to	-
		1.00		N=10 (3,3/4,2,2,2)				medium angular to sub rounded of and mudstone.  HSV - 42 κPa at 0.70m depth.	sandstone	1 _
		1.50	D		1.50			Soft reddish brown slightly gravelly	vory condy	=
								CLAY. High plasticity. Gravel is fine angular to sub angular of sandston	e to coarse	-
		2.00		N=7 (2,2/1,2,1,3)				mudstone.  HSV - 24 kPa at 1.60m depth.	e and	2 _
										-
		2.70		N=50 (11,12/50 for	2.70			End of borehole at 2.70 m		-
				285mm)						3 —
										-
										-
										4 _
										-
										-
										5 —
										-
										-
										6 -
										-
										=
										7 _
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										-
										8 —
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										_ =
										9 —
										=
										=
										10 _

Remarks
1. Groundwater was encountered during the drilling process at 1.40m depth.

2. Borehole was terminated at 2.70m depth due to refusal and backfilled with arisings.



	C	n				_	_		Borehole No.		
700.00	)	Ρ				Bo	ole Log	WS8			
con	sult	ing			D			<u> </u>	Sheet 1 of 1 Hole Type		
Projec	t Name:	: Chatburn I	Road,		Project No. C2099		Co-ords:	-	WS		
Locati	ion:	Clitheroe					Level:		Scale		
							20101.		1:50 Logged By		
Client		Oakmere I					Dates:	17/04/2015 -17/04/2015	LB	,	
Well	Water Strikes			In Situ Testing	Depth (m)	Level (m)	Legend	StratumDescription			
	Strikes	Depth (m)	Туре	Results		(111)	XX(1XX(1)	Grass overlying brown sandy grave	elly clayey	_	
					0.20			TOPSOIL. High plasticity. Gravel is medium angular to sub rounded of	fine to	-	
		0.50	D					and mudstone.  Firm orangish brown slightly gravelly	y very sandy	_	
		1.00		N=5 (1,1/1,1,1,2)				CLAY. High plasticity. Gravel is fine t	o coarse	1 —	
		1.00		(1,1,1,1,1,2)				angular to sub rounded of sandston HSV - 68 kPa at 0.70m depth.	-	' =	
										-	
		1.80		N=50 (11,14/50 fo	r 1.80			End of borehole at 1.80 m			
				285mm)						2 —	
										_	
										-	
										3 —	
										_	
										_	
										4 _	
										· -	
										_	
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										6 _	
										-	
										_	
										7 —	
										-	
										_	
										-	
										8 —	
										9 _	
										10 —	
Rema	rks			Ĺ							

- 1. No groundwater was encountered during the drilling process.
- 2. Borehole was terminated at 1.80m depth due to refusal and backfilled with arisings.



	C	n						_	Borehole No	0.	
	2	P				Boi	ole Log	WS9			
con	sulti	ing						3	Sheet 1 of		
Project	t Name:	Chatburn F	₹oad,		Project No. C2099		Co-ords:	-	Hole Type WS	!	
Location	on:	Clitheroe			02000		Level:		Scale		
Localic	JII.						Level.		1:50		
Client:		Oakmere I	Homes	i			Dates:	17/04/2015 -17/04/2015	Logged By LB	,	
Well	Water			n Situ Testing	Depth (m)	Level	Legend	StratumDescription	ı		
Well	Water Strikes -	Depth (m)  0.50  1.00  2.00	Type  D		(m) 0.30 ) 1.20	Level (m)	Legend	Grass overlying brown sandy grave TOPSOIL. High plasticity. Gravel is medium angular to sub angular of smudstone.  Firm orangish brown slightly gravelly CLAY. High plasticity. Gravel is fine tangular to sub rounded of sandston mudstone.  HSV-42 kPa at 0.80m depth.  Firm greenish brown silty sandy gravelly gravelly gravelly to coasub angular of sandstone.  HSV-79 kPa at 1.50m depth.  End of borehole at 2.00m.	elly clayey fine to sandstone and y very sandy o coarse e and avelly CLAY.	1 2 3 4 5 6 7 7 9 9	
										10 —	
Remar	ks					I					

- 1. No groundwater was encountered during the drilling process.
- 2. Borehole was terminated at 2.0m depth due to refusal and backfilled with arisings.



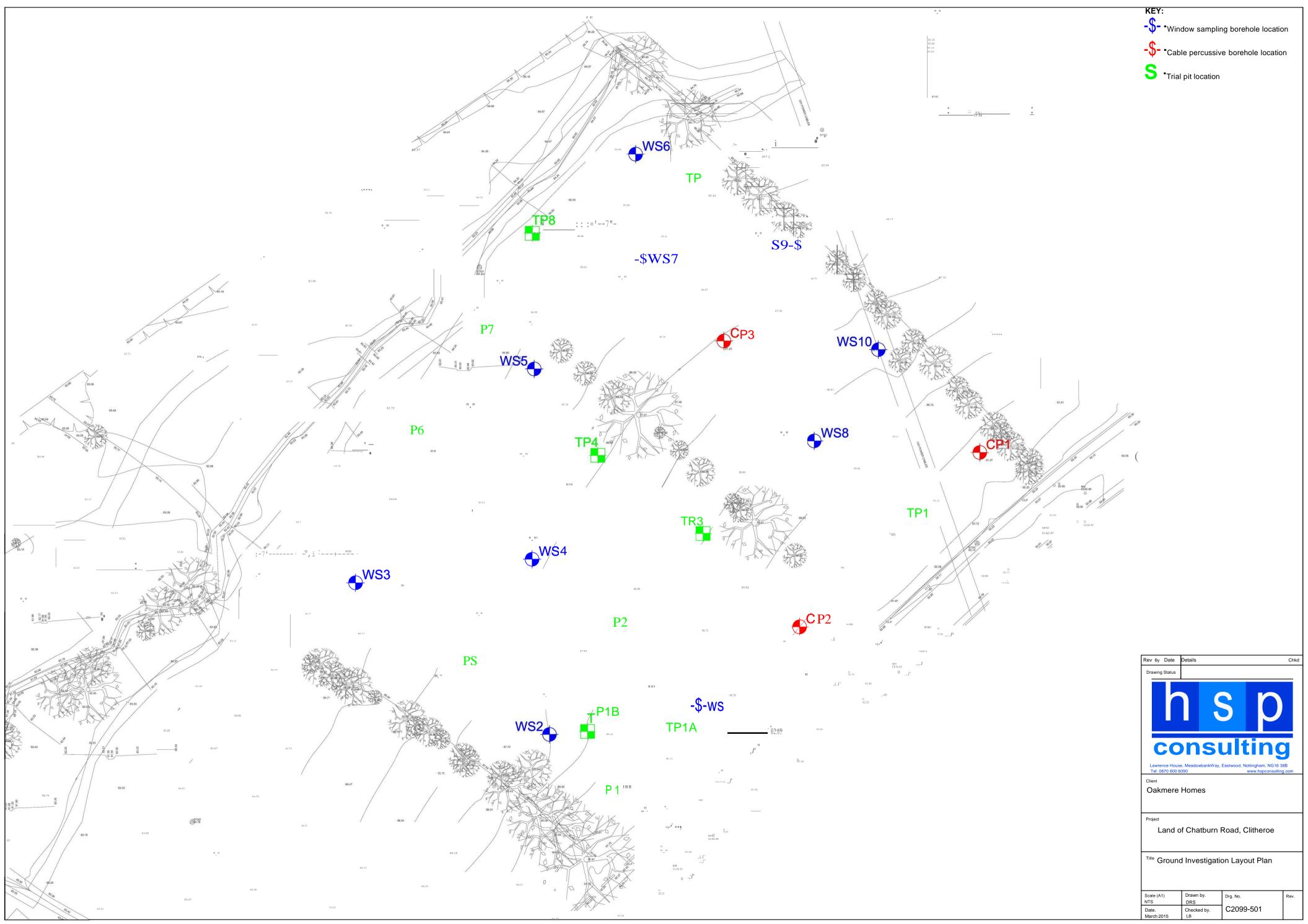
	C	n							Borehole N	lo.	
	5	þ				ole Log	WS10	)			
con	sult	ing			<b>.</b>				Sheet 1 of		
Projec	t Name	: Chatburn I	₹oad,		Project No. C2099		Co-ords	: -	Hole Type WS		
Locat	ion:	Clitheroe					Level:		Scale		
LUCAI		Cittieide					Levei.		1:50		
Client	:	Oakmere I					Dates:	Logged B	У		
Well	Water Strikes			In Situ Testing	Depth	Level	Legend	StratumDescription			
	Strikes	Depth (m)	Туре	Results	(m)	(m)	377837783				
	Stilkes	0.50 1.0	D	50 (4 for 0mm/50f 0mm)	0.15			Turf over dark brown sandy gravelly plasticity (field description). Gravel angular to angular fine and medium and mudstone.  Firm orangish brown slightly gravelly CLAY. High plasticity (field description is sub-rounded to angular fine to consandstone and mudstone.	is sub- of sandstone very sandy on). Gravel	3	
										10	

- 1. No groundwater was encountered during the drilling process.
- 2. Borehole was terminated at 1.00m depth due to refusal and backfilled with arisings.





## **Appendix III**





## **Appendix IV**

## **SUMMARY OF GEOTECHNICAL TESTING**

	Sample details						n Te	sts		Densit	Density Tests		Undrained Triaxial Compression			hemical Te	ests	
Borehole / Sample Ref	Depth (m)	Туре	Description	МС	LL	PL	P		425 Jm	Bulk	Dry	Cell Pressure	Deviator Stress	Shear Stress	рН	2:1 W/S SO4	W/S Mg	Other tests and comments
	,			(%)	(%)	(%)	(%	%) (	(%)	Mg/m³	Mg/m³	kPa	kPa	kPa		(g/L)	(mg/L)	
BH1	2.80-3.30	В	Grey sandy silty CLAY with abundant gravel	8.4	27	14	1	3 6	62									2.5kg Compaction
BH2	1.00-1.50	В	Grey brown gravelly sandy silty CLAY. Gravel is mudstone.	37	43	23	2	20 8	80									Particle Size Distribution 2.5kg Compaction
BH2	1.90-2.35	U	Soft to firm brown silty CLAY with abundant gravel and rootlets															Oedometer consolidation
BH2	2.70-3.20	В	Grey brown gravelly sandy silty CLAY															Particle Size Distribution
BH2	3.50-3.80	В	Grey brown sandy silty clayey GRAVEL. Gravel is fine to cobble sized limestone.	4.9	24	13	1	1 2	28									Particle Size Distribution Compaction cancelled - insufficient material
BH2	3.50	U	Firm to stiff grey sandy gravelly CLAY	9.9						2.29	2.08	35	196	98				
ВН3	1.10-1.60	В	Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized limestone.	14	28	16	1:	2 5	54									Particle Size Distribution Compaction cancelled - unsuitable material, too much coarse gravel present
ВН3	1.80-2.20	U	Firm grey silty CLAY with abundant gravel	9.5	24	15	9.	.0	60									Oedometer consolidation
ВН3	2.70-3.20	В	Dark grey-brown sandy very gravelly silty CLAY. Gravel includes cobble sized gravel.	10	27	14	1:	3 4	47									
ВН3	3.70-4.15	U	Soft to firm grey brown sandy gravelly silty CLAY. Gravel is fine to medium.	12						2.39	2.13	37 74	87 90	44				
TP10	0.60	В	Brown sandy silty CLAY with rare fine to medium gravel															Particle Size Distribution California Bearing Ratio
TP10	1.50	В	Brown sandy very gravelly silty CLAY. Gravel includes cobble sized gravel.	18	36	17	1:	9 !	56									

Sample type: B (Bulk disturb.) BLK (Block) C (Core) D (Disturbed) LB (Large Bulk dist.) U (Undisturbed)

Operations Manager 27/04/2015 Project Number:

Project Name:

GEO / 22476

**C2099 CLITHEROE** 

**GEOLABS** 

## **SUMMARY OF GEOTECHNICAL TESTING**

			Sample d	etails		Class	sificatio	n Test	ts		Density Tests		Undrained	Triaxial Co	mpression	CI	emical Te	ests	
Borehole / Trial Pit	ample Ref	Depth (m)	Туре	Description	МС	LL		PI	μι	25 m	Bulk	Dry	Cell Pressure	Deviator Stress	Shear Stress	рН	2:1 W/S SO4	W/S Mg	Other tests and comments
					(%)	(%	5) (%)	(%)	) (%	%)	Mg/m³	Mg/m³	kPa	kPa	kPa		(g/L)	(mg/L)	
TP10		2.40	В	Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble sized.															Particle Size Distribution
TP2		0.80	В	Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized															California Bearing Ratio
TP3		0.50	В	Brown gravelly sandy silty CLAY. Gravel is fine to cobble sized limestone.															Particle Size Distribution California Bearing Ratio
TP3		1.50	В	Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized limestone.	17	37	18	19	5	6									Particle Size Distribution Compaction cancelled - unsuitable material, too much coarse gravel present
TP4		0.60	В	Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble sized sandstone.															California Bearing Ratio
TP5		0.50	В	Brown sandy silty CLAY with rare fine to medium gravel															Particle Size Distribution
TP5		1.60	В	Brown mottled orange sandy gravelly silty CLAY. Gravel includes cobble sized gravel with rare rootlets.	17	36	20	16	6	57									
TP5		1.90	D	Brown sandy gravelly silty CLAY.	8.4	27	13	14	4	4									
TP6		0.50	В	Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized.	28	41	19	22	2 8	6									California Bearing Ratio
TP6		1.50	В	Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble sized.															Particle Size Distribution
TP7		0.50	D	Mottled brown grey and dark orange silty CLAY with rare rootlets.	28	51	27	24	1 10	00									
TP8		0.50	В	Brown sandy gravelly silty CLAY. Gravel is sandstone.															California Bearing Ratio

Sample type: B (Bulk disturb.) BLK (Block) C (Core) D (Disturbed) LB (Large Bulk dist.) U (Undisturbed)

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15/
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27/04/2015

Project Number:

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**GEOLABS** 

## **SUMMARY OF GEOTECHNICAL TESTING**

	Sample details							Tests		Densit	y Tests	Undrained Triaxial Compression		С	hemical Te	sts		
Borehole / Trial Pit	Sample Ref	Depth (m)	Туре	Description	MC (%)			PI %)	<425 μm (%)	Bulk	Dry Mg/m³	Cell Pressure kPa	Deviator Stress kPa	Shear Stress kPa	рН	2:1 W/S SO4 (g/L)	W/S Mg (mg/L)	Other tests and comments
TP9		0.60	В	Brown gravelly sandy silty CLAY. Gravel is sandstone.	30	39	28	11	73									Particle Size Distribution 2.5kg Compaction
TP9		1.50	В	Grey brown sandy gravelly silty CLAY.														Particle Size Distribution

Sample type: B (Bulk disturb.) BLK (Block) C (Core) D (Disturbed) LB (Large Bulk dist.) U (Undisturbed)

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Operations Manager 27/04/2015	C2099 CLITHEROE	

BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

Hole ID Depth (m) 2.80-3.30 Sample Type

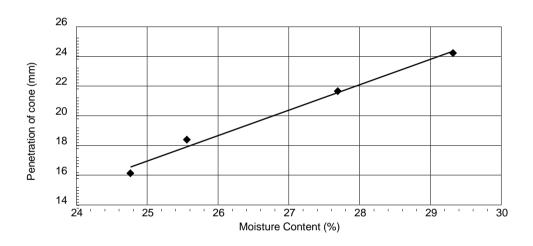
Description:

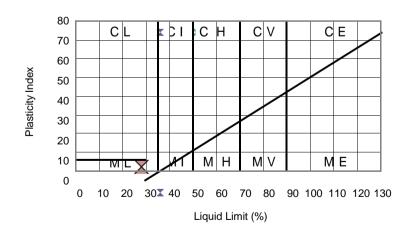
Grey sandy silty CLAY with abundant gravel

Sample washed and air dried Preparation:

Moisture content: (BS1377:Part 2:Clause 3:1990) 8.4 % Percentage passing 425µm sieve : 62 % Liquid Limit: 27 % Plastic Limit: 14 % Plasticity Index: 13

Equivalent moisture content of material passing 425µm sieve : 14 % Liquidity Index: -0.03





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GL:Version 1.54 - 23/04/2015

BH2 01.00

1220 - LLPL

## BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

Hole ID Depth (m) 1.00-1.50 Sample Type

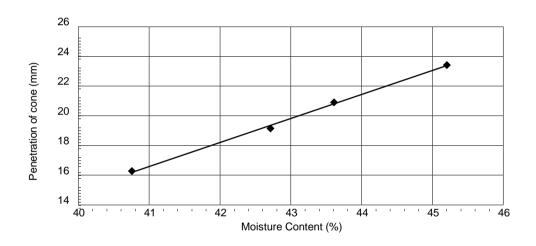
Description:

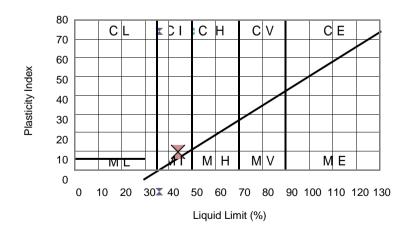
Grey brown gravelly sandy silty CLAY. Gravel is mudstone.

Preparation: Sample washed and air dried

Moisture content: (BS1377:Part 2:Clause 3:1990) 37 % Percentage passing 425µm sieve : 80 % Liquid Limit: 43 % Plastic Limit: 23 % Plasticity Index: 20

Equivalent moisture content of material passing 425µm sieve : 46 % Liquidity Index: 1.17





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BH2 03.50

1220 - LLPL

## BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

Hole ID Depth (m) Sample Type

BH2 3.50-3.80 Description:

Grey brown sandy silty clayey GRAVEL. Gravel is fine to cobble sized limestone.

Preparation: Sample washed and air dried

Moisture content : (BS1377:Part 2:Clause 3:1990)

Percentage passing 425μm sieve :

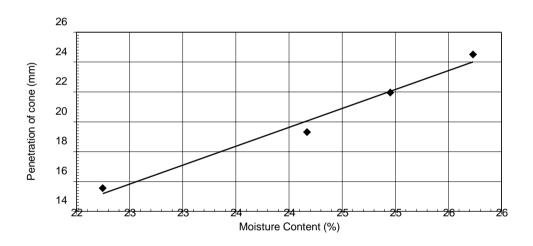
Liquid Limit :

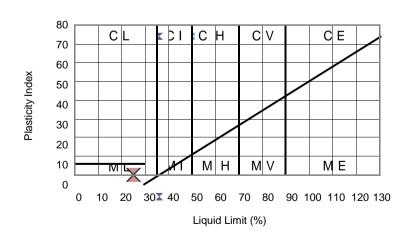
Plastic Limit :

Plasticity Index :

11

Equivalent moisture content of material passing 425µm sieve : 18 % Liquidity Index : 0.42





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GEO / 22476

Project Name:



# B - 22476-113804.xls

BH3 01.10

1220 - LLPL

## BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

Hole ID Depth (m) Sample Type

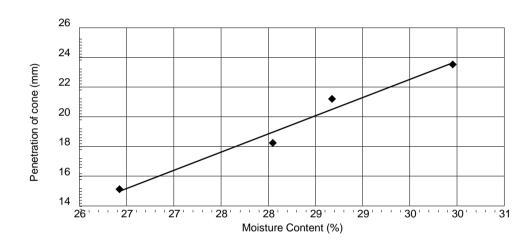
BH3 1.10-1.60 Description:

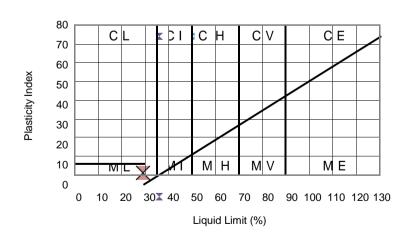
Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized limestone.

Sample washed and air dried Preparation:

Moisture content: (BS1377:Part 2:Clause 3:1990) 14 % Percentage passing 425µm sieve : 54 % Liquid Limit: 28 % Plastic Limit: 16 % Plasticity Index: 12

Equivalent moisture content of material passing 425µm sieve : 26 % Liquidity Index: 0.84





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Project Name:



# 220 - LLPL BH3 01.80 U - 22476-113820.xls

## BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

Hole ID BH3 Depth (m) 1.80-2.20 Sample Type

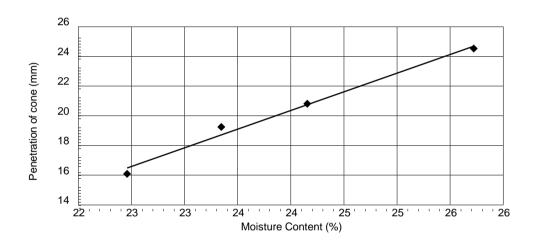
Description:

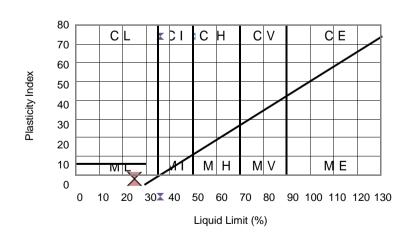
Firm grey silty CLAY with abundant gravel

Sample washed and air dried Preparation:

Moisture content: (BS1377:Part 2:Clause 3:1990) 9.5 % Percentage passing 425µm sieve : 60 % Liquid Limit: 24 % Plastic Limit: 15 % Plasticity Index: 9.0

Equivalent moisture content of material passing 425µm sieve : 16 % Liquidity Index: 0.08





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C2099 CLITHEROE



GL:Version 1.54 - 23/04/2015

BH3 02.70

1220 - LLPL

BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

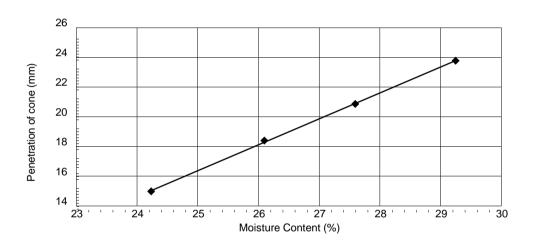
Hole ID Depth (m) Sample Type BH3 2.70-3.20 Description:

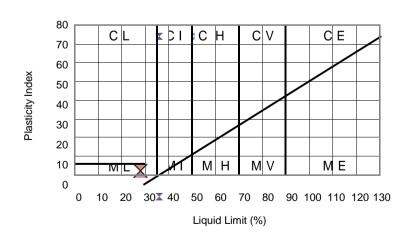
Dark grey-brown sandy very gravelly silty CLAY. Gravel includes cobble sized gravel.

Preparation: Sample washed and air dried

Moisture content : (BS1377:Part 2:Clause 3:1990)	10	%
Percentage passing 425µm sieve :	47	%
Liquid Limit :	27	%
Plastic Limit :	14	%
Plasticity Index :	13	

Equivalent moisture content of material passing 425µm sieve : 21 % Liquidity Index: 0.57





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Project Name:



# ģ 1220 - LLPL TP10 01.50

## BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

TP10 Hole ID 1.50 Depth (m) Sample Type В

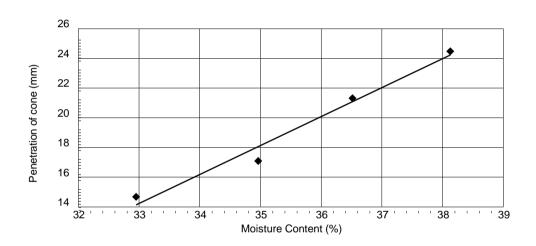
Description:

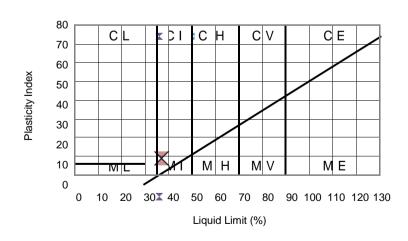
Brown sandy very gravelly silty CLAY. Gravel includes cobble sized gravel.

Sample washed and air dried Preparation:

Moisture content: (BS1377:Part 2:Clause 3:1990) 18 % Percentage passing 425µm sieve : 56 % Liquid Limit: 36 % Plastic Limit: 17 % Plasticity Index: 19

Equivalent moisture content of material passing 425µm sieve : 32 % Liquidity Index: 0.79





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Project Name:



## BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

TP3 Hole ID 1.50 Depth (m) Sample Type В

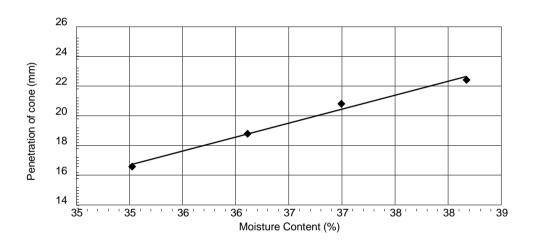
Description:

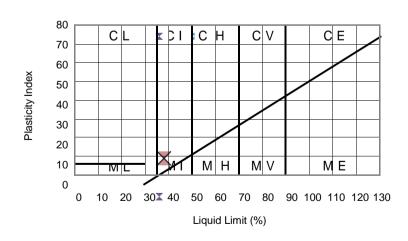
Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized limestone.

Sample washed and air dried Preparation:

Moisture content: (BS1377:Part 2:Clause 3:1990) 17 % Percentage passing 425µm sieve : 56 % Liquid Limit: 37 % Plastic Limit: 18 % Plasticity Index: 19

Equivalent moisture content of material passing 425µm sieve : 31 % Liquidity Index: 0.66





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## BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

TP5 Hole ID Depth (m) 1.60 Sample Type В

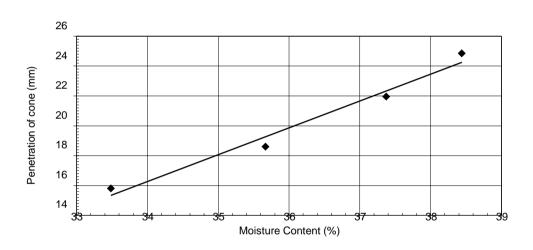
Description:

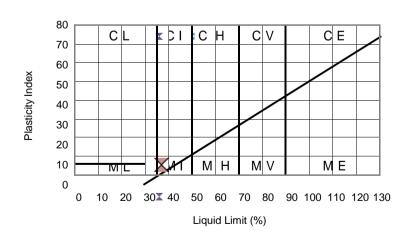
Brown mottled orange sandy gravelly silty CLAY. Gravel includes cobble sized gravel with rare rootlets.

Preparation: Sample washed and air dried

Moisture content : (BS1377:Part 2:Clause 3:1990)	17	%
Percentage passing 425µm sieve :	67	%
Liquid Limit :	36	%
Plastic Limit :	20	%
Plasticity Index :	16	

Equivalent moisture content of material passing 425µm sieve : 25 % Liquidity Index: 0.33





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## BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

TP5 Hole ID 1.90 Depth (m) Sample Type D

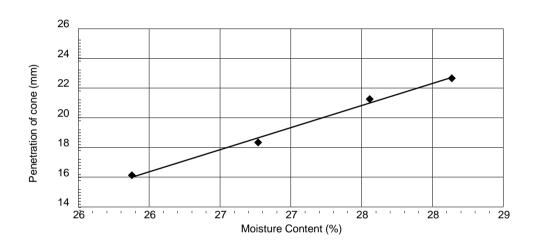
Description:

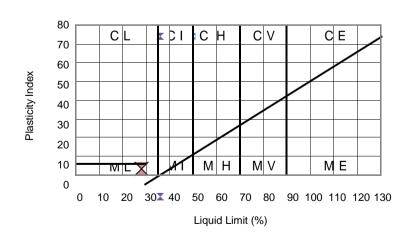
Brown sandy gravelly silty CLAY.

Preparation: Sample washed and air dried

Moisture content: (BS1377:Part 2:Clause 3:1990) 8.4 % Percentage passing 425µm sieve : 44 % Liquid Limit: 27 % Plastic Limit: 13 % Plasticity Index: 14

Equivalent moisture content of material passing 425µm sieve : 19 % Liquidity Index: 0.42





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## BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

TP7 Hole ID 0.50 Depth (m) Sample Type D

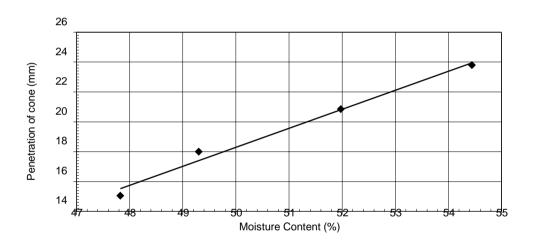
Description:

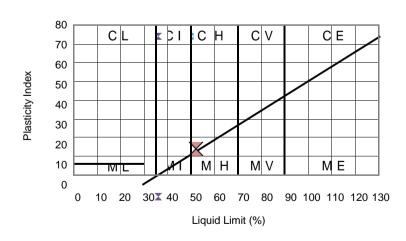
Mottled brown grey and dark orange silty CLAY with rare rootlets.

Preparation: Sample as received

Moisture content: (BS1377:Part 2:Clause 3:1990) 28 % Percentage passing 425µm sieve : 100 % Liquid Limit: 51 % Plastic Limit: 27 % Plasticity Index: 24

Equivalent moisture content of material passing 425µm sieve : 28 % Liquidity Index: 0.04





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## BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

TP9 Hole ID 0.60 Depth (m) Sample Type В

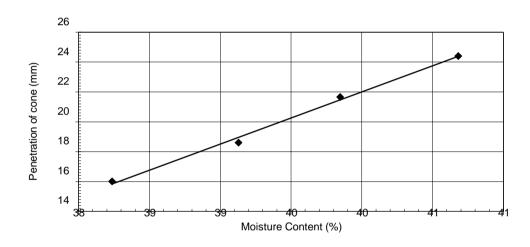
Description:

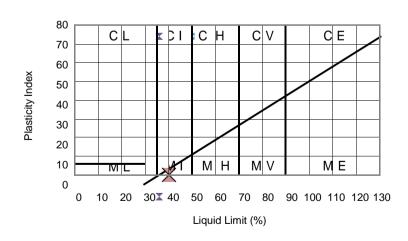
Brown gravelly sandy silty CLAY. Gravel is sandstone.

Preparation: Sample washed and air dried

Moisture content: (BS1377:Part 2:Clause 3:1990) 30 % Percentage passing 425µm sieve : 73 % Liquid Limit: 39 % Plastic Limit: 28 % Plasticity Index:

Equivalent moisture content of material passing 425µm sieve : 41 % Liquidity Index: 1.19





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BS1377: Part 2: Clause 9: 1990

## PARTICLE SIZE DISTRIBUTION

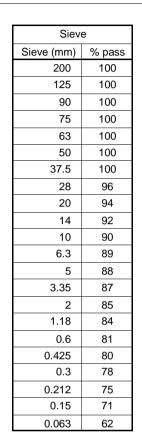
BH/TP No: BH₂ Depth (m): 1.00-1.50 Sample Type

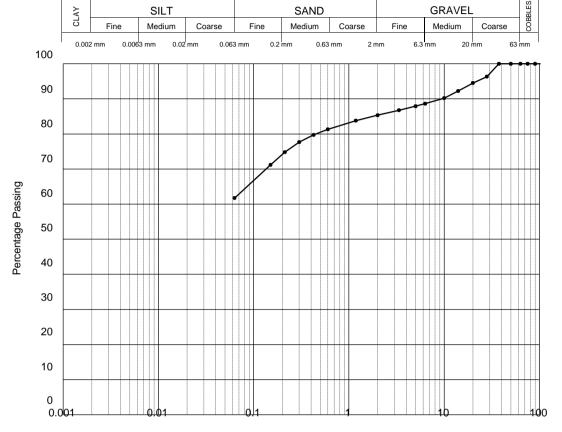
В

Description:

Grey brown gravelly sandy silty CLAY. Gravel is mudstone.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions									
Cobbles	0.0 %								
Gravel	14.7 %								
Sand	23.6 %								
Silt & Clay	61.8 %								

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BS1377: Part 2: Clause 9: 1990

## PARTICLE SIZE DISTRIBUTION

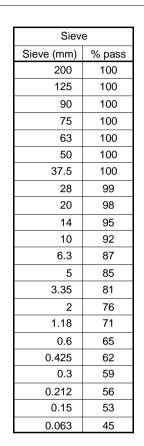
BH/TP No: BH₂ Depth (m): 2.70-3.20 Sample Type

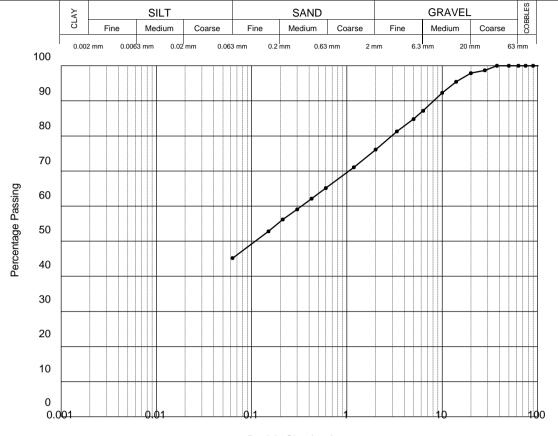
В

Description:

Grey brown gravelly sandy silty CLAY

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions								
Cobbles	0.0 %							
Gravel	23.9 %							
Sand	30.9 %							
Silt & Clay	45.2 %							

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BS1377: Part 2: Clause 9: 1990

## **PARTICLE SIZE DISTRIBUTION**

 BH/TP No:
 BH2

 Depth (m):
 3.50-3.80

 Sample Type
 B

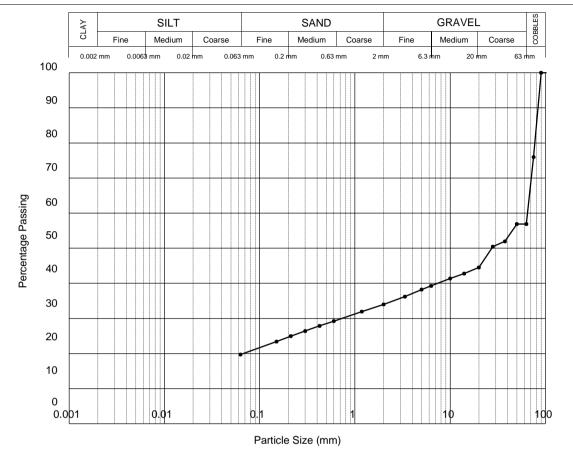
Description:
Grey brow

Grey brown sandy silty clayey GRAVEL. Gravel is fine to cobble sized limestone.

Insufficient material supplied to be representative in accordance with BS1377 requirements.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Sieve	
Sieve (mm)	% pass
200	100
125	100
90	100
75	76
63	57
50	57
37.5	52
28	50
20	45
14	43
10	41
6.3	39
5	38
3.35	36
2	34
1.18	32
0.6	29
0.425	28
0.3	26
0.212	25
0.15	23
0.063	20



Particle Proportions	
Cobbles	43.1 %
Gravel	22.9 %
Sand	14.3 %
Silt & Clay	19.7 %

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# **PARTICLE SIZE DISTRIBUTION**

BH/TP No: BH3 Depth (m): 1.10-1.60 Sample Type В

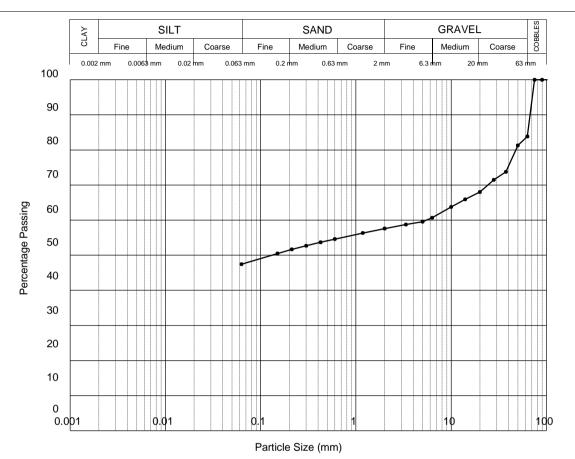
Description:

Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized limestone.

Insufficient material supplied to be representative in accordance with BS1377 requirements.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Siev	е
Sieve (mm)	% pass
200	100
125	100
90	100
75	100
63	84
50	81
37.5	74
28	71
20	68
14	66
10	64
6.3	61
5	60
3.35	59
2	58
1.18	56
0.6	55
0.425	54
0.3	53
0.212	52
0.15	50
0.063	47



Particle Proportions	
Cobbles	16.2 %
Gravel	26.2 %
Sand	10.1 %
Silt & Clay	47.5 %

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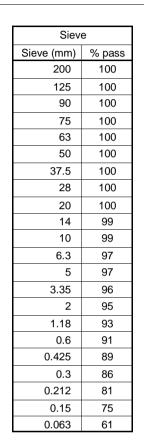
# PARTICLE SIZE DISTRIBUTION

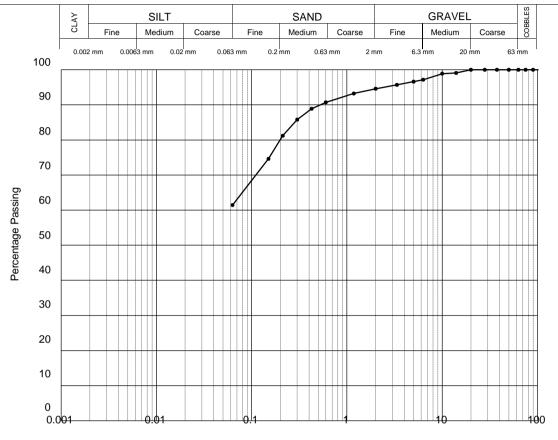
BH/TP No: TP10
Depth (m): 0.60
Sample Type B

Description:

Brown sandy silty CLAY with rare fine to medium gravel

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions	
Cobbles	0.0 %
Gravel	5.4 %
Sand	33.1 %
Silt & Clay	61.5 %

Checked and Approved by

Project Number:

GEO / 22476

Operations Manager

Project Name:



# **PARTICLE SIZE DISTRIBUTION**

BH/TP No: TP10
Depth (m): 2.40
Sample Type B

Description:

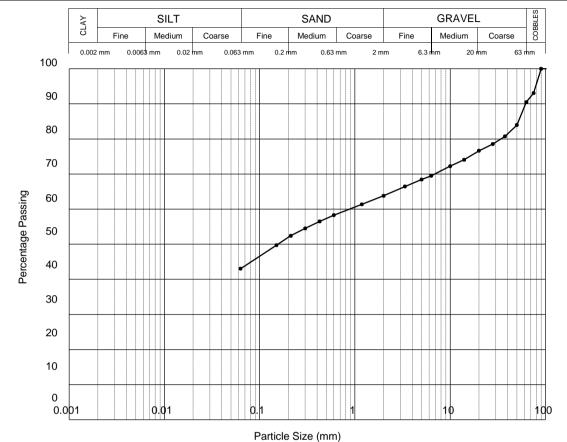
Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble

sized.

Insufficient material supplied to be representative in accordance with BS1377 requirements.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Sieve	
Sieve (mm)	% pass
200	100
125	100
90	100
75	93
63	91
50	84
37.5	81
28	79
20	77
14	74
10	72
6.3	70
5	68
3.35	66
2	64
1.18	61
0.6	58
0.425	56
0.3	55
0.212	52
0.15	50
0.063	43



Particle Proportions	
Cobbles	9.5 %
Gravel	26.7 %
Sand	20.8 %
Silt & Clay	43.0 %

Checked and Approved by

Operations Manager 27/04/2015 Project Number:

Project Name:

GEO / 22476



# PARTICLE SIZE DISTRIBUTION

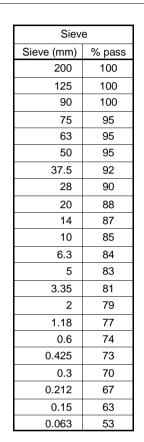
BH/TP No: TP3
Depth (m): 0.50
Sample Type B

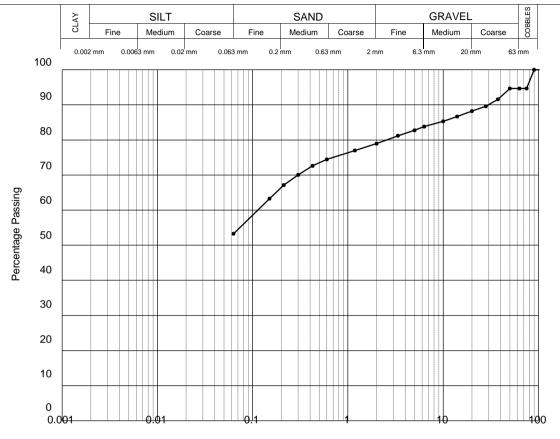
Description:

Brown gravelly sandy silty CLAY. Gravel is fine to cobble sized

limestone.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions	
Cobbles	5.3 %
Gravel	15.7 %
Sand	25.7 %
Silt & Clay	53.3 %

Checked and Approved by

Project Number:

GEO / 22476

151

Project Name:



# **PARTICLE SIZE DISTRIBUTION**

BH/TP No: TP3
Depth (m): 1.50
Sample Type B

Description:

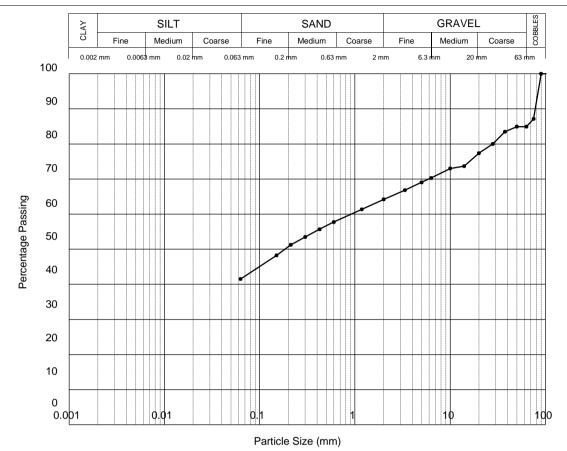
Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized

limestone.

Insufficient material supplied to be representative in accordance with BS1377 requirements.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Siev	e
Sieve (mm)	
200	% pass 100
125	100
90	100
75	87
63	85
50	85
37.5	83
28	80
20	77
14	74
10	73
6.3	70
5	69
3.35	67
2	64
1.18	61
0.6	58
0.425	56
0.3	53
0.212	51
0.15	48
0.063	42



Particle Proportions	
Cobbles	15.1 %
Gravel	20.7 %
Sand	22.7 %
Silt & Clay	41.5 %

Checked and Approved by

Operations Manager 27/04/2015 Project Number:

Project Name:

GEO / 22476



# PARTICLE SIZE DISTRIBUTION

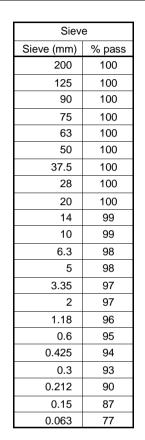
BH/TP No: TP5
Depth (m): 0.50

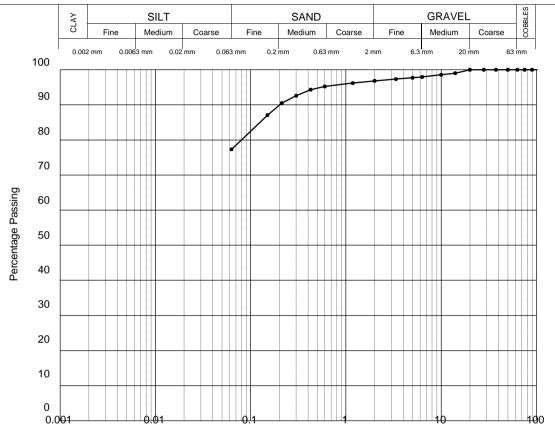
Description:

Brown sandy silty CLAY with rare fine to medium gravel

Sample Type B

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions	
Cobbles	0.0 %
Gravel	3.2 %
Sand	19.5 %
Silt & Clay	77.3 %

Checked and Approved by

Project Number:

GEO / 22476

Operation

Project Name:



# **PARTICLE SIZE DISTRIBUTION**

BH/TP No: TP6
Depth (m): 1.50
Sample Type B

Description:

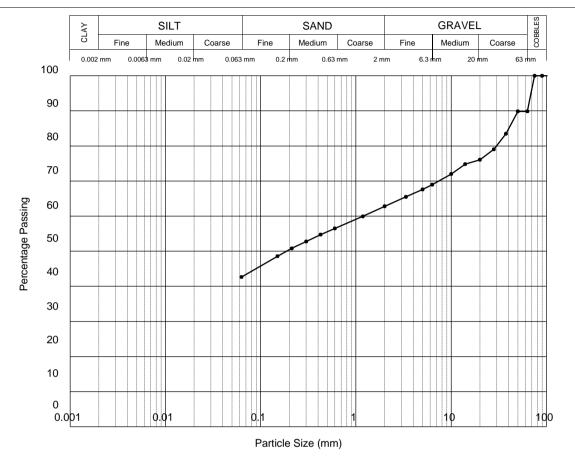
Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble

sized.

Insufficient material supplied to be representative in accordance with BS1377 requirements.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Siev	e
Sieve (mm)	% pass
200	100
125	100
90	100
75	100
63	90
50	90
37.5	83
28	79
20	76
14	75
10	72
6.3	69
5	68
3.35	66
2	63
1.18	60
0.6	57
0.425	55
0.3	53
0.212	51
0.15	49
0.063	43



Particle Proportions	
Cobbles	10.2 %
Gravel	27.0 %
Sand	20.2 %
Silt & Clay	42.6 %

Checked and Approved by

Operations Manager 27/04/2015 Project Number:

Project Name:

GEO / 22476



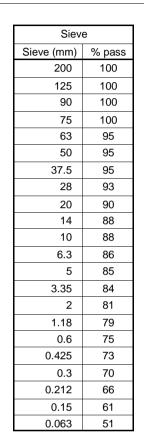
# PARTICLE SIZE DISTRIBUTION

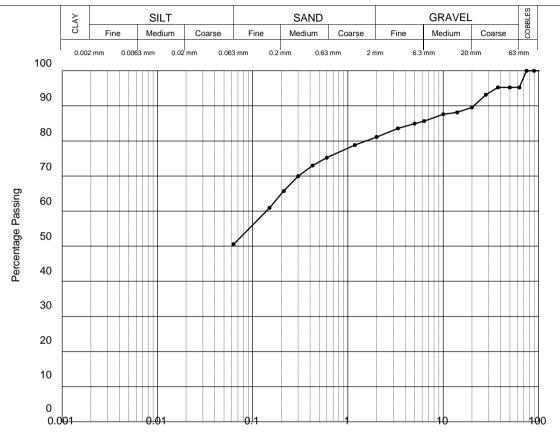
BH/TP No: TP9
Depth (m): 0.60
Sample Type B

Description:

Brown gravelly sandy silty CLAY. Gravel is fine to cobble sized.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions	
Cobbles	4.7 %
Gravel	14.1 %
Sand	30.6 %
Silt & Clay	50.6 %

Checked and Approved by

Operations Manager 27/04/2015

Project Number:

Project Name:

GEO / 22476

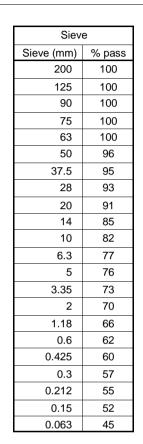


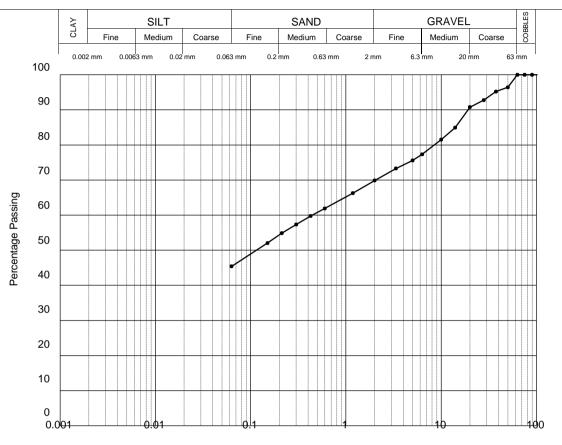
# PARTICLE SIZE DISTRIBUTION

BH/TP No: TP9
Depth (m): 1.50
Sample Type B

Description:
Grey brown sandy gravelly silty CLAY.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions			
Cobbles 0.0 %			
Gravel	30.1 %		
Sand	24.5 %		
Silt & Clay 45.4 %			

Checked and Approved by

Operations Manager 27/04/2015

Project Number:

Project Name:

GEO / 22476



# BS1377:Part 4:1990 Clause 3.4

# **Moisture Content / Dry Density Relationship**

BH/TP BH1
Depth (m) 2.80-3.30
Sample Type B

B - 22476-113809.xls

1410 - Comp BH1 02.80

Description:

Grey sandy silty CLAY with abundant gravel

Preparation Air dried / Oven dried

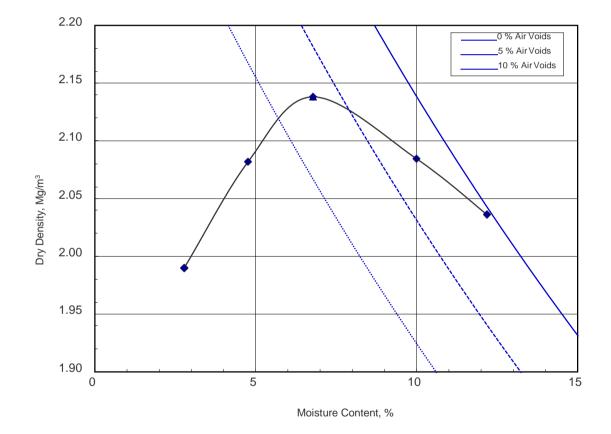
Test Method

2.5kg Rammer for soils with some coarse gravel-size particles

Samples Used Single / Multiple Mass Retained on 37.5 mm Sieve % 9

Mass Retained on 20.0 mm Sieve%15Particle Density - AssumedMg/m³2.72

Maximum Dry Density Mg/m³ 2.14
Optimum Moisture Content % 6.8



Determination	1	2	3	4	5
Moisture Content %	2.8	4.8	6.8	10.0	12
Dry Density Mg/m	1.99	2.08	2.14	2.08	2.04

Checked and Approved by:

GL:Version 1.14 - 10/12/2014

Operations Manager 27/04/2015 Project Number:

Project Name:

GEO / 22476



# BS1377:Part 4:1990 Clause 3.4

# **Moisture Content / Dry Density Relationship**

BH/TP BH2 Depth (m) 1.00-1.50 Sample Type В

B - 22476-114255.xls

1410 - Comp BH2 01.00

Description:

Brown sandy silty CLAY with rare gravel

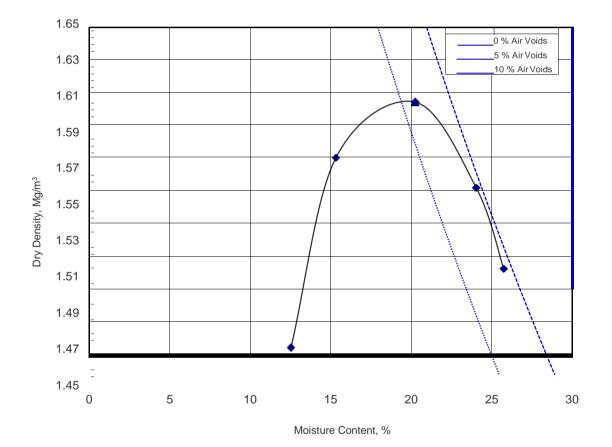
Preparation Air dried / Oven dried

2.5kg Rammer for soils with some coarse Test Method gravel-size particles

Single / Multiple Samples Used

Mass Retained on 37.5 mm Sieve % 9 Mass Retained on 20.0 mm Sieve % 15 Particle Density - Assumed Mg/m³ 2.72

Maximum Dry Density Mg/m³ 1.61 **Optimum Moisture Content** % 20.1



Determination	1	2	3	4	5
Moisture Content %	12	15	20	24	26
Dry Density Mg/m	1.47	1.57	1.61	1.56	1.51

Checked and Approved by:

Operations Manager 27/04/2015

GL:Version 1.14 - 10/12/2014

Project Number:

**GEO / 22476** 

Project Name:



# BS1377:Part 4:1990 Clause 3.4 **Moisture Content / Dry Density Relationship**

TP9 BH/TP Depth (m) 0.60 Sample Type В

B - 22476-113797.xls

1410 - Comp TP9 00.60

Description:

Brown sandy silty CLAY with occasional gravel sized sandstone.

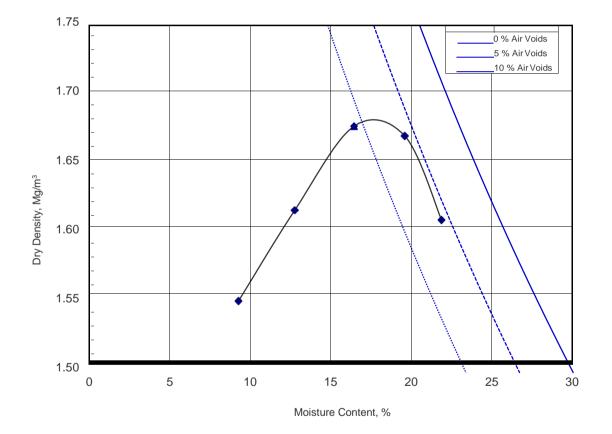
Preparation Air dried / Oven dried

2.5kg Rammer for soils with some coarse Test Method gravel-size particles

Single / Multiple Samples Used Mass Retained on 37.5 mm Sieve % 5 6 Mass Retained on 20.0 mm Sieve %

Particle Density - Assumed Mg/m³ 2.72

Maximum Dry Density Mg/m³ 1.68 **Optimum Moisture Content** % 16.3



Determination	1	2	3	4	5
Moisture Content %	9.1	13	16	19	22
Dry Density Mg/m	³ 1.55	1.62	1.68	1.67	1.61

Checked and Approved by:

Operations Manager 27/04/2015

GL:Version 1.14 - 10/12/2014

Project Number:

**GEO / 22476** 

Project Name:



# **Determination of California Bearing Ratio**

BH/TP No.: TP10
Depth (m): 0.6
Sample Type: B

B - 22476-113784.xls

Description:

Brown very sandy silty CLAY

## **PREPARATION DETAILS**

The specimen was tested in an unsoaked condition.

The specimen was initially air dried

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.87 Mg/m³

Prepared dry density 1.45 Mg/m³

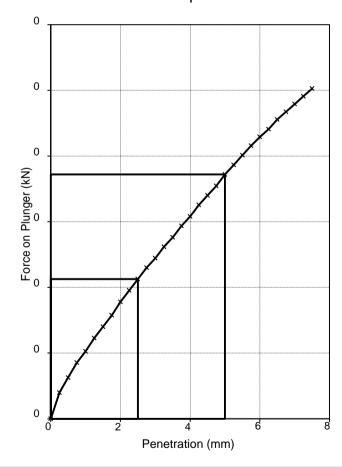
0.0 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base
Surcharge	8.0 kg	8.0 kg
Seating load	10 N	10 N
Moisture content	29 %	29 %
CBR Value	0.8 %	0.9%

# Top of Specimen

# Penetration (mm)

# Base of Specimen



Checked and Approved by:

Operations Manager

Project Number:

Project Name:

GEO / 22476



# **Determination of California Bearing Ratio**

BH/TP No.: TP2
Depth (m): 0.8
Sample Type: B

B - 22476-113801.xls

Description:

Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized

## PREPARATION DETAILS

The specimen was tested in an unsoaked condition.

The specimen was tested at its existing moisture content

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.98 Mg/m³

Prepared dry density 1.61 Mg/m³

10.8 % of the sample was retained on a 20mm sieve

Test DetailsTopBaseSurcharge8.0 kg8.0 kgSeating load10 N10 NMoisture content23 %22 %

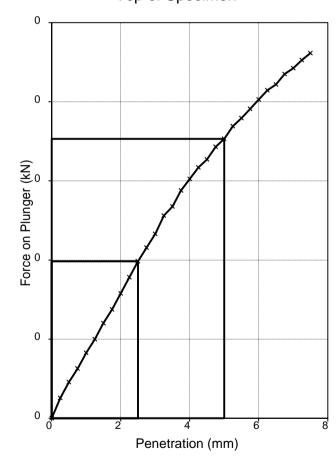
0.9 %

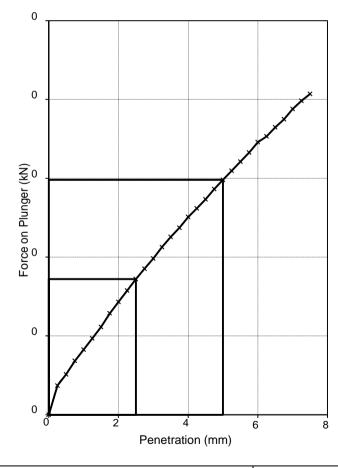
Top of Specimen

**CBR Value** 

# Base of Specimen

0.7 %





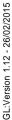
Checked and Approved by:

Operations Manager 27/04/2015 Project Number:

GEO / 22476

Project Name:

C2099 CLITHEROE



GEOLABS

# **Determination of California Bearing Ratio**

BH/TP No.: TP3
Depth (m): 0.5
Sample Type: B

B - 22476-113799.xls

Description:

Brown gravelly sandy silty CLAY. Gravel is fine to cobble sized

limestone.

## PREPARATION DETAILS

The specimen was tested in an unsoaked condition. The specimen was tested at its existing moisture content

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.88 Mg/m³

Prepared dry density 1.46 Mg/m³

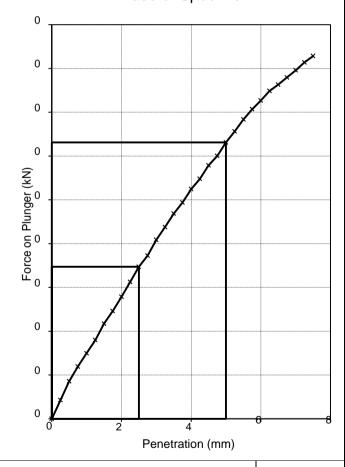
10.3 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base
Surcharge	8.0 kg	8.0 kg
Seating load	10 N	10 N
Moisture content	30 %	29 %
CBR Value	0.7 %	0.6 %

# Top of Specimen

# 

# Base of Specimen



Checked and Approved by:

Operations Manager

Project Number:

Project Name:

GEO / 22476



# **Determination of California Bearing Ratio**

BH/TP No.: TP4
Depth (m): 0.6
Sample Type: B

B - 22476-113814.xls

Description:

Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble sized sandstone.

## **PREPARATION DETAILS**

The specimen was tested in an unsoaked condition.

The specimen was tested at its existing moisture content

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.91 Mg/m³

Prepared dry density 1.54 Mg/m³

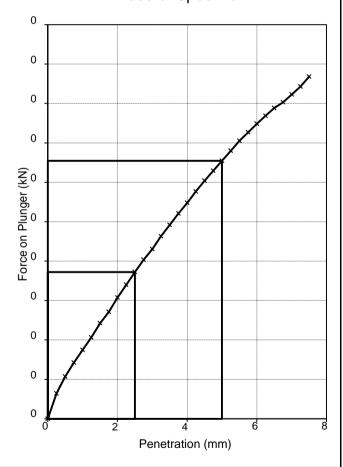
14.1 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base
Surcharge	8.0 kg	8.0 kg
Seating load	10 N	10 N
Moisture content	23 %	25 %
CBR Value	0.7 %	0.7 %

# Top of Specimen

# Penetration (mm)

# Base of Specimen



Checked and Approved by:

Operations Manager

Project Number:

GEO / 22476

Project Name:



# **Determination of California Bearing Ratio**

BH/TP No.: TP6
Depth (m): 0.5
Sample Type: B

B - 22476-113805.xls

Description:

Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized.

## **PREPARATION DETAILS**

The specimen was tested in an unsoaked condition.

The specimen was tested at its existing moisture content

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.87 Mg/m³

Prepared dry density 1.46 Mg/m³

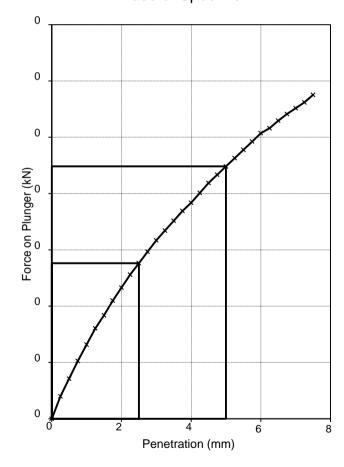
Prepared dry density 1.46 Mg/m³
3.8 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base
Surcharge	8.0 kg	8.0 kg
Seating load	10 N	10 N
Moisture content	28 %	28 %
CBR Value	1.2 %	1.1 %

# Top of Specimen

# 

# Base of Specimen



Checked and Approved by:

Operations Manager

JEN.

Project Number:

Project Name:

GEO / 22476

**C2099 CLITHEROE** 



GL:Version 1.12 - 26/02/2015

# **Determination of California Bearing Ratio**

BH/TP No.: TP8
Depth (m): 0.5
Sample Type: B

B - 22476-113825.xls

Description:

Brown sandy gravelly silty CLAY. Gravel is sandstone.

## **PREPARATION DETAILS**

The specimen was tested in an unsoaked condition.

The specimen was tested at its existing moisture content

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.84 Mg/m³

Prepared dry density 1.41 Mg/m³

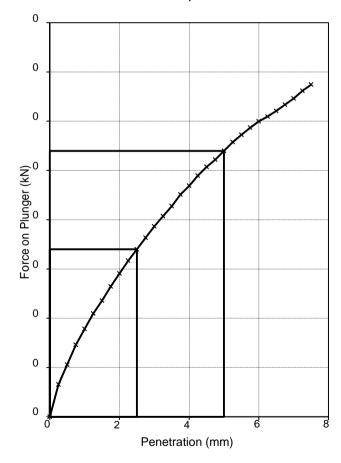
2.1 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base
Surcharge	8.0 kg	8.0 kg
Seating load	10 N	10 N
Moisture content	30 %	30 %
CBR Value	13%	13%

# Top of Specimen

# 

# Base of Specimen



Checked and Approved by:

Operations Manager

Project Number:

GEO / 22476

Project Name:



# BS 1377: Part 7: 1990 Clause 8

# **Quick Undrained Triaxial Compression Test**

BH/TP No BH2 Depth (m) 3.50 Sample Type

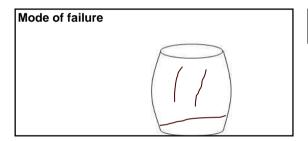
Description:

Firm to stiff grey sandy gravelly CLAY

Remarks: Sample reached 20% strain on first stage of multistage test

# **Specimen Details**

Specimen conditions		Undisturbed
Length	(mm)	201.6
Diameter	(mm)	102.1
Moisture Content	(%)	9.9
Bulk Density	(Mg/m³)	2.29
Dry Density	(Mg/m³)	2.08
Test Details		
Latex membrane thickness	(mm)	0.3
Membrane correction	(kPa)	1.1
Axial displacement rate	(%/min)	2.0
Cell pressure	(kPa)	35
Strain at failure	(%)	20.8
Maximum Deviator Stress	(kPa)	196
Shear Stress Cu	(kPa)	98



Orientation of the sample	Vertical
Distance from top of tube mm	50



GEO / 22476

**C2099 CLITHEROE** 



GL:Version 1.44 - 16/03/2015

Project Name:

# **Quick Undrained Triaxial Compression Test**

BH/TP No BH3
Depth (m) 3.70-4.15
Sample Type U

731 - UUTXL BH3 03.70 U - 22476-113830.xls

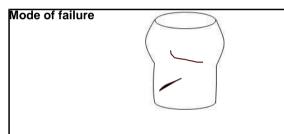
Description:

Soft to firm grey brown sandy gravelly silty CLAY. Gravel is fine to medium.

Remarks: Sample went to 20% on second stage of multistage test

# **Specimen Details**

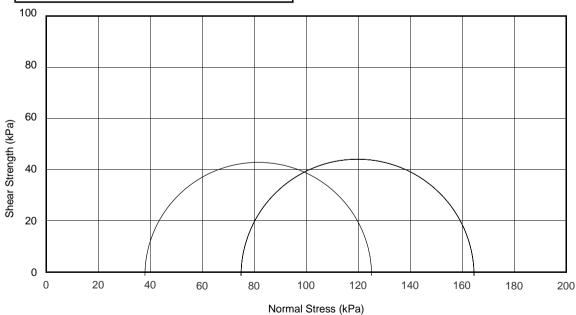
Specimen conditions		Undisturbed	
Length	(mm)	202.5	
Diameter	(mm)	101.7	
Moisture Content	(%)	12	
Bulk Density	(Mg/m³)	2.39	
Dry Density	(Mg/m³)	2.13	
Test Details		1	2
Latex membrane thickness	(mm)	0.3	0.3
Membrane correction	(kPa)	1.0	1.1
Axial displacement rate	(%/min)	1.0	1.0
Cell pressure	(kPa)	37	74
Strain at failure	(%)	17.3	20.7
Maximum Deviator Stress	(kPa)	87	90
Shear Stress Cu	(kPa)	44	45



Orientation of the sample	Vertical
Distance from top of tube mm	80

# **Shear Strength Parameters**

Cohesion (kPa) 41
Angle of Shear Resistance (°) 2.0



Checked and Approved by:

Operations Manager 27/04/2015

Project Number:

GEO / 22476

Project Name:

C2099 CLITHEROE



Version 1.44 - 16/03

# **Determination of One Dimensional Consolidation Properties of Soil**

Borehole No.: BH2

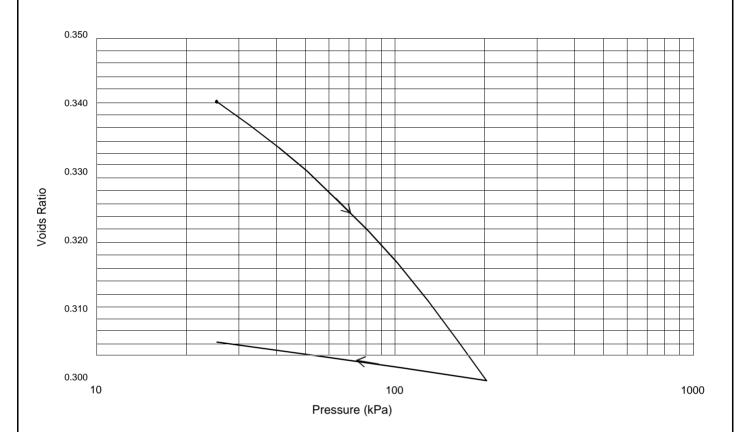
Sample Ref.: -

Depth (m): 1.90
Depth within original: 1.94
Orientation within original: Vertical
Specimen preparation: Undisturbed

Description:

Soft to firm brown silty CLAY with abundant gravel and roots

Oedometer taken from base of sample due to amount of gravel present



# **Initial Conditions:**

Initial (mm) 18.7 Moisture Content (%) 14

Height 18.7 (%) Diameter Voids Ratio (mm) 76.1 0.364 Area (mm2) 4550.8 **Bulk Density** (Mg/m!) 2.26 Volume (cm!) 85.1 Dry Density (Mg/m!) 1.98

Laboratory Temperature ("C) 20 Particle density (Mg/m!) 2.7 (Assumed)

Pressure Range (kPa)	m _v (m2/MN)	c _√ (m2/year)	Time Fitting Method	Void Ratio
0 - 25	0.65	2.4	t50	0.342
25 - 50	0.28	6.8	t50	0.333
50 - 100	0.18	5.6	t50	0.321
100 - 200	0.12	5.8	t50	0.305
200 - 25	0.010	7.0 (Sv)	t50	0.307

Checked and Approved by

Project Number:

JS J Sturges (Ops Mgr)

Date:

23/04/2015

Project Name:

GEO / 22476

C2099 Clitheroe

C2099 C



# **Determination of One Dimensional Consolidation Properties of Soil**

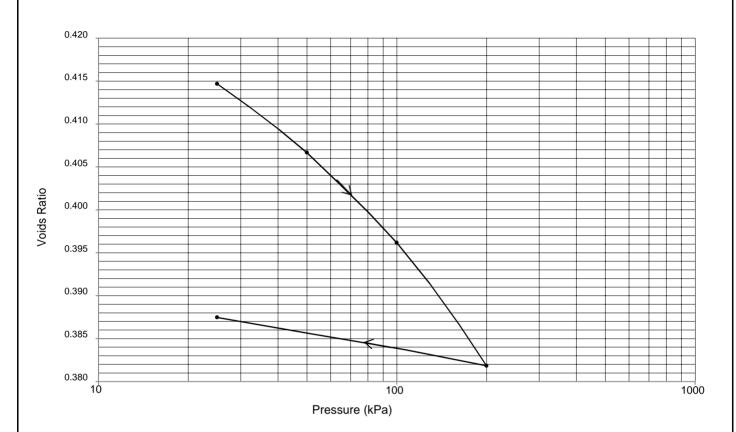
Borehole No.: BH3

Sample Ref.: -

Depth (m): 1.80
Depth within original: 1.84
Orientation within original: Vertical
Specimen preparation: Undisturbed

Description:

Firm grey silty CLAY with abundant gravel



# **Initial Conditions:**

Initial Height (mm) 18.4 Moisture Content (%) 11 Diameter (mm) 76.2 Voids Ratio 0.418 Area (mm2) 4556.8 **Bulk Density** (Mg/m!) 2.11 Volume (cm!) 83.8 Dry Density (Mg/m!) 1.90

Laboratory Temperature ("C) 20 Particle density (Mg/m!) 2.7 (Assumed)

Pressure Range (kPa)	$ m m_{v}$ (m2/MN)	c _∨ (m2/year)	Time Fitting Method	Void Ratio
0 - 25	0.015	0.72	t90	0.417
25 - 50	0.16	9.8	t90	0.412
50 - 100	0.10	5.8	t90	0.404
100 - 200	0.086	7.2	t90	0.392
200 - 25	-0.0073	4.3 (Sv)	t90	0.391

Checked and Approved by

23/04/2015

Project Number:

J Sturges (Ops Mgr)

Date:

Project Name:

GEO / 22476

C2099 Clitheroe





# **Appendix V**





# Chemtest The right chemistry to deliver results

Chemtest Ltd.
Depot Road
Newmarket
CB8 0AL
Tel: 01638 606070

Email: info@chemtest.co.uk

# **Final Report**

Report Number: 15-06828 Issue-1

Initial Date of Issue: 31-Mar-2015

Client: HSP Consulting Engineers Limited

Lawrence House

Meadowbank Way

Client Address: Eastwood

Nottinghamshire

NG16 3SB

Contact(s): Luke Bradley

Project: C2099 - Clitheroe

Quotation No.: Date Received: 25-Mar-2015

Order No.: Date Instructed: 25-Mar-2015

No. of Samples: 15

Turnaround: (Wkdays) 5 Results Due Date: 31-Mar-2015

Date Approved: 31-Mar-2015

Approved By:

**Details:** Keith Jones, Technical Manager



# **Results Summary - Soil**

Client: HSP Consulting Engineers Limited		Chem	ntest Jo	b No.:	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828
Quotation No.:	С	hemtes	st Samp	le ID.:	120111	120112	120113	120114	120115	120116	120117	120118
Order No.:		Clien	t Sample	e Ref.:								
		Clier	nt Samp	le ID.:	WS1	WS2	WS3A	WS6	WS7	WS8	WS5	WS5
			Sample		SOIL							
		7	op Dep	th (m):	0.5	0.5	0.1	0.5	0.5	0.6	0.5	0.1
			tom Dep									
			Date Sar	mpled:	17-Mar-15							
Determinand	Accred.	SOP	Units	LOD								
Moisture	N	2030	%	0.02	17	26	44	21	19	24	22	31
Soil Colour	N				Brown							
Other Material	N				Stones							
Soil Texture	N				Clay	Clay	Sand	Clay	Clay	Clay	Clay	Clay
рН	M	2010			6.9	7.2	5.7	7.4	6.5	7.5	7.0	6.2
Boron (Hot Water Soluble)	M	2120	mg/kg	0.4	< 0.40	0.41	1.9	0.66	0.51	0.52	< 0.40	1.8
Sulphate (2:1 Water Soluble) as SO4	M	2120	g/l	0.01	0.017	0.032	0.082	0.036	0.023	0.085	0.028	0.21
Total Sulphur	M	2175	%	0.01	< 0.010		0.070	< 0.010		< 0.010		
Sulphur (Elemental)	M	2180	mg/kg	1	< 1.0	< 1.0	4.2	4.0	< 1.0	< 1.0	< 1.0	6.5
Cyanide (Total)	M	2300	mg/kg	0.5	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50
Cyanide (Free)	M	2300	mg/kg	0.5	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50
Sulphide (Easily Liberatable)	M	2325	mg/kg	0.5	1.6	1.7	1.3	1.8	1.4	0.96	1.3	1.1
Sulphate (Acid Soluble)	M	2430	%	0.01	0.021		0.17	0.046		0.080		
Arsenic	M	2450	mg/kg	1	54	10	13	4.2	8.8	6.9	15	9.9
Cadmium	M	2450	mg/kg	0.1	< 0.10	0.21	0.77	0.41	0.85	0.42	2.3	0.50
Chromium	M	2450	mg/kg	1	28	29	33	19	26	23	29	25
Copper	M	2450	mg/kg	0.5	54	20	32	4.1	14	11	23	21
Mercury	M	2450	mg/kg	0.1	0.15	< 0.10	0.20	< 0.10	< 0.10	< 0.10	< 0.10	0.13
Nickel	M	2450	mg/kg	0.5	15	31	22	8.6	21	16	55	18
Lead	M	2450	mg/kg	0.5	85	27	100	23	45	31	56	65
Selenium	M	2450	mg/kg	0.2	0.52	0.25	0.72	0.27	0.38	0.23	1.0	0.34
Zinc	M	2450	mg/kg	0.5	75	80	130	68	130	95	280	100
Chromium (Hexavalent)	N	2490	mg/kg	0.5	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50
Organic Matter	M	2625	%	0.4	0.90	1.7	15	1.5	1.1	1.7	1.1	7.8
Aliphatic TPH >C5-C6	N		mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aliphatic TPH >C6-C8	N	2675	mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aliphatic TPH >C8-C10	M		mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aliphatic TPH >C10-C12	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C12-C16	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C16-C21	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C21-C35	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C35-C44	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Total Aliphatic Hydrocarbons	M	2675	mg/kg	5	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0



# **Results Summary - Soil**

Client: HSP Consulting Engineers Limited		Chen	ntest Jo	b No.:	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828
Quotation No.:	С		st Samp		120111	120112	120113	120114	120115	120116	120117	120118
Order No.:			t Sample		-							
		Clier	nt Samp	le ID.:	WS1	WS2	WS3A	WS6	WS7	WS8	WS5	WS5
			Sample		SOIL							
			op Dep		0.5	0.5	0.1	0.5	0.5	0.6	0.5	0.1
		Bot	tom Dep	oth(m):								
			Date Sai		17-Mar-15							
Determinand	Accred.	SOP	Units	LOD								
Aromatic TPH >C5-C7	N	2675	mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aromatic TPH >C7-C8	N	2675	mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aromatic TPH >C8-C10	М	2675	mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aromatic TPH >C10-C12	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C12-C16	М	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C16-C21	M	2675	mg/kg	1	< 1.0	< 1.0	3.9	< 1.0	< 1.0	< 1.0	< 1.0	2.6
Aromatic TPH >C21-C35	М	2675	mg/kg	1	< 1.0	< 1.0	11	< 1.0	< 1.0	< 1.0	< 1.0	11
Aromatic TPH >C35-C44	N	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Total Aromatic Hydrocarbons	M		mg/kg	5	< 5.0	< 5.0	15	< 5.0	< 5.0	< 5.0	< 5.0	15
Total Petroleum Hydrocarbons	М	2675	mg/kg	10	< 10	< 10	15	< 10	< 10	< 10	< 10	15
Naphthalene	М	2700	mg/kg	0.1	< 0.10	< 0.10	0.44	< 0.10	< 0.10	< 0.10	< 0.10	0.48
Acenaphthylene	M	2700	mg/kg	0.1	< 0.10	< 0.10	0.34	< 0.10	< 0.10	< 0.10	< 0.10	0.39
Acenaphthene	М		mg/kg	0.1	< 0.10	< 0.10	0.16	< 0.10	< 0.10	< 0.10	< 0.10	0.34
Fluorene	М	2700	mg/kg		< 0.10	< 0.10	0.15	< 0.10	< 0.10	< 0.10	< 0.10	0.45
Phenanthrene	М	2700	mg/kg	0.1	< 0.10	< 0.10	1.3	< 0.10	< 0.10	< 0.10	< 0.10	3.5
Anthracene	M		mg/kg		< 0.10	< 0.10	0.33	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Fluoranthene	М		mg/kg	0.1	< 0.10	< 0.10	3.1	< 0.10	< 0.10	< 0.10	< 0.10	5.5
Pyrene	М	2700	mg/kg	0.1	< 0.10	< 0.10	3.2	< 0.10	< 0.10	< 0.10	< 0.10	5.5
Benzo[a]anthracene	М	2700	mg/kg	0.1	< 0.10	< 0.10	1.6	< 0.10	< 0.10	< 0.10	< 0.10	2.6
Chrysene	М		mg/kg	0.1	< 0.10	< 0.10	2.3	< 0.10	< 0.10	< 0.10	< 0.10	3.3
Benzo[b]fluoranthene	М		mg/kg	0.1	< 0.10	< 0.10	1.9	< 0.10	< 0.10	< 0.10	< 0.10	3.5
Benzo[k]fluoranthene	М	2700	mg/kg		< 0.10	< 0.10	0.34	< 0.10	< 0.10	< 0.10	< 0.10	2.0
Benzo[a]pyrene	М		mg/kg	0.1	< 0.10	< 0.10	1.7	< 0.10	< 0.10	< 0.10	< 0.10	2.3
Indeno(1,2,3-c,d)Pyrene	М		mg/kg		< 0.10	< 0.10	0.54	< 0.10	< 0.10	< 0.10	< 0.10	0.78
Dibenz(a,h)Anthracene	M	2700	mg/kg	0.1	< 0.10	< 0.10	0.44	< 0.10	< 0.10	< 0.10	< 0.10	0.43
Benzo[g,h,i]perylene	М	2700	mg/kg	0.1	< 0.10	< 0.10	0.45	< 0.10	< 0.10	< 0.10	< 0.10	0.47
Total Of 16 PAH's	М		mg/kg	2	< 2.0	< 2.0	18	< 2.0	< 2.0	< 2.0	< 2.0	32
Benzene	M	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Toluene	M	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Ethylbenzene	М	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
m & p-Xylene	M	2760		1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
o-Xylene	M	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Methyl Tert-Butyl Ether	M	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0



# **Results Summary - Soil**

Client: HSP Consulting Engineers Limited		Chen	ntest Jo	b No.:	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828
Quotation No.:	CI	hemtes	st Samp	le ID.:	120111	120112	120113	120114	120115	120116	120117	120118
Order No.:		Client Sample Ref.:										
		Client Sample ID.:				WS2	WS3A	WS6	WS7	WS8	WS5	WS5
		Sample Type:			SOIL	SOIL						
		Top Depth (m):				0.5	0.1	0.5	0.5	0.6	0.5	0.1
		Bottom Depth(m):										
	Date Sampled:			17-Mar-15								
Determinand	Accred.	SOP	Units	LOD								
Total Phenols	M	2920	mg/kg	0.3	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30



# **Report Information**

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All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVCOs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

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Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

# **Sample Deviation Codes**

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container

# Sample Retention and Disposal

All soil samples will be retained for a period of 60 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: customerservices@chemtest.co.uk



# Chemtest The right chemistry to deliver results

Chemtest Ltd.
Depot Road
Newmarket
CB8 0AL
Tel: 01638 606070

Email: info@chemtest.co.uk

# **Final Report**

Report Number: 15-06833 Issue-1

Initial Date of Issue: 31-Mar-2015

Client: HSP Consulting Engineers Limited

Lawrence House

Meadowbank Way

Client Address: Eastwood

Nottinghamshire

NG16 3SB

Contact(s): Luke Bradley

Project: C2099 - Clitheroe

Quotation No.: Date Received: 25-Mar-2015

Order No.: Date Instructed: 25-Mar-2015

No. of Samples: 2

Turnaround: (Wkdays) 5 Results Due Date: 31-Mar-2015

Date Approved: 31-Mar-2015

Approved By:

**Details:** Darrell Hall, Laboratory Director



# **Results Summary - 2 Stage WAC**

Chemtest Job No: 15-06833 Chemtest Sample ID: 120134							Landfill Wa	aste Acceptar Limits	ce Criteria
Sample Ref: Sample ID: WS2 Top Depth(m): 0.5 Bottom Depth(m): Sampling Date: 17-Mar-2015 Determinand	SOP	Accred.	Units				Inert Waste Landfill	Stable Non- reactive Hazardous waste in non- hazardous	Hazardous Waste Landfill
Total Organic Carbon	2625	U	%			0.92	3	5	6
Loss on Ignition	2610	U	%			4.3			10
Total BTEX	2760	U	mg/kg			< 0.01	6		
Total PCBs (7 congeners)	2815	U	mg/kg			< 0.10	1		
TPH Total WAC (Mineral Oil)	2670	U	mg/kg			< 10	500		
Total (of 17) PAHs	2700	N	mg/kg			< 2.0	100		
pH	2010	U				7.1		>6	
Acid Neutralisation Capacity	2015	N	mol/kg			0.003		To evaluate	To evaluate
Eluate Analysis			2:1 mg/l	8:1 mg/l	2:1 mg/kg	Cumulative 10:1 mg/kg		s for compliar S EN 12457-3	nce leaching at L/S 10 l/kg
Arsenic	1450	U	< 0.001	0.002	< 0.050	< 0.050	0.5	2	25
Barium	1450	U	0.003	0.008	< 0.50	< 0.50	20	100	300
Cadmium	1450	U	0.00068	0.0035	< 0.010	0.033	0.04	1	5
Chromium	1450	U	0.004	0.011	< 0.050	0.11	0.5	10	70
Copper	1450	U	0.003	0.011	< 0.050	< 0.050	2	50	100
Mercury	1450	U	< 0.0005	< 0.0005	< 0.001	< 0.005	0.01	0.2	2
Molybdenum	1450	U	< 0.001	< 0.001	< 0.050	< 0.050	0.5	10	30
Nickel	1450	U	0.002	0.006	< 0.050	0.054	0.4	10	40
Lead	1450	U	0.001	0.005	< 0.010	0.048	0.5	10	50
Antimony	1450	U	< 0.001	< 0.001	< 0.010	< 0.010	0.06	0.7	5
Selenium	1450	U	0.001	< 0.001	< 0.010	< 0.010	0.1	0.5	7
Zinc	1450	U	0.033	0.14	< 0.50	1.3	4	50	200
Chloride	1220	U	4.6	1.3	< 10	14	800	15000	25000
Fluoride	1220	U	0.19	0.14	< 1.0	1.4	10	150	500
Sulphate	1220	U	4.9	< 1.0	< 10	< 10	1000	20000	50000
Total Dissolved Solids	1020	N	48	17	93	180	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.030	< 0.30	< 0.50	1	-	-
Dissolved Organic Carbon	1610	U	31	18	60	190	500	800	1000

Soild Information							
Dry mass of test portion/kg	0.175						
Moisture (%)	23						

Leachate Test Information							
Leachant volume 1st extract/l	0.296						
Leachant volume 2nd extract/l 1.4							
Eluant recovered from 1st extract/l	0.084						



# **Results Summary - 2 Stage WAC**

Chemtest Job No: 15-06833 Chemtest Sample ID: 120135							Landfill Wa	aste Acceptan Limits	ce Criteria
Sample Ref: Sample ID: WS4 Top Depth(m): 0.1 Bottom Depth(m): Sampling Date: 17-Mar-2015 Determinand	SOP	Accred.	Units				Inert Waste Landfill	Stable Non- reactive Hazardous waste in non- hazardous	Hazardous Waste Landfill
Total Organic Carbon	2625	U	%			3.5	3	5	6
Loss on Ignition	2610	U	%			9.4			10
Total BTEX	2760	U	mg/kg			< 0.01	6		
Total PCBs (7 congeners)	2815	U	mg/kg			< 0.10	1		
TPH Total WAC (Mineral Oil)	2670	U	mg/kg			37	500		
Total (of 17) PAHs	2700	N	mg/kg			34	100		
рН	2010	U				7.2		>6	
Acid Neutralisation Capacity	2015	N	mol/kg			0.003		To evaluate	To evaluate
Eluate Analysis			2:1 mg/l	8:1 mg/l	2:1 mg/kg	Cumulative 10:1 mg/kg		s for compliar S EN 12457-3	_
Arsenic	1450	U	0.002	0.001	< 0.050	< 0.050	0.5	2	25
Barium	1450	U	0.014	0.019	< 0.50	< 0.50	20	100	300
Cadmium	1450	U	0.0021	0.0015	< 0.010	0.015	0.04	1	5
Chromium	1450	U	0.005	0.007	< 0.050	0.064	0.5	10	70
Copper	1450	U	0.01	0.012	< 0.050	< 0.050	2	50	100
Mercury	1450	U	< 0.0005	< 0.0005	< 0.001	< 0.005	0.01	0.2	2
Molybdenum	1450	U	< 0.001	< 0.001	< 0.050	< 0.050	0.5	10	30
Nickel	1450	U	0.005	0.004	< 0.050	< 0.050	0.4	10	40
Lead	1450	U	0.01	0.012	0.019	0.12	0.5	10	50
Antimony	1450	U	< 0.001	< 0.001	< 0.010	< 0.010	0.06	0.7	5
Selenium	1450	U	0.001	0.001	< 0.010	0.011	0.1	0.5	7
Zinc	1450	U	0.077	0.05	< 0.50	0.51	4	50	200
Chloride	1220	U	4.1	2	< 10	21	800	15000	25000
Fluoride	1220	U	0.13	0.085	< 1.0	< 1.0	10	150	500
Sulphate	1220	U	15	2.2	29	28	1000	20000	50000
Total Dissolved Solids	1020	N	60	19	120	210	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.030	< 0.30	< 0.50	1	-	-
Dissolved Organic Carbon	1610	U	83	22	160	250	500	800	1000

Soild Information							
Dry mass of test portion/kg	0.175						
Moisture (%)	18						

Leachate Test Information							
Leachant volume 1st extract/l	0.313						
Leachant volume 2nd extract/l	1.4						
Eluant recovered from 1st extract/l	0.086						



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# **Appendix VI**

		-							Borehole N	lo.	
nsp					Borehole Log				CP1		
consulting								Sheet 1 of 1			
Projec	t Name:	Chatburn	Road,		Project No.		Co-ords:	<del>-</del>	Hole Type	е	
				C2099				CP			
Location: Clitheroe						Level:		Scale			
										1:50 Logged By	
Client:		Oakmere Homes Ltd					Dates:	17/03/2015 -17/03/2015	Driller		
Well	Water Strikes	Samples and In Situ Testing			Depth	Lovel		]			
		Depth (m)	Туре	Results	(m)	Level (m)	Legend	Stratum Description			
9 9,		0.00 - 0.20	В	results		, ,	3000000	Grass overyling brown clayey TOPS	OIL		
		0.10 0.20	D D		0.20		× × × × × × × × × × × × × × × × × × ×	Soft to firm orange brown mottled sil		-	
		0.20	В					CLAY.		-	
							$\overline{x}$				
		1.00		50 (7,16/50 for						1 -	
		1.00 - 1.50	В	270mm)			x	with a few large boulder and cobbles and	a little coarse	]	
							<del></del> ==	gravel.		_	
:   <del> </del>		1.60 1.70 - 1.80	D U		1.60			Stiff dark grey CLAY.			
		1.80		50 (25 for 85mm/5	0					2 -	
		1.80 - 2.30	В	for 290mm)						_ =	
: =: ;										=	
		2.80		50 (9,12/50 for 265mm)						3 —	
-		2.80 - 3.30	В	200,						3 –	
										-	
										=	
		3.80		50 (25 for 95mm/5	0					]	
		3.80 - 4.10	В	for 275mm)	4.10			LIMESTONE boulder.		4 —	
					4.20			End of borehole at 4.20 m			
										=	
										-	
										5 —	
										_	
										_	
										-	
										6 -	
										]	
										_	
										=	
										7 -	
										_	
										]	
										=	
										8 —	
										=	
										=	
										=	
										9 —	
										=	
										=	
			1		1	1	1			140	

Remarks

. No groundwater was encountered during the drilling proces

2. Borehole was terminated at 4.20m due to refusal

3. Gas and water monitoring standpipe installed to 4.20m depth.



		10							Borehole N	۱o.
nsp				Borehole Log				CP2		
consulting								Sheet 1 of 1		
Projec	t Name:	Chatburn Road,			Project No.		Co-ords:	: -	Hole Typ	е
					C2099				CP Scale	
Locati	on:	Clitheroe					Level:		1:50	
Client:		Oakmere	Homes	e l td			Dates:	18/03/2015 -19/03/2015	Logged B	Sy
01101111	· 				1	1	Batoo.	10,00,2010 10,00,2010	Driller	
Well	Water			In Situ Testing	Depth	Level	Legend	Stratum Description		
	Strikes	Depth (m)	Туре	Results	(m)	(m)	V//AV//AV			
		0.00 - 0.25 0.10	B D		0.25			Grass overlying silty TOPSOIL.		] :
		0.30 0.50 - 1.00	D B		0.20		×	Ornage brown mottled silty sandy C	LAY.	
		0.50 - 1.00					Xx-			-
		1.00		N=19 (1,1/3,4,6,6)			XX-			:
		1.00 - 1.50	В	14=13 (1,1/3,4,0,0)			XX-			1 -
							X——x			:
							×	with sandstone gravel and occasional co.	bbles.	-
		1.80 1.90 - 2.35	D U		1.80		×	Firm to stiff brown grey mottled CLA	Υ.	:
		1.90 - 2.33						to c 2.c g.c,c c		2 -
		2.40	D		2.40					:
		2.40	D		2.60			Firm to stiff brown grey mottled CLA	Y.	1 -
		2.70 2.70 - 3.20	В	N=42 (4,9/14,11,8,9	9)			Grey brown very sandy CLAY with r	nuch gravel	1 :
		2.70 - 3.20						and cobbles.		3 -
-: -: :		0.40			0.40					:
$\cdot : \exists :$		3.40 3.50 - 3.65	D U		3.40			Stiff grey gravelly CLAY. Gravel is o	f limestone	-
.: □: :		3.50 - 3.80	B U					San gray graverny SEXT. Stavorio S	i iiiiootorio.	-
		3.80 - 4.20	0							:
		4.20	D							4 -
										-
				( ( (						-
		4.80		50 (25 for 90mm/50 for 295mm)	)					:
		4.80 - 5.20	В	,						5 _
		5.30 5.30	D	50 (25 for 85mm/50	5.30					] :
				for 245mm)				Grey sandy CLAY.		-
		5.30 - 5.50	D		5.80			with limestone gravels, boulder and cobb	oles.	:
		5.30 - 5.80	В		6.00			LIMESTONE		6 _
		5.80 5.80	D	50 (25 for 90mm/50				End of borehole at 6.00 m		:
				for 250mm)						:
		6.00		50 (25 for 80mm/50 for 225mm)	'					-
										7 -
										-
										:
										8 –
										-
										:
										a
										9 –
										-
										-
										10 –

Remarks

. No groundwater was encountered during the drilling proces

2. Borehole was terminated at 6.00m due to refusa

3. Gas and water monitoring standpipe installed to 4.20m depth



0		n							Borehole N	No.
1126				Borehole Log				CP3		
consulting							1	<u> </u>	Sheet 1 of 1	
Projec	ct Name:	Chatburn I	Road,		Project No. C2099		Co-ords	: -	Hole Typ CP	е
					02099		Lovol		Scale	
Location: Clitheroe							Level:		1:50	
Client	:	Oakmere	Homes	s Ltd			Dates:	18/03/2015 - 18/03/2015	Logged B Driller	sy
Well	Water	Samples	s and l	n Situ Testing	Denth	LOVOI	Legend	Stratum Description		
vveii	Strikes	Depth (m)	Type B	Results	(m)	(m)				
		0.00 - 0.20 0.10 0.20	ם מ		U.∠U			Grass overyling brown clayey TOPS  Orange brown mottled silty sandy C		-
		0.20 - 0.70	B		0.70		×——×	-		-
		0.70 0.70 - 1.00	В		0.70		<u>~×</u> -	Grey brown mottled silty sandy CLA	Y with some	-
		0.80 1.00	W		1.00			gravel and cobbles.  Brown clayey GRAVEL.		1 -
.: Н.: ·		1.10 1.10 - 1.60	В	N=41 (4,5/8,11,14,8	5)					-
: H: 1		1.70	D		1.70					-
		1.80 - 2.20	U					Firm to stiff brown grey mottled CLA	Υ.	
: H: 3		2.20	D							2 -
: H: ;		2.40 2.50 - 2.70	D B		2.40					:
H								Stiff grey CLAY with many limestone	cobbles and	-
· F:		2.7 <b>0</b> .7 <b>0</b> .15 2.70 - 3.20	D B	N=33 (4,7/6,8,8,11)				boulders.		
: H: :										3
							~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~			-
		0.70 4.45					~~~~~			-
		3.70 - 4.15	U				~~~~~			:
		4.00					<u> </u>			4 -
		4.20	D				<u> </u>			-
		4.70		50 /05 for 00 mm /50			<u> </u>			:
		4.70		50 (25 for 80mm/50 for 250mm)	'		<u>~~~~</u>			
		4.70 - 5.20	В				<u></u>			5 -
							~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~			-
		E 70 E 0E	l							:
		5.70 - 5.85 5.70 - 6.20	U B							
: E							<u> </u>			6 _
							<u> </u>			-
							<u>~~~~</u>			-
		6.70		50 (25 for 135mm/50	6.70					] -
				for 228mm)				LIMESTONE		-
		6.70 - 6.80 7.00	D	50 (25 for 85mm/50	7.00			End of borehole at 7.00 m		7 -
				for 225mm)						-
										-
										-
										8 -
										-
										9 -
										-
										10 —

Remarks

No groundwater was encountered during the drilling process.

2. Borehole was terminated at 7.00m due to refusal.

3. Gas and water monitoring standpipe installed to 4.20m depth.



6	6 5							Trialpit I	No
Ш	5 P					Tri	ial Pit Log	TP1	I
con	sulting						_	Sheet 1	
Projec	ct Chatburr	n Road,		Projec			Co-ords: -	Date	
Name	:			C2099	9		Level:	18/03/20	
Locati	on: Clitheroe	)					Dimensions (m):	Scale 1:25	
Client	:: Oakmere	e Home	s Ltd				Depth 1.30	Logge LEB	d
	Sample	es and I	n Situ Testing	Damth	Laval		1.30	LED	
Water Strike	-			Depth (m)	Level (m)	Legend	Stratum Description		
Wat Nati	0.80 1.20	В	Results	(m) 0.25	(m)	Legell C	Grass overlying brown sandy clayey TOPSOIL plasticity.  Grey slightly clayey sandy GRAVEL & COBBLE and cobbles is fine to coarse angular to sub an sandstone.  End of pit at 1.30 m	S. Gravel	1
Rema	rks: 1 No.	aroundw	rater was encountered du	ring the e	excavation	nrocess			5—

2. Trial pit was terminated at 1.30m depth and backfilled with arisings.

0 6 0								Trialpit N	٧o
n	Sp					Tri	ial Pit Log	TP1E	3
con	sulting						_	Sheet 1 c	of 1
Projec	t Chatburr	n Road,		Projec			Co-ords: -	Date	
Name				C2099	9		Level: Dimensions	18/03/20 ⁻ Scale	
Locati	on: Clitheroe	<del>)</del>					(m):	1:25	
Client	: Oakmere	Home	s Ltd				Depth 0.50	Logged	
er	Sample	s and I	n Situ Testing	Depth	Level	Lagana	Stratum Decembrian		
Water Strike	Depth	Туре	Results	(m)	(m)	Legeno	Stratum Description		
We Stri	Depth	Type	Results	0.15 0.50	(m)		Grass overlying blackish brown slightly gravelly sandy clayey TOPSOIL. High plasticity. Gravel medium angular to sub angular of sandstone.  Firm orange brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angular angular of sandstone and mudstone.  Find of pit at 0.50 m	is fine to	2
Rema	rks: 1. No c	groundw	ater was encountered dur	ing the d	rilling pro	ocess.			5—

2. Trial pit was terminated at 0.50m depth due to bedrock and backfilled with  $% \left( 1\right) =\left( 1\right) \left( 1\right)$  arisings.

								Trialpit N	٧o
n	SD					Tri	al Pit Log	TP1	
	sulting						_	Sheet 1 c	of 1
Projec Name		า Road,		Project C209			Co-ords: -	Date 18/03/20	4.5
				C209	9		Level: Dimensions	18/03/20 Scale	
Locati	on: Clitheroe	<i>;</i>					(m):	1:25	
Client	: Oakmere	Home	s Ltd				Depth	Logged	
							0.50	LB	
Water Strike	Depth	Type	n Situ Testing  Results	Depth (m)	Level (m)	Legend	Stratum Description		
				0.15			Grass overlying blackish brown slightly gravelly sandy clayey TOPSOIL. High plasticity. Gravel medium angular to sub angular of sandstone.  Firm orange brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angular angular of sandstone and mudstone.  ———————————————————————————————————	is fine to CLAY.	1   1   1   1   1   1   1   1   1   1
Poma	rlen 1 No.	aroundu	vater seenage was encol	intered di	ring the	veovetio	n process		

2. Trial pit was terminated at 0.50 m depth due to bedrock and backfilled with arisings.



	6 5							Trialpit I	No
	5 P					Tr	ial Pit Log	TP2	<u>.</u>
con	sulting						9	Sheet 1	of 1
Projec	ct Chatburr	n Road,		Projec			Co-ords: -	Date	
Name	:			C2099	9		Level:	18/03/20	
Locati	on: Clitheroe	)					Dimensions (m):	Scale 1:25	
Client	:: Oakmere	Home	e l td				Depth	Logge	
Cilein					I		2.00	LEB	
Water Strike			n Situ Testing	Depth	Level	Legend	Stratum Description		
Str Str	Depth	Туре	Results	(m)	(m)	N//AN///A	Cross quarking blooking brown and player.	ODCOII	
v v v v v v v v v v v v v v v v v v v	0.80	В		2.00			Grass overlying blackish brown sandy clayey Low plasticity.  Firm orange brown slightly gravelly sandy CLAY plasticity. Gravel is fine to coarse angular to su of sandstone and mudstone.  HSV - 24 kPa at 0.50m depth.  HSV - 32 kPa at 1.50m depth.  End of pif at 2.00 m	7. High ab rounded	1   1   1   1   1   1   1   1   1   1
Romo	rke: 1 No.	arounde	rater was encountered du	ring the d	rilling pr	DOSS.			5—

2. Trial pit was terminated at 2.00m depth and backfilled with arisings.

								Trialpit N	No.
n	5 D					Tri	al Pit Log	TP3	
:011	sulting						_	Sheet 1 c	of 1
Projed Name	Ct Chatburr	n Road,		Projec			Co-ords: -	Date	
INAIIIE	<b>.</b> .			C2099	9		Level: Dimensions	18/03/20	
Locat	ion: Clitheroe	;					(m):	Scale 1:25	
Client	t: Oakmere	Home	s I td				Depth	Logged	t
Oncin	1						1.70	LEB	
Water Strike	Sample		n Situ Testing	Depth	Level	Legend	StratumDescription		
Str S	Depth	Type	Results	(m)	(m)				
							Grass overlying blackish brown sandy clayey T High plasticity.		-
				0.15			Firm orangish brown sandy gravelly CLAY. High plasticity. Gravel is fine to coarse angular to sub	) rounded	=
							of sandstone and mudstone.	Tourided	=
	0.50	В							=
	0.50	ь							=
							HSV - 48 kPa at 0.60m depth.		=
									=
									. =
									1—
									=
•									=
		_					with low sub rounded boulders of sandstone at 1.40m dep	th.	=
	1.50	В					HSV - 14 kPa at 1.50m depth.		=
				1.70			End of pit at 1.70 m		=
									=
									_
									2—
									-
									=
									-
									=
									-
									3
									-
									3—
									=
									=
									-
									=
									=
									-
									_ =
									4-
									=
									=
									=
									-
									-
									_ =
<u> </u>	4 00	h4 a	divintaria e e e e e		ali india		tion process at 4.20cm, depth		5— ——
Rema	ırks: 1. Sliql	nt groun	uwater seepage was en	countered	auring th	e excava	tion process at 1.30m depth.	1	

2. Trial pit was terminated at 1.70m depth and backfilled with arisings.

Trial Pit Log  Sheet 1 of 1 Date Name: Chatburn Road.  Collection: Cittherce  Depth 1.00		CD							Trialpit	No
Sheet 1.01	Ш	5 P					Tri	al Pit Log	TP4	
Name: Chatarum Road.  Cititoroo Scale  Cititoroo Depth   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170   170	con	sulting						3	Sheet 1	of 1
Directions   Dir	Projec	t Chatburr	n Road,							
Citient   Collement   Collem	Name	<u> </u>			C209	9				
Client: Cakmere Homes Ltd  Samples and In Situ Testing Depth Type Results  O.15  O.50  Depth Type Results  O.15  Depth Type Results  Depth Legend  Stratum Description  Grass overlying blacksich brown andrdy gravelly clayvey and under the subdivision of sendations.  Type Type Results on Sendations.  Type T	Locati	on: Clitheroe	)							
Samples and in Situ Testing Depth Type Results  0.15  0.15  0.50 B  0.50 B  1.90  Depth Type Results  0.15  Depth Type Results  Depth Level Legend  Type Results  Depth Type Results  Depth Level Legend  Type Results  Depth Type Results  Depth Type Results  Depth Level Legend  Type Results  Depth Type Results  Depth Type Results  Depth Level Legend  Type Results  Depth Type Results  Depth Type Results  Depth Type Results  Depth Level Legend  Type Results  Depth Type Resul	Client	: Oakmere	e Homes	s Ltd					Logge	
Depth Type Results (h) (m) (m) (m) (m) (m) (m) (m) (m) (m) (m					Τ	l		1.90	LB	
Grass overlying blackish brown sandy gravely (aleys)  ORDIL High plasticity, Gravel is fine to medium angular to sub angular of sandstone,  TOP-SILL High plasticity, Gravel is fine to coarse angular to sub rounded of sandstone and mudstone.  TOP-SILL HIGH plasticity, Gravel is fine to coarse angular to sub rounded of sandstone and mudstone.  THOW SILVERS WITH ROUND TO SUBSILIE TO SUBSILI	Water Strike						Legend	Stratum Description		
nemains. It in groundwater was encountered during the drilling process.	Rema			ater was encountered	1.90	drilling pr	ocess.	TOPSOIL. High plasticity. Gravel is fine to med angular to sub angular of sandstone.  Firm orangish brown sandy gravelly CLAY. Hig plasticity. Gravel is fine to coarse angular to sub of sandstone and mudstone.  HSV - 38 kPa at 0.50m depth. with a medium sub rounded boulder of sandstone.  HSV - 22 kPa at 1.40m.	dium h	2

2. Trial pit was terminated at 2.00m depth and backfilled with arisings.

	c n							Trialpit N	10
	5 P					Tr	ial Pit Log	TP5	,
con	sulting						•	Sheet 1 c	of 1
Projec Name		n Road,		Projec			Co-ords: -	Date	
INAIIIE	•			C2099	9		Level: Dimensions	18/03/20 ⁻ Scale	
Locati	on: Clitheroe	)					(m):	1:25	
Client	: Oakmere	e Home	s Ltd				Depth 2.10	Logged	
	Sample	s and l	n Situ Testing	I	l		2.10	LB	
Water Strike				Depth (m)	Level (m)	Legend	Stratum Description		
We We Str	0.50  1.60 1.90	B B	Results	1.90 2.10	(m)		Grass overlying blackish brown sandy gravelly TOPSOIL. High plasticity. Gravel is fine to med angular to sub angular of sandstone.  Firm orange brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angular rounded of sandstone and mudstone.  HSV - 63 kPa at 0.50m.  HSV - 46 kPa at 1.10m depth. with medium angular to sub rounded sandstone.  Greyish brown clayey sandy GRAVEL. Gravel is fine to medium angular to rounded of sandstone.  End of pit at 2.10 m	ium CLAY. to sub	2 3 3 4 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Rema	rke 1 No.	aroundw	ater was encountered d	uring the e	excavatio	n process	5.		5—
Cilid	11.0	5 a. 14W	a.a. maa onoodintered u	~g alo c	avalio	p. 00033	• •	· · · · · · · · · · · · · · · · · · ·	_

2. Trial pit was terminated at 2.10m depth and backfilled with arisings.

0	CD							Trialpit No
	<b>5 P</b>					Tri	ial Pit Log	TP6
con	sulting						3	Sheet 1 of 1
Projec		n Road,		Projec			Co-ords: -	Date
Name	:			C209	9		Level:	18/03/2015
_ocati	on: Clitheroe	•					Dimensions (m):	Scale 1:25
Client	: Oakmere	Homes L	td				Depth	Logged
	Sample	e and In S	Situ Testing	Ī	Ι		2.50	LB
Water Strike	Depth	Туре	Results	Depth (m)	Level (m)	Legend	d Stratum Description	
<b>V</b>	0.50	В		1.00			Grass overlying blackish brown sandy gravelly TOPSOIL. High plasticity. Gravel is fine to med angular to sub angular of sandstone.  Firm orange brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angular angular ofsandstone.  HSV - 56 kPa at 0.60m.  Firm greyish brown sandy gravelly CLAY. High Gravel is fine to coarse angular to sub rounded sandstone.	CLAY. to sub
	2.20	В		2.10 2.30 2.50			Stiff dark grey silty sandy gravelly CLAY. Low Gravel is fine to coarse angular to sub rounded sandstone.  Dark grey sandy very clayey GRAVEL. Gravel is coarse angular to sub rounded of sandstone.  End of pit at 2.50 m	d of
Rema	rko: 1 No.	aroundwata	er was encountered d	luring the c	veavatio	n process		3-
\€IIId	INO. I. INU (	g, our iuwalt	,, was choodileted t	anny uic t	mouvali0	610069	··	I

2. Trial pit was terminated at 2.50m depth and backfilled with arisings.



	6 5							Trialpit N	10
	5 p					Tr	ial Pit Log	TP7	,
con	sulting						•	Sheet 1 o	of 1
Projec Name		n Road,		Projec			Co-ords: -	Date	
INAIIIE				C2099	9		Level: Dimensions	18/03/20 Scale	
Locati	on: Clitheroe	)					(m):	1:25	
Client	: Oakmere	e Home	s Ltd				Depth 1.10	Logged LB	
e e	Sample	s and I	n Situ Testing	Depth	Level				
Water Strike	Depth	Туре	Results	(m)	(m)	Legend			
	0.50	В		0.30			Grass overlying blackish brown slightly gravelly sandy clayey TOPSOIL. High plasticity. Gravel coarse sub angular to sub rounded of sandstor Firm light grey to orangish brown slightly grave sandy CLAY. High plasticity. Gravel is fine to mangular to sub angular of sandstone and mudst HSV-32 kPa at 0.60m depth.  HSV-32 kPa at 0.60m depth.  End of pit at 1.10 m  Find of pit at 1.10 m	is fine to ne. Ily very edium	3 3 4 4
Rema	rks: 1. Slig	ht groun	dwater seepage was er	 ncountered	during th	ne excava	ation process at 0.70m depth.		5—

2. Trial pit was terminated at 1.10m depth due to collapsing and backfilled with arisings.

<b>a</b>	ch							Trialpit N	No
Ш	5 P					Tri	al Pit Log	TP8	
con	sulting						<b>)</b>	Sheet 1 o	of 1
Projec Name:		า Road,		Project C209			Co-ords: - Level:	Date 18/03/20	15
_ocati	on: Clitheroe	 }					Dimensions	Scale	
							(m): Depth	1:25 Logged	
Client	: Oakmere	e Homes I	∟td 				2.50	LB	
ke fe	Sample	s and In S	Situ Testing	Depth	Level	Legend	Stratum Description		
Water Strike	Depth	Туре	Results	(m)	(m)	Z/GOV/			
•	0.50	В		0.25			Grass overlying blackish brown slightly gravell sandy clayey TOPSOIL. High plasticity. Grave coarse sub angular to sub rounded of sandston Firm greyish brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angular rounded of sandstone and mudstone.  HSV - 48 kPa at 0.50m depth. with medium angular to sub rounded sandstone gravel at depth.	l is fine to ne. CCLAY. to sub	
	1.50	В					HSV - 38 kPa at 1.30m depth. becoming softer with increased depth.		
	2.40	В		2.50			End of pit at 2:50-m		2
									3-
									4
Rema	rks: 1. Slig	ht groundw	vater seepage was e	encountered	during th	ne excava	tion process at 0.80m depth.		5_

2. Trial pit was terminated at 2.50m depth and backfilled with arisings.



6	CD							Trialpit	No
Ш	<b>5 P</b>					Tri	al Pit Log	TPS	)
con	sulting							Sheet 1	of 1
Projec Name		n Road,		Projec			Co-ords: -	Date	
				C209	9		Level: Dimensions	18/03/20 Scale	
Locati	on: Clitheroe	Э					(m):	1:25	
Client	: Oakmer	e Homes l	_td				Depth 2.50	Logge LB	d
<u>-</u> 0	Sample	es and In S	Situ Testing	Depth	Level			LD	
Water Strike	Depth	Туре	Results	(m)	(m)	Legend	StratumDescription		
•	0.60	В		0.20			Grass overlying blackish brown slightly gravel sandy clayey TOPSOIL. High plasticity. Grave medium angular to sub angular of sandstone.  Firm greyish brown slightly gravelly very sandy High plasticity. Gravel is fine to coarse angula angular of sandstone and mudstone.  HSV - 61 kPa at 0.40m depth. becoming very gravelly at 0.70m depth.	I is fine to CLAY.	1 —
	1.50	В		1.20			Firm dark grey slightly sandy gravelly CLAY. L plasticity. Gravel is fine to coarse angular to st of sandstone.  HSV - 28 kPa at 1.40m depth. with a low sub rounded boulder content of sandstone at depth.	ub angular	2-
	2.20	В		2.50			End of pit at 2:50 m		
									4-
Rema	rks: 1. Slic	ht groundw	rater seepage was e	encountered	during th	ne excava	tion process at 1.00m depth.		5-

2. Trial pit was terminated at 2.50m depth and backfilled with arisings.

6	CD							Trialpit N	No
Щ	2 P					Tri	ial Pit Log	TP10	)
con	sulting						<u> </u>	Sheet 1 d	of 1
Projec		n Road,		Projec			Co-ords: -	Date	
Name	); 			C209	9		Level: Dimensions	18/03/20 Scale	
Locati	ion: Clitheroe	)					(m):	1:25	
Client	t: Oakmere	e Homes	s Ltd				Depth 2.60	Logged	t
<u> </u>	Sample	s and Ir	n Situ Testing	Don't			2.00	LB	
Water Strike	Depth	Туре	Results	Depth (m)	Level (m)	Legend	Stratum Description		
> 00	2 9 7 11	.,,,,		0.25			Grass overlying blackish brown slightly gravel sandy clayey TOPSOIL. High plasticity. Grave medium angular to sub angular of sandstone.	l is fine to	-
				0.23			Firm orangish brown slightly gravelly very san High plasticity. Gravel is fine to coarse angula rounded of sandstone.  HSV - 52 kPa at 0.50m depth.		- - - -
	0.60	В					with medium angular to sub rounded content of sandstor depth.	ne at 0.80m	-
							with low angular to sub rounded content of sandstone at	1 20m	1 —
							depth.	1.2011	-
	1.50	В					HSV - 40 kPa at 1.40m depth.		_
	1.50						74 		_
							دوا . الله الله الله الله الله الله الله ا		-
							or o		-
									2_
									-
									-
	2.40	В					9 d		-
				2.60			End of pit at 2:60 m		-
									_
									-
									3—
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									-
									-
									-
									-
									-
									4—
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									-
									-
									-
									5—
Rema	rks: 1. No	groundwa	ater was encountered	during the	excavatio	n process			-

2. Trial pit was terminated at 2.60m depth and backfilled with arisings.

		n						_	Borehole N	lo.
	2	P				Boi	reho	ole Log	WS1	
con	sult	ing						<b>J</b>	Sheet 1 of	
Proje	ct Name:	Chatburn I	₹oad,		Project No. C2099		Co-ords	; <del>-</del>	Hole Type WS	Э
_ocat	ion.	Clitheroe			02000		Level:		Scale	
							2010		1:50 Logged By	v
Client	:	Oakmere I	Homes	s Ltd			Dates:	17/04/2015 - 17/04/2015	LEB	<b>,</b>
Well	Water Strikes			In Situ Testing	Depth	Level	Legend	StratumDescription	1	
(/A)\(\)	Strikes	Depth (m)	Туре	Results	(m)	(m)	NV/ANV/ANV	MADE GROUND - blackish brown		
		0.15	D		0.15			clayey TOPSOIL. Lowplasticity. Firm orangish brown slightly gravelly	/	=
		0.50	D					CLAY. High plasticity. Gravel is fine subangular to angular of sandstone	to coarse	=
		1.00		N=12 (0,0/1,2,3,6	6)			HSV - 46 kPa at 0.70m depth.		1 _
					,					· <u>-</u>
		1.50	D							_
		1.75	ES		1.75 1.80			Grey SANDSTONE. Recovered as	a coarse	=
								i_ <u>angular gravel.</u> End of borehole at 1.80 m		2 —
										=
										=
										3 —
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Remarks
1. No groundwater was encountered during the drilling process.

2. Borehole was terminated at 1.80m depth due to refusal and backfilled with arisings.



	6	<b>1</b>							Borehole N	lo.
	5	þ				Boi	reho	ole Log	WS2	
con	sult	ing			Drain at Na		1		Sheet 1 of	
Projec	t Name	Chatburn I	Road,		Project No. C2099		Co-ords:	: -	Hole Type WS	3
Locat	ion:	Clitheroe					Level:		Scale	
LUCAL	1011.	Cittieroe					Level.		1:50	.,
Client	:	Oakmere I	Homes	s Ltd			Dates:	17/04/2015 -17/04/2015	Logged B LEB	У
Well	Water			In Situ Testing	Depth	Level	Legend	StratumDescription		
W CII	Strikes	Depth (m)	Туре	Results	(m)	(m)	377637763	Grass overlying blackish brown very	, candy	
		0.10	D		0.20			clavey TOPSOIL, Low plasticity.		-
		0.50	D					Firm orangish brown slightly gravelly CLAY. High plasticity. Gravel is fine	very sandy to coarse sub	_
	_							angular to angular of sandstone.  HSV - 42 kPa at 0.80m depth.		-
		1.00		N=10 (1,1/2,2,3,3)						1 –
		4.50	_							=
		1.50	D							-
		2.00		N=16 (2,3/4,4,4,4)				HSV - 62 kPa at 1.80m depth		2 —
				- ( )	2.10			Firm greyish brown sandy gravelly of plasticity. Gravel is fine to coarse su	CLAY. High	=
·		2.50	D	( (				angular of sandstone and mudstone  HSV - 52 kPa at 2.40m depth.		_
		2.50		50 (25 for 105mm/5 for 255mm)	2.80			End of borehole at 2.80 m		-
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No groundwater was encountered during the drilling proce

2. Borehole was terminated at 2.50m depth due to refusal.

. Gas and water monitoring standpipe installed to 2.50m depth.



0	C	n							Borehole N	lo.
	>	P				Boi	reho	ole Log	WS3	
con	sult	ing			Project No.				Sheet 1 of Hole Type	
Projec	ct Name:	Chatburn I	Road,		C2099		Co-ords:	: -	WS	5
Locat	ion:	Clitheroe					Level:		Scale	
Client	:	Oakmere	 Homes	s Ltd			Dates:	17/04/2015 -17/04/2015	1:50 Logged B LEB	У
\A/~!I	Water	Sample	s and	In Situ Testing	Depth	Level	1	Ctratura Description		
vveii	Strikes	Depth (m)	Туре	Results	(m)	(m)	Legena			
				T.	(m) 0.20		Legend	Grass overlying blackish brown ventopsoll. Lowplasticity.  Firm light grey and oranish brown satisfirm light grey and oranish brown light grey light gre	y sandy clay	3
										8
										10 —

Remarks

1. No groundwater was encountered during the drilling process.

2. Borehole was terminated at 1.80m depth due to refusal and backfilled with arisings.



	C	n							Borehole No.
	2	Ρ				Boi	reho	ole Log	WS3A
	s u l t ct Name		Road,		Project No.		Co-ords:		Sheet 1 of 1 Hole Type WS
Looot	ion	Clitheroe			C2099		Loveli		Scale
Locat	ion:	Ciltneroe					Level:		1:50
Client	:	Oakmere				1	Dates:	17/04/2015 -17/04/2015	Logged By LEB
Well	Water Strikes		1	In Situ Testing	Depth (m)	Level (m)	Legend	StratumDescription	
		Depth (m) 0.10	Type D	Results	0.15	()		Grass overlying blackish brown san	dy clayey
		0.10 0.50 1.00 1.40 1.60	D D	N=16 (1,1/3,4,5,4 50 (25 for 135mm/	50 1.60			TOPSOIL. Highplasticity.  Firm light grey and oragnish brown gravelly sandy CLAY. High plasticit to medium sub rounded to angular and mudstone. with a cobble of sandstone at 1.50m depth.	y. Gravel is fine of sandstone
<u>(1334)</u>		1		for 235mm)	1.70			Extremely weak grey medium grain SANDSTONE recovered as angula	ed
								Land of borehole at 1.70 m	
									3 -
									4 -
									5 -
									6 -
									7 -
									8 -
									9-

#### Domorko

- 1. No groundwater was encountered during the drilling process.
- 2. Borehole was terminated at 1.70m depth due to refusal and backfilled with arisings.



	C	n				_	_	_	Borehole N	10.
	2	P				Boi	reho	ole Log	WS4	
con	sulti	ng			Droin at Na				Sheet 1 of	
Projec	t Name:	Chatburn I	Road,		Project No. C2099		Co-ords	: -	Hole Type WS	е
Locat	ion:	Clitheroe			02000		Level:		Scale	
Locat		Charletoe							1:50 Logged B	V
Client	:	Oakmere I	Homes	s Ltd			Dates:	17/04/2015 - 17/04/2015	LEB	y
Well	Water Strikes			In Situ Testing	Depth	Level (m)	Legend	StratumDescription	ı	
V/834/	Strikes	Depth (m)	Туре	Results	(m)	(111)	X(/A)X(/A)X	Grass overlying blackish brown slig	ihtly gravelly	
		0.10	D		0.20			sandy clay TOSPOIL. High plasticit fine to medium angular to sub angu	v. Gravel is	
		0.50	D					sandstone Firm orangish brown slightly gravelly	y sandy	
		1.00		N=5 (1,1/1,2,1,1)				CLAY. High plasticity. Gravel is fine angular to sub angular of sandstone	e to medium	1 —
		1.00		11-0(1,1/1,2,1,1)				mudstone.  HSV - 44 kPa at 0.80m depth.		-
		1.50	D							_
										-
		2.00		N=31 (4,6/5,7,8,11)	)					2 _
		2.50	D		2.30			Stiff dark grey silty sandy gravelly C		
		2.70		35 (25 for 115mm/3	2.70			plasticity. Gravel is fine to coarse a rounded of sandstone and mudstor	ngular to sub ne.	-
				for 245mm)				End of borehole at 2.70 m	<i>-</i> /	3
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- 1. No groundwater was encountered during the drilling process.
- 2. Borehole was terminated at 2.70m depth due to refusal and backfilled with arisings.



		n							Borehole N	lo.
- St	2	P				Boi	reho	ole Log	WS5	
con	sulti	ing			Project No.		_		Sheet 1 of	
Proje	ct Name:	Chatburn I	Road,		Project No. C2099		Co-ords	: -	Hole Type WS	3
_ocat	ion:	Clitheroe					Level:		Scale	
Client		Oakmere l	 Homes	s Ltd			Dates:	17/04/2015 -17/04/2015	1:50 Logged B	у
	Water	Samples	s and	In Situ Testing	Depth	Level		2		
Well	Water Strikes	Depth (m)	Туре	Results	(m)	(m)	Legend	-		
		0.10	D		0.20			Grass overlying brown slightly sand very clayey TOPSOIL. High plastici	ty. Gravel is	-
		0.50	D					fine to medium angular to sub angusandstone.	1	-
								Firm yellowish brown and grey sligh very sandy CLAY. High plasticity. Gr	avel is fine to	-
		1.00		N=5 (3,2/2,1,1,1)				coarse angular to sub rounded ofsa mudstone. HSV - 19 kPa at 0.90m depth.	andstone and	1 _
		1.50	D					becoming soft from 1.20m depth.		=
2000					1.70			End of borehole at 1.70 m		- -
		2.00		50 (25 for 135mm/s for 275mm)	50					2
				,						=
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Remarks
1. Groundwater was encountered during the drilling process at 1.10m depth.

2. Borehole was terminated at 1.70m depth due to refusal and backfilled with arisings.



h	S	р					coho		Borehole N WS6	lo.
n o n	sult	ing				DUI	GIII	ole Log		4
	ct Name:		Poad		Project No.		Co-ords:		Sheet 1 of Hole Type	
10,00	il Maine.	. Chaibuin i	Noau,		C2099		CO-oras.		WS	
Locat	ion:	Clitheroe					Level:		Scale 1:50	
Client	:	Oakmere I			1		Dates:	17/04/2015 -17/04/2015	Logged B	у
Well	Water			In Situ Testing	Depth	Level	Legend	StratumDescription		
WI IN	Strikes		Туре	Results	(m)	(m)	**************************************	Grass overlying blackish brown slig		
		0.10	D		0.15			sandy clayey TOPSOIL. High plasti fine to medium sub angular of sand	city. Gravel is	
		0.50	D					mudstone.  Soft light grey to orangish brown sli	//	
		1.00		N=14(1,2/2,3,4,5	5) 1.20			very sandy CLAY. High plasticity. G coarse angular to sub angular of sal mudstone.	Gravel is fine to	1 —
	*	1.50	D		1.20			HSV - 32 kPa at 0.60m depth. Dark grey gravelly clayey SAND. So coarse. Gravel is fine to coarse ang	ular to sub	
		1.90		50 (25 for 85mm/24,12,14,	1.90			angular of sandstone and mudstone	<b>).</b>	2_
										3 - 3 - 4
										9

. Groundwater was encountered during the drilling process at 1.00m dept

2. Borehole was terminated at 1.90m depth due to refusa

Gas and water monitoring standpipe installed to 1.90m depth.



0 6	n				_	_		Borehole No.
11 2	P				Bo	reho	ole Log	WS7
consul	ting			N==:=== N:=				Sheet 1 of 1
Project Nam	e: Chatburn	Road,		Project No. C2099		Co-ords:	<del>-</del>	Hole Type WS
Location:	Clitheroe					Level:		Scale 1:50
Client:	Oakmere	Homes	s Ltd			Dates:	17/04/2015 -17/04/2015	Logged By LB
Well Water Strikes			In Situ Testing	Depth	Level	Legend	StratumDescription	
Strikes	2 op ()	Туре	Results	(m)	(m)	N///N///	Grass overlying blackish brown slic	ihtly gravelly
	0.10 0.50	D D		0.25			sandy clayey TOPSOIL. High plast fine to medium sub angular of sand	icity. Gravel is
	0.50						mudstone. Firm light brown and orangish brown gravelly CLAY. High plasticity. Grave	n sandy
	1.00		N=10 (3,3/4,2,2,2)				medium angular to sub rounded of s and mudstone.  HSV - 42 KPa at 0.70m depth.	sandstone 1 —
	1.50	D		1.50			Soft reddish brown slightly gravelly	very sandy
	2.00		N=7 (2,2/1,2,1,3)				CLAY. High plasticity. Gravel is fine angular to sub angular of sandston mudstone.	e to coarse e and
							HSV - 24 kPa at 1.60m depth.	- - -
	2.70		N=50 (11,12/50 for 285mm)	2.70			End of borehole at 2.70 m	
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								-
								6 —
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								10 —
Remarks		Ь						

- 1. Groundwater was encountered during the drilling process at 1.40m depth.
- 2. Borehole was terminated at 2.70m depth due to refusal and backfilled with arisings.



	<b>n</b>				_	_		Borehole No.
	<b>9</b>				Boı	reho	ole Log	WS8
consulti	ng			<u> </u>			9	Sheet 1 of 1
Project Name:	Chatburn F	≀oad,		Project No. C2099		Co-ords:	-	Hole Type WS
Location:	Clitheroe			02000		Level:		Scale
Location.	Cillileide					Level.		1:50
Client:	Oakmere H					Dates:	17/04/2015 -17/04/2015	Logged By LB
Well Water Strikes	-		In Situ Testing	Depth (m)	Level (m)	Legend	StratumDescription	
Well Strikes	Depth (m)  0.50  1.00  1.80	Type	N=5 (1,1/1,1,1,2) N=50 (11,14/50 for 285mm)		(m)	Legello	Grass overlying brown sandy grave TOPSOIL. High plasticity. Gravel is medium angular to sub rounded of and mudstone.  Firm orangish brown slightly gravelly CLAY. High plasticity. Gravel is fine to angular to sub rounded of sandstone.  HSV - 68 kPa at 0.70m depth.  End of borehole at 1.80 m.	silly clayey fine to sandstone yvery sandy o coarse
Pomorko								10 —
Remarks								

- 1. No groundwater was encountered during the drilling process.
- 2. Borehole was terminated at 1.80m depth due to refusal and backfilled with arisings.



									Borenoie No.	
N	Sulti	<u>D</u>				Bo	reh	ole Log	WS9	
UI	sulti	ng			Drojoot No		1		Sheet 1 of 1 Hole Type	
Proje	ct Name:	Chatburn F	₹oad,		Project No.		Co-ords	: -		
					C2099				WS Scale	
_ocat	ion:	Clitheroe					Level:		1:50	
Client	:	Oakmere I	Homes	s Ltd			Dates:	17/04/2015 - 17/04/2015	Logged By LB	
Well	Water Strikes			In Situ Testing	Depth (m)	Level (m)	Legend	StratumDescription	1	
778SV7	Otrikes	Depth (m)	Туре	Results	(111)	(111)	W//AW//AW	Grass overlying brown sandy grave	ally clavey	
					0.30			TOPSOIL. High plasticity. Gravel is	s fine to	-
		0.50	D		0.00			medium angular to sub angular of s mudstone.	sandstone and	_
								Firm orangish brown slightly gravell	y very sandy	-
		1.00		N=8 (2,2/2,2,2,2)	,			CLAY. High plasticity. Gravel is fine to angular to sub rounded of sandstor		1 —
		1.00		(2,2,2,2,2,2,	1.20			mudstone.  HSV - 42 kPa at 0.80m depth.		· -
		1.50	D				$\overline{X} = \overline{X} + \overline{X}$	Firm greenish brown silty sandy gra	avelly CLAY.	=
		1.50					X-1-X	High plasticity. Gravel is fine to coasub angular of sandstone.  HSV - 79 kPa at 1.50m depth.	irse angular to	-
		2.00		N=50 (4,6/50 for 255mm)	2.00			End of borehole at 2.00 m	2	2
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1. No groundwater was encountered during the drilling process.

2. Borehole was terminated at 2.0m depth due to refusal and backfilled with arisings.



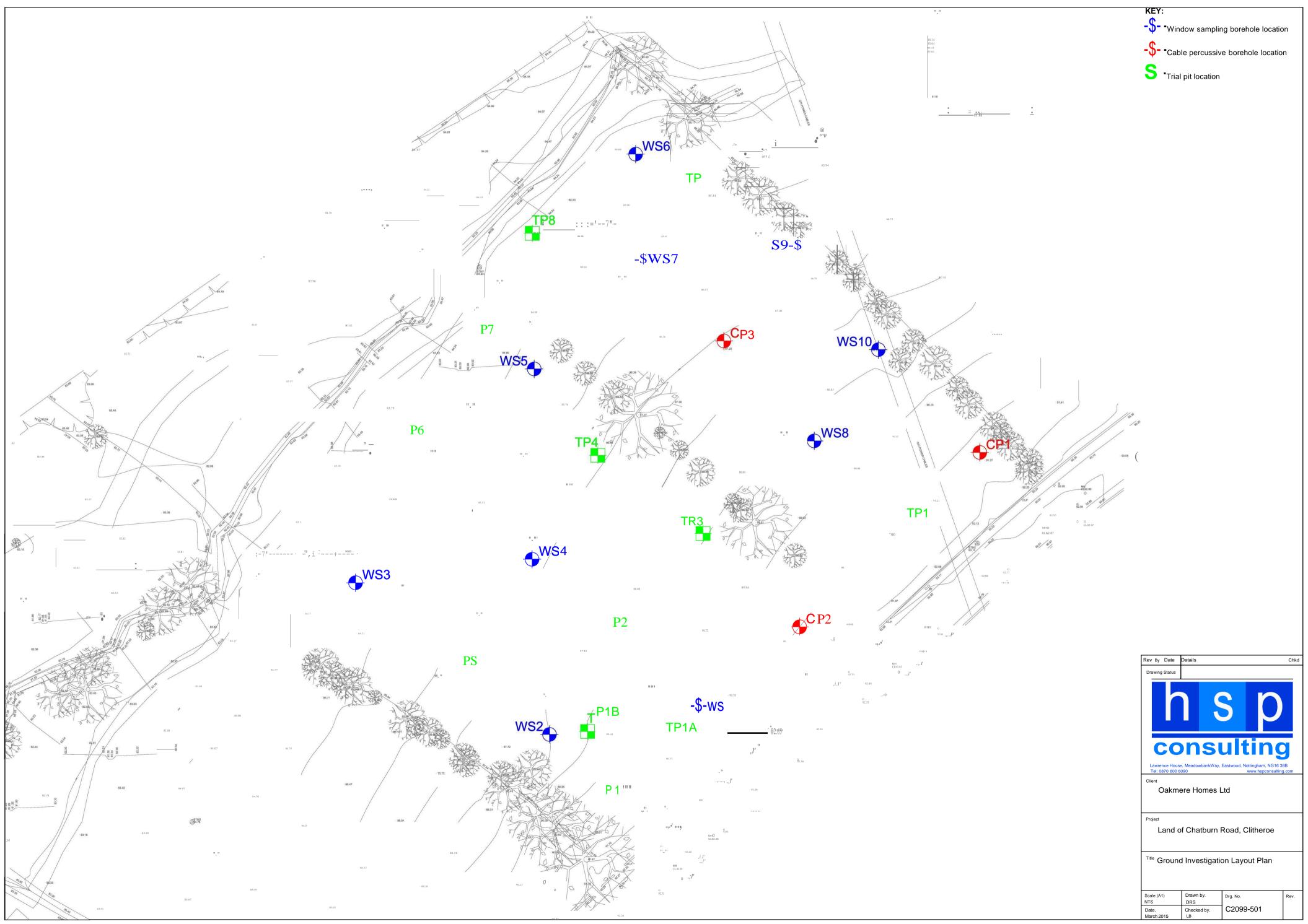
	C	n				_			Borehole N	lo.
	2	P				Boı	eho	ole Log	WS10	)
con	sult	ing			<b>.</b>				Sheet 1 of	
Projec	t Name	: Chatburn I	Road,		Project No. C2099		Co-ords	: -	Hole Type WS	е
Locati	ion:	Clitheroe					Level:		Scale	
							20701.		1:50	.,
Client:		Oakmere I					Dates:	-	Logged B	у
Well	Water Strikes			In Situ Testing	Depth (m)	Level (m)	Legend	StratumDescription		
Well	Water Strikes	Samples  Depth (m)  0.50  1.0	Type  D		(m) 0.15	Level (m)	Legend	Turf over dark brown sandy gravelly plasticity (field description). Gravel angular to angular fine and medium and mudstone.  Firm orangish brown slightly gravelly CLAY. High plasticity (field description is sub-rounded to angular fine to cosandstone and mudstone.	/ CLAY. High is sub- of sandstone / very sandy on). Gravel	1
										9
										10

- 1. No groundwater was encountered during the drilling process.
- 2. Borehole was terminated at 1.00m depth due to refusal and backfilled with arisings.





## **Appendix III**





# **Appendix IV**

#### **SUMMARY OF GEOTECHNICAL TESTING**

		Sample of	letails		Clas	sificatio	n Te	sts		Densit	y Tests	Undrained	d Triaxial Co	mpression	C	hemical Te	ests	
Borehole / Sample Ref	Depth (m)	Туре	Description	МС	LL	PL	P		425 Jm	Bulk	Dry	Cell Pressure	Deviator Stress	Shear Stress	рН	2:1 W/S SO4	W/S Mg	Other tests and comments
	,			(%)	(%)	(%)	(%	%) (	(%)	Mg/m³	Mg/m³	kPa	kPa	kPa		(g/L)	(mg/L)	
BH1	2.80-3.30	В	Grey sandy silty CLAY with abundant gravel	8.4	27	14	1	3 6	62									2.5kg Compaction
BH2	1.00-1.50	В	Grey brown gravelly sandy silty CLAY. Gravel is mudstone.	37	43	23	2	20 8	80									Particle Size Distribution 2.5kg Compaction
BH2	1.90-2.35	U	Soft to firm brown silty CLAY with abundant gravel and rootlets															Oedometer consolidation
BH2	2.70-3.20	В	Grey brown gravelly sandy silty CLAY															Particle Size Distribution
BH2	3.50-3.80	В	Grey brown sandy silty clayey GRAVEL. Gravel is fine to cobble sized limestone.	4.9	24	13	1	1 2	28									Particle Size Distribution Compaction cancelled - insufficient material
BH2	3.50	U	Firm to stiff grey sandy gravelly CLAY	9.9						2.29	2.08	35	196	98				
ВН3	1.10-1.60	В	Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized limestone.	14	28	16	1:	2 5	54									Particle Size Distribution Compaction cancelled - unsuitable material, too much coarse gravel present
ВН3	1.80-2.20	U	Firm grey silty CLAY with abundant gravel	9.5	24	15	9.	.0	60									Oedometer consolidation
ВН3	2.70-3.20	В	Dark grey-brown sandy very gravelly silty CLAY. Gravel includes cobble sized gravel.	10	27	14	1:	3 4	47									
ВН3	3.70-4.15	U	Soft to firm grey brown sandy gravelly silty CLAY. Gravel is fine to medium.	12						2.39	2.13	37 74	87 90	44				
TP10	0.60	В	Brown sandy silty CLAY with rare fine to medium gravel															Particle Size Distribution California Bearing Ratio
TP10	1.50	В	Brown sandy very gravelly silty CLAY. Gravel includes cobble sized gravel.	18	36	17	1:	9 !	56									

Sample type: B (Bulk disturb.) BLK (Block) C (Core) D (Disturbed) LB (Large Bulk dist.) U (Undisturbed)

Operations Manager 27/04/2015 Project Number:

Project Name:

GEO / 22476

**C2099 CLITHEROE** 

**GEOLABS** 

#### **SUMMARY OF GEOTECHNICAL TESTING**

Sample details						Classification Tests				Density Tests		Undrained Triaxial Compression			Chemical Tests					
Borehole / Trial Pit	ample Ref	Depth (m)	Туре	Description	МС	LL		PI	μι	125 m	Bulk	Dry	Cell Pressure	Deviator Stress	Shear Stress	рН	2:1 W/S SO4	W/S Mg	Other tests and comments	
					(%)	(%	5) (%)	(%)	) (%	%)	Mg/m³	Mg/m³	kPa	kPa	kPa		(g/L)	(mg/L)		
TP10		2.40	В	Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble sized.															Particle Size Distribution	
TP2		0.80	В	Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized															California Bearing Ratio	
TP3		0.50	В	Brown gravelly sandy silty CLAY. Gravel is fine to cobble sized limestone.															Particle Size Distribution California Bearing Ratio	
TP3		1.50	В	Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized limestone.	17	37	18	19	5	56									Particle Size Distribution Compaction cancelled - unsuitable material, too much coarse gravel present	
TP4		0.60	В	Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble sized sandstone.															California Bearing Ratio	
TP5		0.50	В	Brown sandy silty CLAY with rare fine to medium gravel															Particle Size Distribution	
TP5		1.60	В	Brown mottled orange sandy gravelly silty CLAY. Gravel includes cobble sized gravel with rare rootlets.	17	36	20	16	6	i7										
TP5		1.90	D	Brown sandy gravelly silty CLAY.	8.4	27	13	14	4	4										
TP6		0.50	В	Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized.	28	41	19	22	2 8	36									California Bearing Ratio	
TP6		1.50	В	Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble sized.															Particle Size Distribution	
TP7		0.50	D	Mottled brown grey and dark orange silty CLAY with rare rootlets.	28	51	27	24	1 10	00										
TP8		0.50	В	Brown sandy gravelly silty CLAY. Gravel is sandstone.															California Bearing Ratio	

Sample type: B (Bulk disturb.) BLK (Block) C (Core) D (Disturbed) LB (Large Bulk dist.) U (Undisturbed)

Checked and Approved by
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15/
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27/04/2015

Project Number:

Project Name:

GEO / 22476

**C2099 CLITHEROE** 

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#### **SUMMARY OF GEOTECHNICAL TESTING**

Sample details					Classification Tests				Density Tests		Undrained Triaxial Compression			Chemical Tests				
Borehole / Trial Pit	Sample Ref	Depth (m)	Туре	Description	MC (%)	LL (%)	PL (%)		μm	Bulk Mg/m³	Dry Mg/m³	Cell Pressure kPa	Deviator Stress kPa	Shear Stress kPa	рН	2:1 W/S SO4 (g/L)	W/S Mg (mg/L)	Other tests and comments
TP9		0.60	В	Brown gravelly sandy silty CLAY. Gravel is sandstone.	30	39	28	11	1 73									Particle Size Distribution 2.5kg Compaction
TP9		1.50	В	Grey brown sandy gravelly silty CLAY.														Particle Size Distribution

Sample type: B (Bulk disturb.) BLK (Block) C (Core) D (Disturbed) LB (Large Bulk dist.) U (Undisturbed)

101	Project Number:  GEO / 22476  Project Name:	[GEOLABS]®
Operations Manager 27/04/2015	C2099 CLITHEROE	

# B - 22476-113809.xls BH1 02.80

1220 - LLPL

## BS1377: Part 2: 1990 Clauses 4.3 & 5

#### **Liquid & Plastic Limits**

Hole ID Depth (m) 2.80-3.30 Sample Type

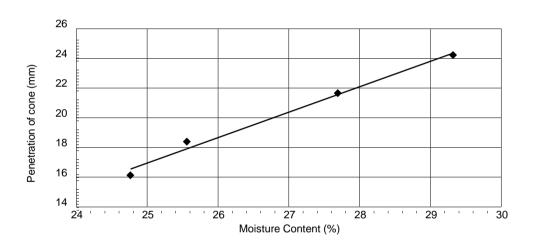
Description:

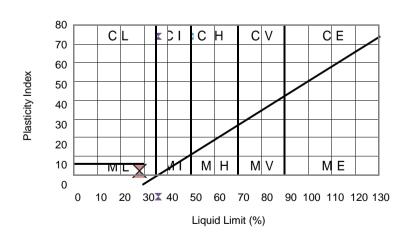
Grey sandy silty CLAY with abundant gravel

Preparation: Sample washed and air dried

Moisture content : (BS1377:Part 2:Clause 3:1990)	8.4	%
Percentage passing 425µm sieve :	62	%
Liquid Limit :	27	%
Plastic Limit :	14	%
Plasticity Index :	13	

Equivalent moisture content of material passing 425µm sieve : 14 % Liquidity Index: -0.03





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BH2 01.00

1220 - LLPL

## BS13

BS1377: Part 2: 1990 Clauses 4.3 & 5

#### **Liquid & Plastic Limits**

Hole ID Depth (m) Sample Type

BH2 1.00-1.50 Description:

Grey brown gravelly sandy silty CLAY. Gravel is mudstone.

Preparation: Sample washed and air dried

Moisture content : (BS1377:Part 2:Clause 3:1990)

Percentage passing 425μm sieve :

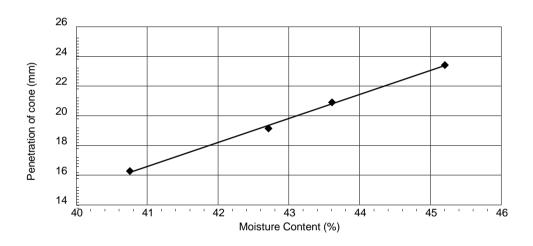
Liquid Limit :

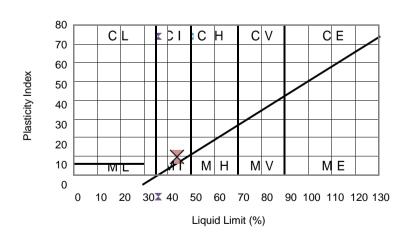
Plastic Limit :

Plasticity Index :

20

Equivalent moisture content of material passing 425µm sieve : 46 % Liquidity Index : 1.17





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GEO / 22476

Project Name:

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GL:Version 1.54 - 23/04/2015

BH2 03.50

1220 - LLPL

#### BS1377: Part 2: 1990 Clauses 4.3 & 5

#### **Liquid & Plastic Limits**

Hole ID Depth (m) Sample Type

BH2 3.50-3.80 Description:

Grey brown sandy silty clayey GRAVEL. Gravel is fine to cobble sized limestone.

Preparation: Sample washed and air dried

Moisture content : (BS1377:Part 2:Clause 3:1990)

Percentage passing 425μm sieve :

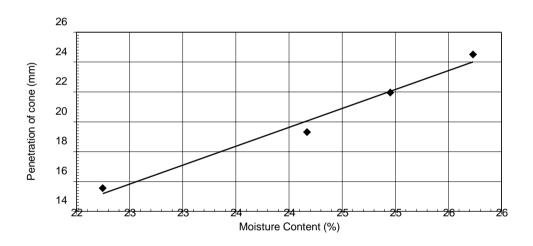
Liquid Limit :

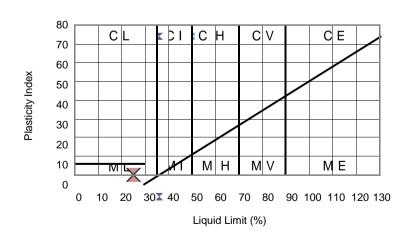
Plastic Limit :

Plasticity Index :

11

Equivalent moisture content of material passing 425µm sieve : 18 % Liquidity Index : 0.42





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Project Name:



BH3 01.10

1220 - LLPL

#### BS1377: Part 2: 1990 Clauses 4.3 & 5

#### **Liquid & Plastic Limits**

Hole ID Depth (m) Sample Type

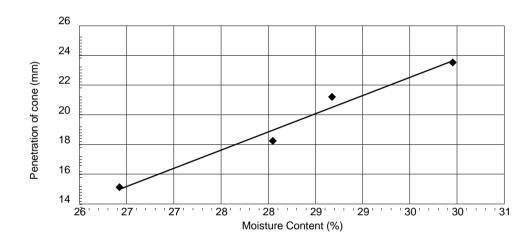
BH3 1.10-1.60 Description:

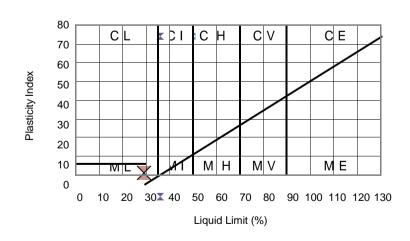
Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized limestone.

Sample washed and air dried Preparation:

Moisture content: (BS1377:Part 2:Clause 3:1990) 14 % Percentage passing 425µm sieve : 54 % Liquid Limit: 28 % Plastic Limit: 16 % Plasticity Index: 12

Equivalent moisture content of material passing 425µm sieve : 26 % Liquidity Index: 0.84





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GL:Version 1.54 - 23/04/2015

### BS1377: Part 2: 1990 Clauses 4.3 & 5

#### **Liquid & Plastic Limits**

Hole ID BH3 Depth (m) 1.80-2.20 Sample Type

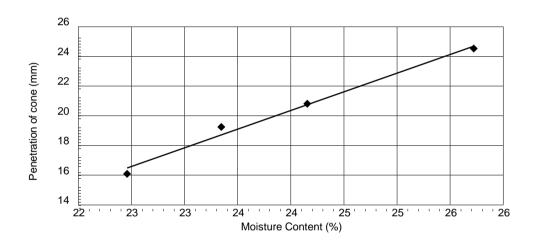
Description:

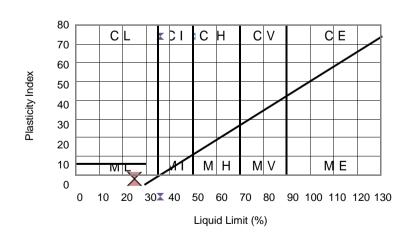
Firm grey silty CLAY with abundant gravel

Sample washed and air dried Preparation:

Moisture content: (BS1377:Part 2:Clause 3:1990) 9.5 % Percentage passing 425µm sieve : 60 % Liquid Limit: 24 % Plastic Limit: 15 % Plasticity Index: 9.0

Equivalent moisture content of material passing 425µm sieve : 16 % Liquidity Index: 0.08





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Project Name:



BH3 02.70

1220 - LLPL

BS1377: Part 2: 1990 Clauses 4.3 & 5

#### **Liquid & Plastic Limits**

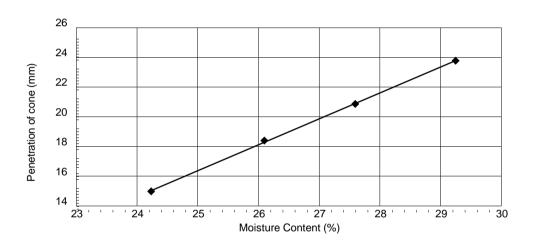
Hole ID Depth (m) Sample Type BH3 2.70-3.20 Description:

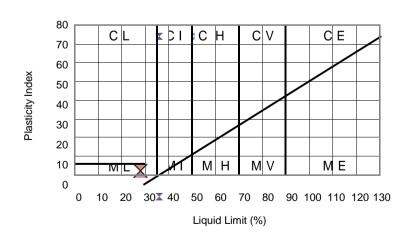
Dark grey-brown sandy very gravelly silty CLAY. Gravel includes cobble sized gravel.

Preparation: Sample washed and air dried

Moisture content : (BS1377:Part 2:Clause 3:1990)	10	%
Percentage passing 425µm sieve :	47	%
Liquid Limit:	27	%
Plastic Limit :	14	%
Plasticity Index :	13	

Equivalent moisture content of material passing 425µm sieve : 21 % Liquidity Index: 0.57





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#### BS1377: Part 2: 1990 Clauses 4.3 & 5

#### **Liquid & Plastic Limits**

TP10 Hole ID 1.50 Depth (m) Sample Type В

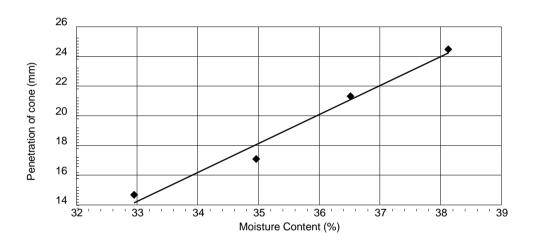
Description:

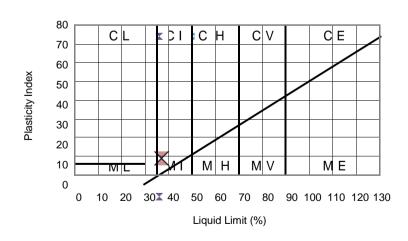
Brown sandy very gravelly silty CLAY. Gravel includes cobble sized gravel.

Sample washed and air dried Preparation:

Moisture content: (BS1377:Part 2:Clause 3:1990) 18 % Percentage passing 425µm sieve : 56 % Liquid Limit: 36 % Plastic Limit: 17 % Plasticity Index: 19

Equivalent moisture content of material passing 425µm sieve : 32 % Liquidity Index: 0.79





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TP3 01.50

1220 - LLPL

#### BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

TP3 Hole ID 1.50 Depth (m) Sample Type В

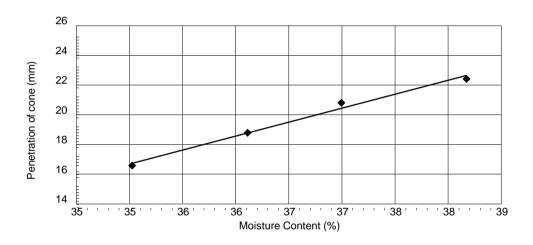
Description:

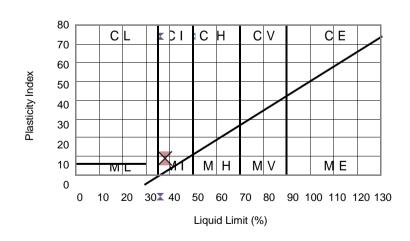
Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized limestone.

Sample washed and air dried Preparation:

Moisture content: (BS1377:Part 2:Clause 3:1990) 17 % Percentage passing 425µm sieve : 56 % Liquid Limit: 37 % Plastic Limit: 18 % Plasticity Index: 19

Equivalent moisture content of material passing 425µm sieve : 31 % Liquidity Index: 0.66





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1220 - LLPL

#### BS1377: Part 2: 1990 Clauses 4.3 & 5

# **Liquid & Plastic Limits**

TP5 Hole ID Depth (m) 1.60 Sample Type В

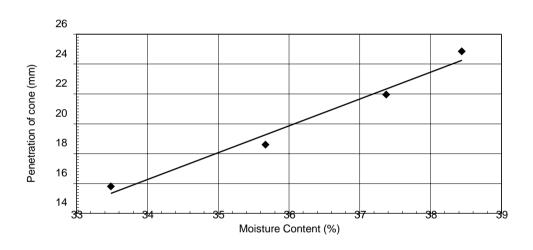
Description:

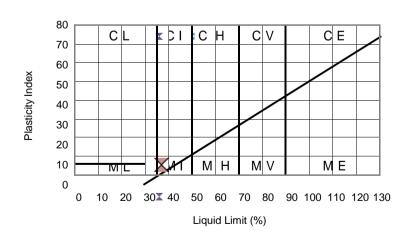
Brown mottled orange sandy gravelly silty CLAY. Gravel includes cobble sized gravel with rare rootlets.

Preparation: Sample washed and air dried

Moisture content : (BS1377:Part 2:Clause 3:1990)	17	%
Percentage passing 425µm sieve :	67	%
Liquid Limit :	36	%
Plastic Limit :	20	%
Plasticity Index :	16	

Equivalent moisture content of material passing 425µm sieve : 25 % Liquidity Index: 0.33





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Project Name:



# D - 22476-113817.xls TP5 01.90

220 - LLPL

## BS1377: Part 2: 1990 Clauses 4.3 & 5

# **Liquid & Plastic Limits**

TP5 Hole ID 1.90 Depth (m) Sample Type D

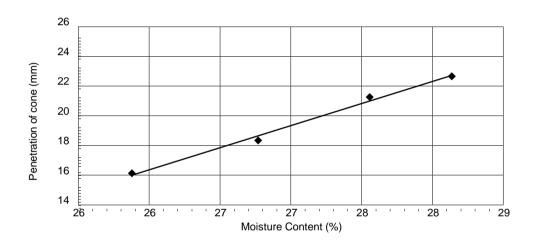
Description:

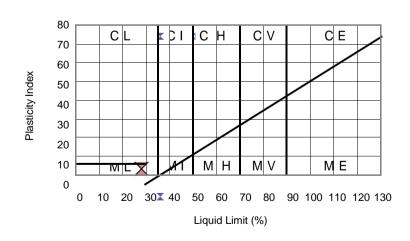
Brown sandy gravelly silty CLAY.

Preparation: Sample washed and air dried

Moisture content: (BS1377:Part 2:Clause 3:1990) 8.4 % Percentage passing 425µm sieve : 44 % Liquid Limit: 27 % Plastic Limit: 13 % Plasticity Index: 14

Equivalent moisture content of material passing 425µm sieve : 19 % Liquidity Index: 0.42





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Project Number:

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Project Name:



TP7 00.50

220 - LLPL

#### BS1377: Part 2: 1990 Clauses 4.3 & 5

### **Liquid & Plastic Limits**

TP7 Hole ID 0.50 Depth (m) Sample Type D

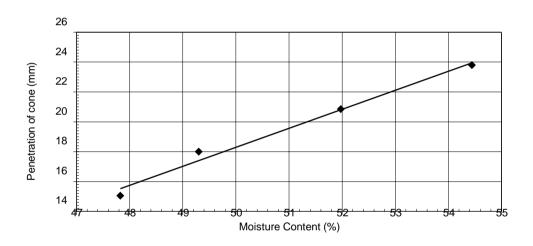
Description:

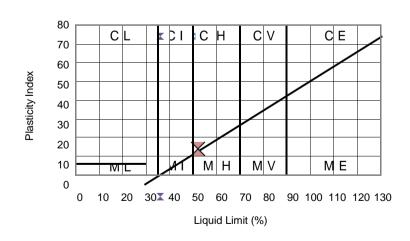
Mottled brown grey and dark orange silty CLAY with rare rootlets.

Preparation: Sample as received

Moisture content: (BS1377:Part 2:Clause 3:1990) 28 % Percentage passing 425µm sieve : 100 % Liquid Limit: 51 % Plastic Limit: 27 % Plasticity Index: 24

Equivalent moisture content of material passing 425µm sieve : 28 % Liquidity Index: 0.04





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Project Name:



TP9 00.60

1220 - LLPL

#### BS1377: Part 2: 1990 Clauses 4.3 & 5

## **Liquid & Plastic Limits**

TP9 Hole ID 0.60 Depth (m) Sample Type В

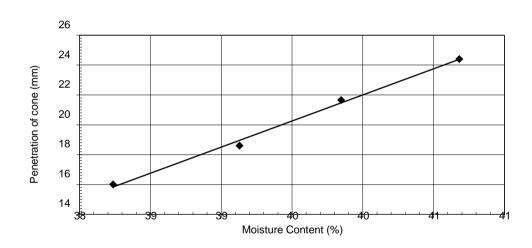
Description:

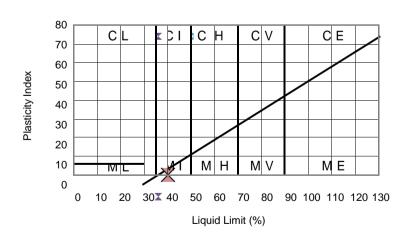
Brown gravelly sandy silty CLAY. Gravel is sandstone.

Preparation: Sample washed and air dried

Moisture content: (BS1377:Part 2:Clause 3:1990) 30 % Percentage passing 425µm sieve : 73 % Liquid Limit: 39 % Plastic Limit: 28 % Plasticity Index:

Equivalent moisture content of material passing 425µm sieve : 41 % Liquidity Index: 1.19





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#### PARTICLE SIZE DISTRIBUTION

BH/TP No: BH2 Depth (m): 1.00-1.50 Sample Type

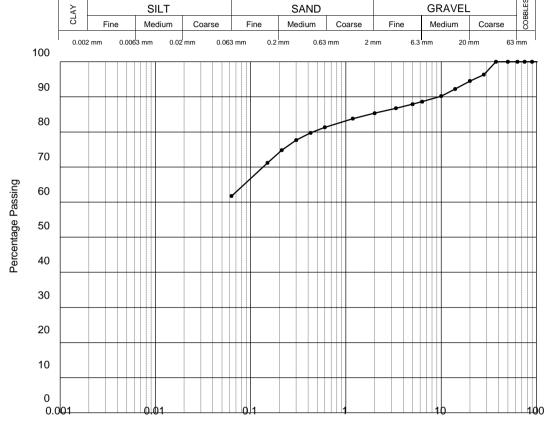
В

Description:

Grey brown gravelly sandy silty CLAY. Gravel is mudstone.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Sieve	
Sieve (mm)	% pass
200	100
125	100
90	100
75	100
63	100
50	100
37.5	100
28	96
20	94
14	92
10	90
6.3	89
5	88
3.35	87
2	85
1.18	84
0.6	81
0.425	80
0.3	78
0.212	75
0.15	71
0.063	62



Particle Size (mm)

Particle Proportions	
Cobbles	0.0 %
Gravel	14.7 %
Sand	23.6 %
Silt & Clay	61.8 %

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Project Name:

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#### PARTICLE SIZE DISTRIBUTION

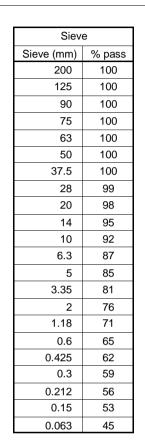
BH/TP No: BH2 Depth (m): 2.70-3.20

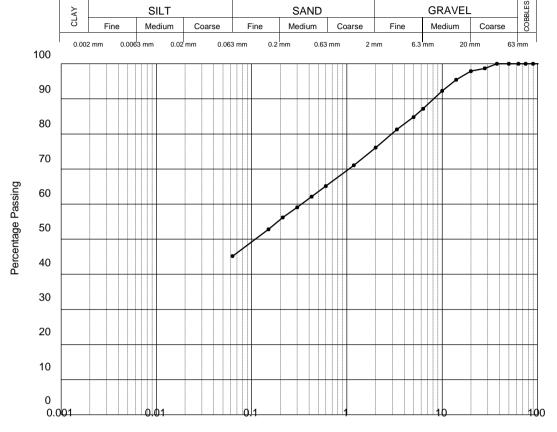
Sample Type B

Description:

Grey brown gravelly sandy silty CLAY

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions	
Cobbles	0.0 %
Gravel	23.9 %
Sand	30.9 %
Silt & Clay	45.2 %

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Project Name:

GEO / 22476



### **PARTICLE SIZE DISTRIBUTION**

BH/TP No: BH2
Depth (m): 3.50-3.80
Sample Type B

Description:

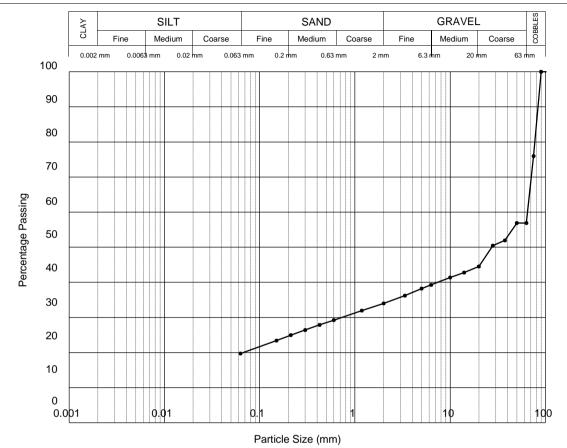
Grey brown sandy silty clayey GRAVEL. Gravel is fine to cobble

sized limestone.

Insufficient material supplied to be representative in accordance with BS1377 requirements.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Sieve	
Sieve (mm)	% pass
200	100
125	100
90	100
75	76
63	57
50	57
37.5	52
28	50
20	45
14	43
10	41
6.3	39
5	38
3.35	36
2	34
1.18	32
0.6	29
0.425	28
0.3	26
0.212	25
0.15	23
0.063	20



Particle Proportions	
Cobbles	43.1 %
Gravel	22.9 %
Sand	14.3 %
Silt & Clay	19.7 %

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Project Name:

GEO / 22476



### **PARTICLE SIZE DISTRIBUTION**

BH/TP No: BH3 Depth (m): 1.10-1.60 Sample Type В

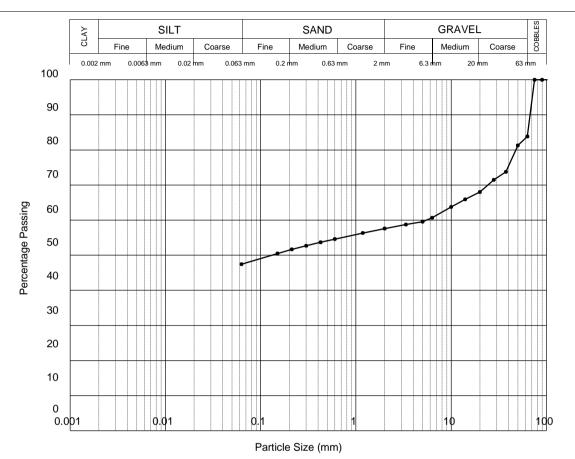
Description:

Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized limestone.

Insufficient material supplied to be representative in accordance with BS1377 requirements.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Siev	е
Sieve (mm)	% pass
200	100
125	100
90	100
75	100
63	84
50	81
37.5	74
28	71
20	68
14	66
10	64
6.3	61
5	60
3.35	59
2	58
1.18	56
0.6	55
0.425	54
0.3	53
0.212	52
0.15	50
0.063	47



Particle Proportions	
Cobbles	16.2 %
Gravel	26.2 %
Sand	10.1 %
Silt & Clay	47.5 %

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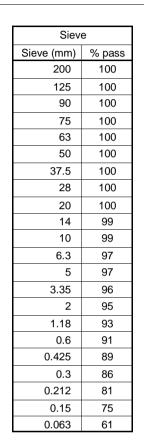
#### PARTICLE SIZE DISTRIBUTION

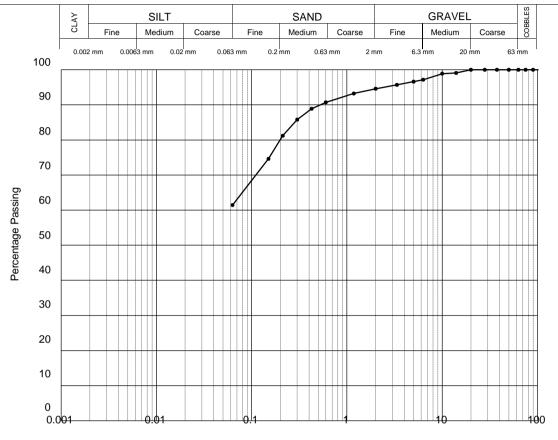
BH/TP No: TP10
Depth (m): 0.60
Sample Type B

Description:

Brown sandy silty CLAY with rare fine to medium gravel

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions		
Cobbles	0.0 %	
Gravel	5.4 %	
Sand	33.1 %	
Silt & Clay	61.5 %	

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Operations Manager

Project Name:



### **PARTICLE SIZE DISTRIBUTION**

BH/TP No: TP10
Depth (m): 2.40
Sample Type B

Description:

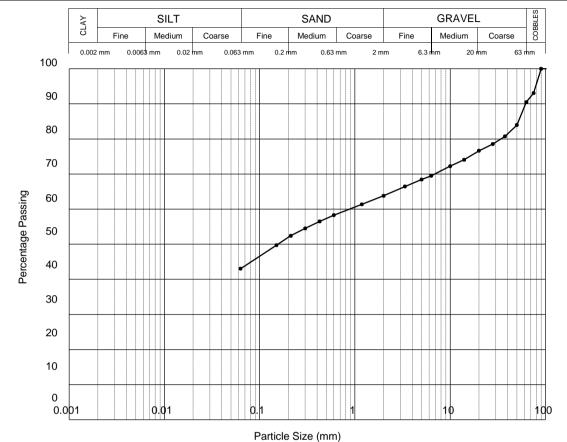
Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble

sized.

Insufficient material supplied to be representative in accordance with BS1377 requirements.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Siev	Sieve	
Sieve (mm)	% pass	
200	100	
125	100	
90	100	
75	93	
63	91	
50	84	
37.5	81	
28	79	
20	77	
14	74	
10	72	
6.3	70	
5	68	
3.35	66	
2	64	
1.18	61	
0.6	58	
0.425	56	
0.3	55	
0.212	52	
0.15	50	
0.063	43	



Particle Proportions	
Cobbles	9.5 %
Gravel	26.7 %
Sand	20.8 %
Silt & Clay	43.0 %

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Operations Manager 27/04/2015 Project Number:

Project Name:

GEO / 22476



#### PARTICLE SIZE DISTRIBUTION

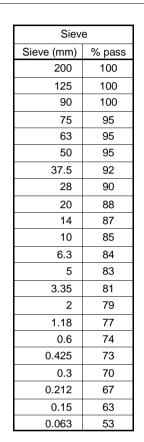
BH/TP No: TP3
Depth (m): 0.50
Sample Type B

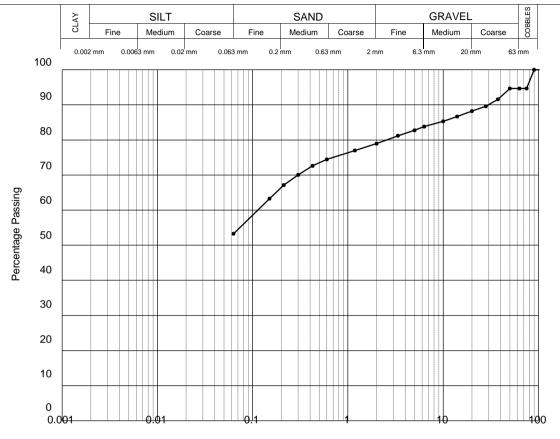
Description:

Brown gravelly sandy silty CLAY. Gravel is fine to cobble sized

limestone.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions	
Cobbles	5.3 %
Gravel	15.7 %
Sand	25.7 %
Silt & Clay	53.3 %

Checked and Approved by

Project Number:

GEO / 22476

151

Project Name:



### **PARTICLE SIZE DISTRIBUTION**

BH/TP No: TP3
Depth (m): 1.50
Sample Type B

Description:

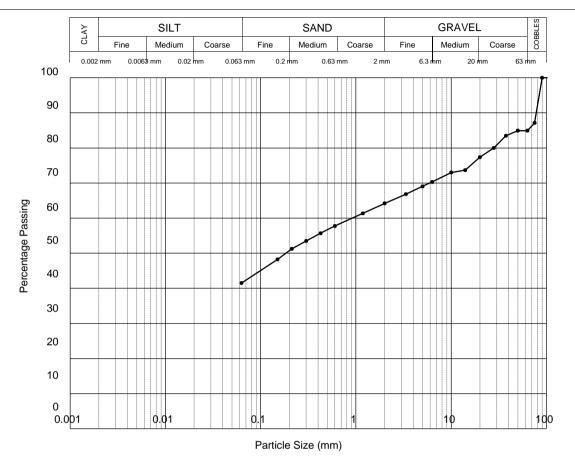
Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized

limestone.

Insufficient material supplied to be representative in accordance with BS1377 requirements.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Sieve				
Sieve (mm) % pass				
200	100			
125	100			
90	100			
75	87			
63	85			
50	85			
37.5	83			
28	80			
20	77			
14	74			
10	73			
6.3	70			
5	69			
3.35	67			
2	64			
1.18	61			
0.6	58			
0.425	56			
0.3	53			
0.212	51			
0.15	48			
0.063	42			



Particle Proportions		
Cobbles 15.1 %		
Gravel	20.7 %	
Sand	22.7 %	
Silt & Clay	41.5 %	

Checked and Approved by

Operations Manager 27/04/2015 Project Number:

Project Name:

GEO / 22476



#### PARTICLE SIZE DISTRIBUTION

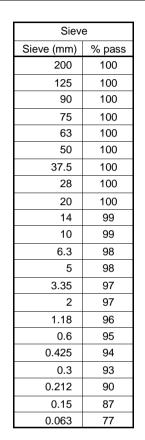
BH/TP No: TP5
Depth (m): 0.50

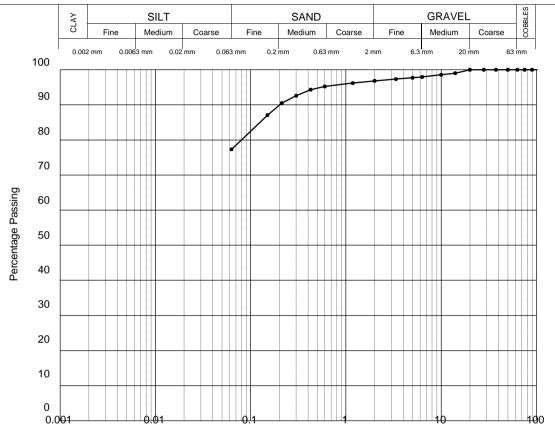
Description:

Brown sandy silty CLAY with rare fine to medium gravel

Sample Type B

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions		
Cobbles 0.0 %		
Gravel	3.2 %	
Sand	19.5 %	
Silt & Clay	77.3 %	

Checked and Approved by

Project Number:

GEO / 22476

Operation

Project Name:



### **PARTICLE SIZE DISTRIBUTION**

BH/TP No: TP6
Depth (m): 1.50
Sample Type B

Description:

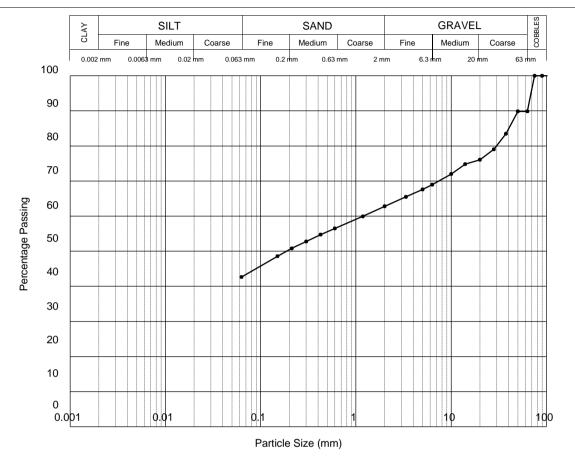
Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble

sized.

Insufficient material supplied to be representative in accordance with BS1377 requirements.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method

Sieve				
Sieve (mm) % pas				
200	100			
125	100			
90	100			
75	100			
63	90			
50	90			
37.5	83			
28	79			
20	76			
14	75			
10	72			
6.3	69			
5	68			
3.35	66			
2	63			
1.18	60			
0.6	57			
0.425	55			
0.3	53			
0.212	51			
0.15	49			
0.063	43			



Particle Proportions		
Cobbles 10.2 %		
Gravel	27.0 %	
Sand	20.2 %	
Silt & Clay 42.6 %		

Checked and Approved by

Operations Manager 27/04/2015 Project Number:

Project Name:

GEO / 22476



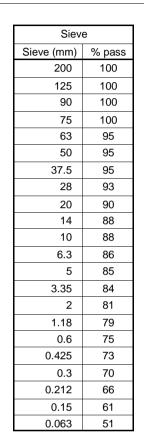
#### PARTICLE SIZE DISTRIBUTION

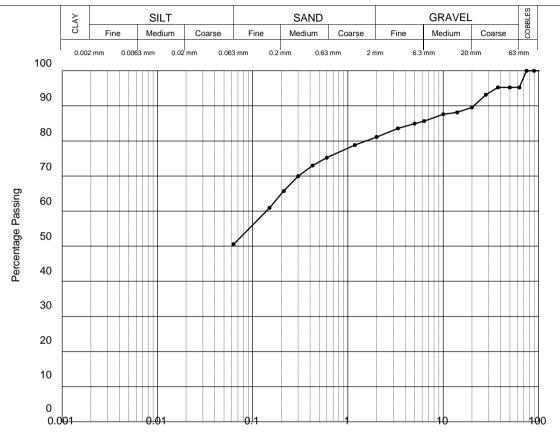
BH/TP No: TP9
Depth (m): 0.60
Sample Type B

Description:

Brown gravelly sandy silty CLAY. Gravel is fine to cobble sized.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions		
Cobbles 4.7 %		
Gravel	14.1 %	
Sand	30.6 %	
Silt & Clay	50.6 %	

Checked and Approved by

Operations Manager 27/04/2015

Project Number:

Project Name:

GEO / 22476

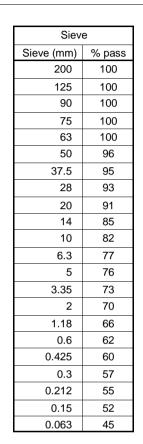


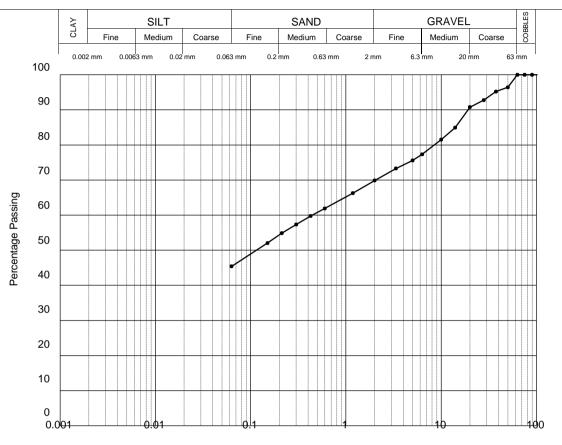
#### PARTICLE SIZE DISTRIBUTION

BH/TP No: TP9
Depth (m): 1.50
Sample Type B

Description:
Grey brown sandy gravelly silty CLAY.

BS1377: Part 2: Clause 9.2: 1990 Wet Sieving Method





Particle Size (mm)

Particle Proportions			
Cobbles 0.0 %			
Gravel	30.1 %		
Sand	24.5 %		
Silt & Clay 45.4 %			

Checked and Approved by

Operations Manager 27/04/2015

Project Number:

Project Name:

GEO / 22476



#### BS1377:Part 4:1990 Clause 3.4

# **Moisture Content / Dry Density Relationship**

BH/TP BH1 Depth (m) 2.80-3.30 Sample Type В

B - 22476-113809.xls

1410 - Comp BH1 02.80

Description:

Grey sandy silty CLAY with abundant gravel

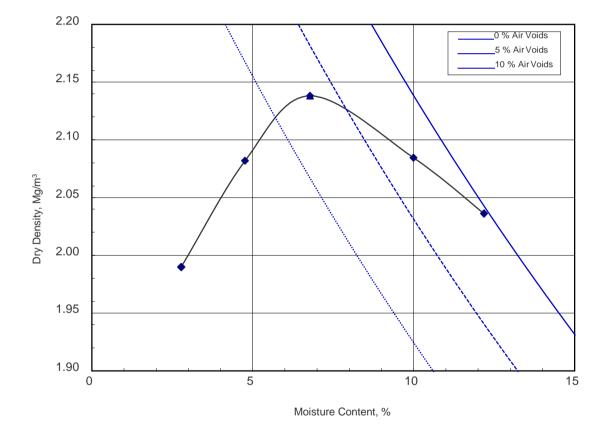
Preparation Air dried / Oven dried

2.5kg Rammer for soils with some coarse Test Method gravel-size particles

Single / Multiple Samples Used Mass Retained on 37.5 mm Sieve % 9 Mass Retained on 20.0 mm Sieve % 15

Particle Density - Assumed Mg/m³ 2.72

Maximum Dry Density Mg/m³ 2.14 **Optimum Moisture Content** % 6.8



Determination	1	2	3	4	5
Moisture Content	6 2.8	4.8	6.8	10.0	12
Dry Density Mg/m	³ 1.99	2.08	2.14	2.08	2.04

Checked and Approved by:

Operations Manager 27/04/2015

GL:Version 1.14 - 10/12/2014

Project Number:

**GEO / 22476** 

Project Name:



#### BS1377:Part 4:1990 Clause 3.4

# **Moisture Content / Dry Density Relationship**

BH/TP BH2 Depth (m) 1.00-1.50 Sample Type В

B - 22476-114255.xls

1410 - Comp BH2 01.00

Description:

Brown sandy silty CLAY with rare gravel

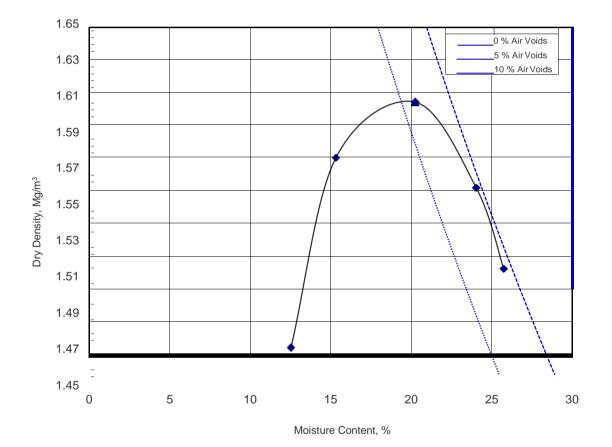
Preparation Air dried / Oven dried

2.5kg Rammer for soils with some coarse Test Method gravel-size particles

Single / Multiple Samples Used

Mass Retained on 37.5 mm Sieve % 9 Mass Retained on 20.0 mm Sieve % 15 Particle Density - Assumed Mg/m³ 2.72

Maximum Dry Density Mg/m³ 1.61 **Optimum Moisture Content** % 20.1



Determination	1	2	3	4	5
Moisture Content %	12	15	20	24	26
Dry Density Mg/m	1.47	1.57	1.61	1.56	1.51

Checked and Approved by:

Operations Manager 27/04/2015

GL:Version 1.14 - 10/12/2014

Project Number:

**GEO / 22476** 

Project Name:



# BS1377:Part 4:1990 Clause 3.4 **Moisture Content / Dry Density Relationship**

TP9 BH/TP Depth (m) 0.60 Sample Type В

B - 22476-113797.xls

1410 - Comp TP9 00.60

Description:

Brown sandy silty CLAY with occasional gravel sized sandstone.

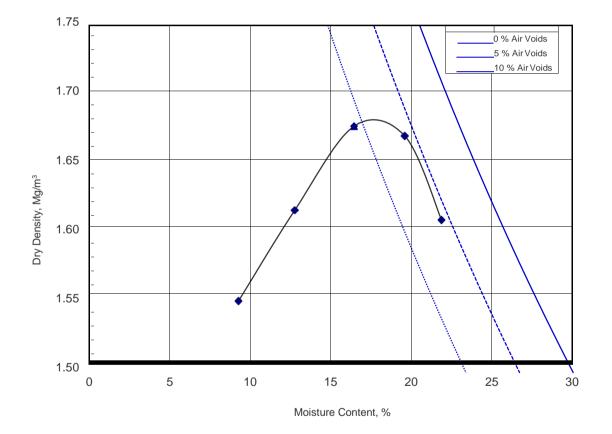
Preparation Air dried / Oven dried

2.5kg Rammer for soils with some coarse Test Method gravel-size particles

Single / Multiple Samples Used Mass Retained on 37.5 mm Sieve % 5 6 Mass Retained on 20.0 mm Sieve %

Particle Density - Assumed Mg/m³ 2.72

Maximum Dry Density Mg/m³ 1.68 **Optimum Moisture Content** % 16.3



Determination	1	2	3	4	5
Moisture Content %	9.1	13	16	19	22
Dry Density Mg/m	³ 1.55	1.62	1.68	1.67	1.61

Checked and Approved by:

Operations Manager 27/04/2015

GL:Version 1.14 - 10/12/2014

Project Number:

**GEO / 22476** 

Project Name:



# **Determination of California Bearing Ratio**

BH/TP No.: TP10 Depth (m): 0.6 Sample Type:

B - 22476-113784.xls

Description:

Brown very sandy silty CLAY

#### **PREPARATION DETAILS**

The specimen was tested in an unsoaked condition.

The specimen was initially air dried

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.87 Mg/m³

Prepared dry density 1.45 Mg/m³

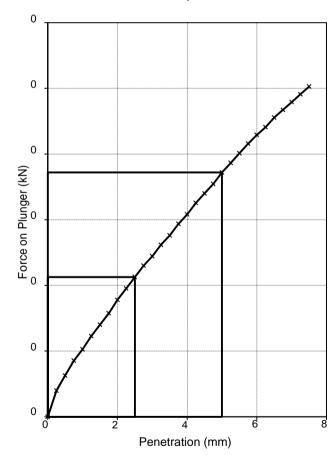
0.0 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base
Surcharge	8.0 kg	8.0 kg
Seating load	10 N	10 N
Moisture content	29 %	29 %
CBR Value	0.8 %	0.9 %

#### Top of Specimen

# 0 0 Force on Plunger (kN) 0 2 Penetration (mm)

#### Base of Specimen



Checked and Approved by:

Operations Manager

Project Number:

GEO / 22476

Project Name:



# **Determination of California Bearing Ratio**

BH/TP No.: TP2 Depth (m): 0.8

Sample Type:

B - 22476-113801.xls

Description:

Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized

#### **PREPARATION DETAILS**

В

The specimen was tested in an unsoaked condition.

The specimen was tested at its existing moisture content

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.98 Mg/m³

Prepared dry density 1.61 Mg/m³

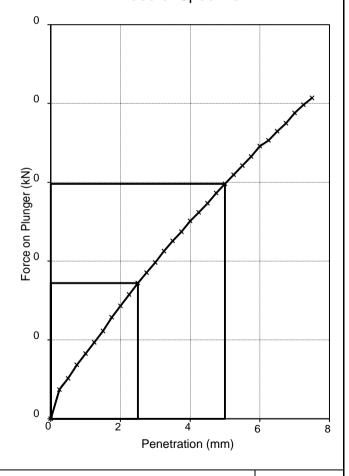
10.8 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base
Surcharge	8.0 kg	8.0 kg
Seating load	10 N	10 N
Moisture content	23 %	22 %
CBR Value	0.9 %	0.7%

#### Top of Specimen

# Penetration (mm)

#### Base of Specimen



Checked and Approved by:

Operations Manager

r: Project Number:

GEO / 22476

C2099 CLITHEROE

Project Name:



GL:Version 1.12 - 26/02/2015

# **Determination of California Bearing Ratio**

BH/TP No.: TP3
Depth (m): 0.5
Sample Type: B

B - 22476-113799.xls

Description:

Brown gravelly sandy silty CLAY. Gravel is fine to cobble sized

limestone.

#### **PREPARATION DETAILS**

The specimen was tested in an unsoaked condition.

The specimen was tested at its existing moisture content

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.88 Mg/m³

Prepared dry density 1.46 Mg/m³

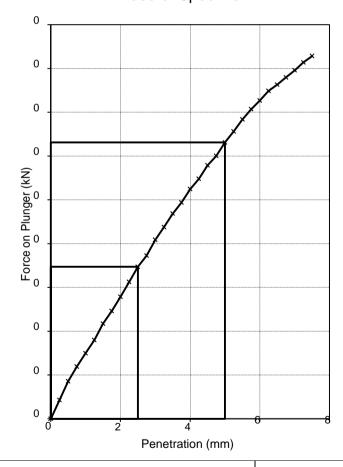
10.3 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base		
Surcharge	8.0 kg	8.0 kg		
Seating load	10 N	10 N		
Moisture content	30 %	29 %		
CBR Value	0.7%	0.6%		

#### Top of Specimen

# 

#### Base of Specimen



Checked and Approved by:

Operations Manager

Project Number:

Project Name:

GEO / 22476



# **Determination of California Bearing Ratio**

BH/TP No.: TP4
Depth (m): 0.6
Sample Type: B

B - 22476-113814.xls

Description:

Grey brown sandy gravelly silty CLAY. Gravel is fine to cobble sized sandstone.

#### **PREPARATION DETAILS**

The specimen was tested in an unsoaked condition.

The specimen was tested at its existing moisture content

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.91 Mg/m³

Prepared dry density 1.54 Mg/m³

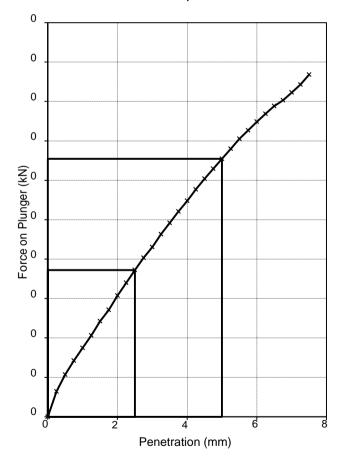
14.1 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base
Surcharge	8.0 kg	8.0 kg
Seating load	10 N	10 N
Moisture content	23 %	25 %
CBR Value	0.7 %	0.7%

#### Top of Specimen

# Penetration (mm)

#### Base of Specimen



Checked and Approved by:

Operations Manager

Project Number:

Project Name:

GEO / 22476



# **Determination of California Bearing Ratio**

BH/TP No.: TP6
Depth (m): 0.5
Sample Type: B

B - 22476-113805.xls

Description:

Brown sandy gravelly silty CLAY. Gravel is fine to cobble sized.

#### **PREPARATION DETAILS**

The specimen was tested in an unsoaked condition.

The specimen was tested at its existing moisture content

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.87 Mg/m³

Prepared dry density 1.46 Mg/m³

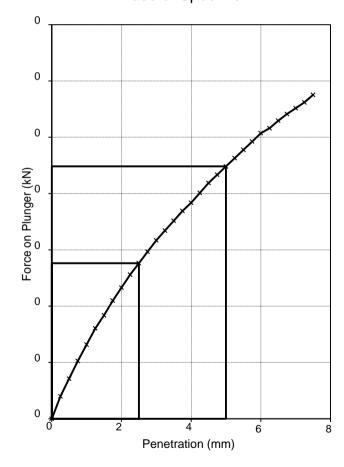
Prepared dry density 1.46 Mg/m³
3.8 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base
Surcharge	8.0 kg	8.0 kg
Seating load	10 N	10 N
Moisture content	28 %	28 %
CBR Value	1.2 %	1.1 %

#### Top of Specimen

# 

#### Base of Specimen



Checked and Approved by:

Operations Manager

JEN.

Project Number:

Project Name:

GEO / 22476

**C2099 CLITHEROE** 



GL:Version 1.12 - 26/02/2015

# **Determination of California Bearing Ratio**

BH/TP No.: TP8
Depth (m): 0.5
Sample Type: B

B - 22476-113825.xls

Description:

Brown sandy gravelly silty CLAY. Gravel is sandstone.

#### **PREPARATION DETAILS**

The specimen was tested in an unsoaked condition.

The specimen was tested at its existing moisture content

The specimen was prepared by dynamic compaction using a 2.5 kg rammer

Prepared bulk density 1.84 Mg/m³

Prepared dry density 1.41 Mg/m³

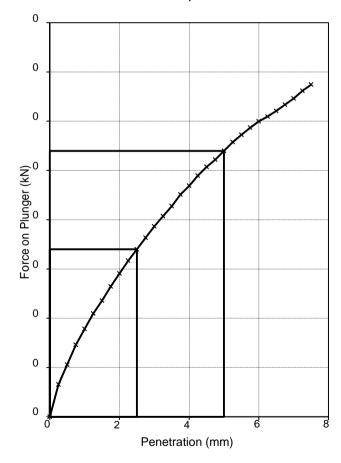
2.1 % of the sample was retained on a 20mm sieve

Test Details	Тор	Base		
Surcharge	8.0 kg	8.0 kg		
Seating load	10 N	10 N		
Moisture content	30 %	30 %		
CBR Value	13%	13%		

#### Top of Specimen

# 

#### Base of Specimen



Checked and Approved by:

Operations Manager

Project Number:

GEO / 22476

Project Name:



#### BS 1377: Part 7: 1990 Clause 8

# **Quick Undrained Triaxial Compression Test**

BH/TP No BH2 Depth (m) 3.50 Sample Type

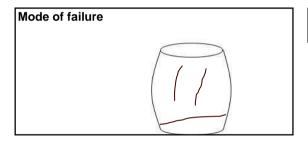
Description:

Firm to stiff grey sandy gravelly CLAY

Remarks: Sample reached 20% strain on first stage of multistage test

#### **Specimen Details**

Specimen conditions		Undisturbed
Length	(mm)	201.6
Diameter	(mm)	102.1
Moisture Content	(%)	9.9
Bulk Density	(Mg/m³)	2.29
Dry Density	(Mg/m³)	2.08
Test Details		
Latex membrane thickness	(mm)	0.3
Membrane correction	(kPa)	1.1
Axial displacement rate	(%/min)	2.0
Cell pressure	(kPa)	35
Strain at failure	(%)	20.8
Maximum Deviator Stress	(kPa)	196
Shear Stress Cu	(kPa)	98



Orientation of the sample	Vertical
Distance from top of tube mm	50



GEO / 22476

C2099 CLITHEROE



GL:Version 1.44 - 16/03/2015

Project Name:

# **Quick Undrained Triaxial Compression Test**

BH/TP No BH3
Depth (m) 3.70-4.15
Sample Type U

731 - UUTXL BH3 03.70 U - 22476-113830.xls

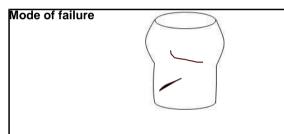
Description:

Soft to firm grey brown sandy gravelly silty CLAY. Gravel is fine to medium.

Remarks: Sample went to 20% on second stage of multistage test

#### **Specimen Details**

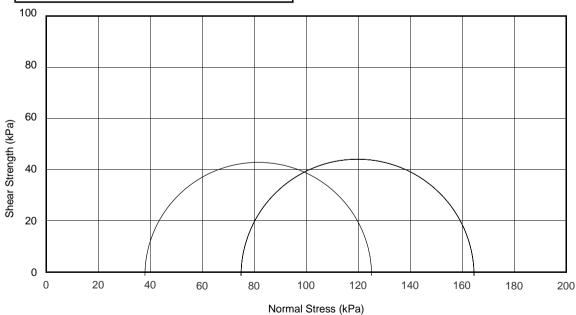
Specimen conditions		Undisturbed	
Length	(mm)	202.5	
Diameter	(mm)	101.7	
Moisture Content	(%)	12	
Bulk Density	(Mg/m³)	2.39	
Dry Density	(Mg/m³)	2.13	
Test Details		1	2
Latex membrane thickness	(mm)	0.3	0.3
Membrane correction	(kPa)	1.0	1.1
Axial displacement rate	(%/min)	1.0	1.0
Cell pressure	(kPa)	37	74
Strain at failure	(%)	17.3	20.7
Maximum Deviator Stress	(kPa)	87	90
Shear Stress Cu	(kPa)	44	45



Orientation of the sample	Vertical
Distance from top of tube mm	80

#### **Shear Strength Parameters**

Cohesion (kPa) 41
Angle of Shear Resistance (°) 2.0



Checked and Approved by:

Operations Manager 27/04/2015

Project Number:

GEO / 22476

Project Name:

C2099 CLITHEROE



Version 1.44 - 16/03

# **Determination of One Dimensional Consolidation Properties of Soil**

Description:

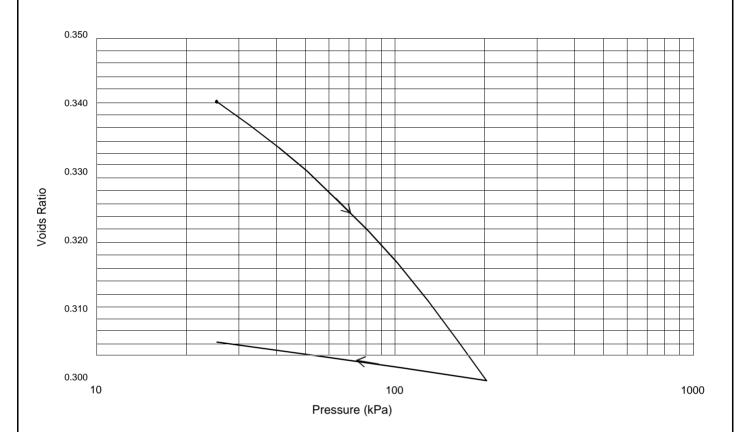
Borehole No.: BH2

Sample Ref.:

Depth (m): 1.90 Depth within original: 1.94 Orientation within original: Vertical Specimen preparation: Undisturbed

Soft to firm brown silty CLAY with abundant gravel and roots

Oedometer taken from base of sample due to amount of gravel present



#### **Initial Conditions:**

Initial

Height	(mm)	18.7	Moisture Content	(%)	14
Diameter	(mm)	76.1	Voids Ratio		0.364
Area	(mm2)	4550.8	Bulk Density	(Mg/m!)	2.26
Volume	(cm!)	85.1	Dry Density	(Mg/m!)	1.98

**Laboratory Temperature** ("C) 20 Particle density (Mg/m!) 2.7 (Assumed)

Pressure Range (kPa)	m _v (m2/MN)	c _v (m2/year)	Time Fitting Method	Void Ratio
0 - 25	0.65	2.4	t50	0.342
25 - 50	0.28	6.8	t50	0.333
50 - 100	0.18	5.6	t50	0.321
100 - 200	0.12	5.8	t50	0.305
200 - 25	0.010	7.0 (Sv)	t50	0.307

Checked and Approved by

Project Number:

J Sturges (Ops Mgr)

Date:

23/04/2015

Project Name:

C2099 Clitheroe



# **Determination of One Dimensional Consolidation Properties of Soil**

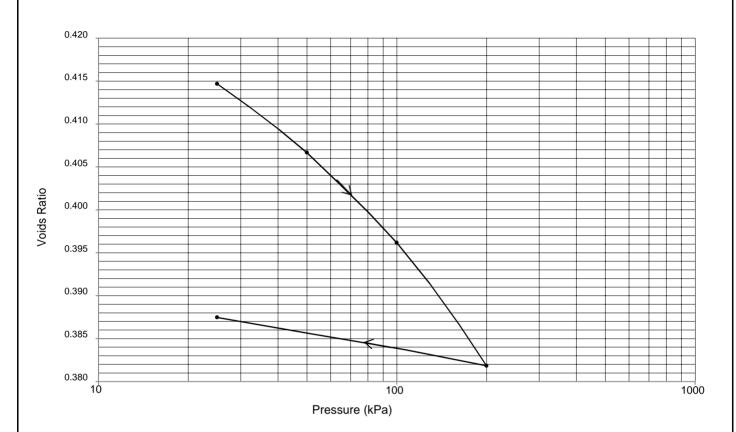
Borehole No.: BH3

Sample Ref.: -

Depth (m): 1.80
Depth within original: 1.84
Orientation within original: Vertical
Specimen preparation: Undisturbed

Description:

Firm grey silty CLAY with abundant gravel



#### **Initial Conditions:**

Initial Height (mm) 18.4 Moisture Content (%) 11 Diameter (mm) 76.2 Voids Ratio 0.418 Area (mm2) 4556.8 **Bulk Density** (Mg/m!) 2.11 Volume (cm!) 83.8 Dry Density (Mg/m!) 1.90

Laboratory Temperature ("C) 20 Particle density (Mg/m!) 2.7 (Assumed)

Pressure Range (kPa)	$ m m_{v}$ (m2/MN)	c _∨ (m2/year)	Time Fitting Method	Void Ratio
0 - 25	0.015	0.72	t90	0.417
25 - 50	0.16	9.8	t90	0.412
50 - 100	0.10	5.8	t90	0.404
100 - 200	0.086	7.2	t90	0.392
200 - 25	-0.0073	4.3 (Sv)	t90	0.391

Checked and Approved by

23/04/2015

Project Number:

J Sturges (Ops Mgr)

Date:

Project Name:

GEO / 22476

C2099 Clitheroe





# **Appendix V**





# Chemtest The right chemistry to deliver results

Chemtest Ltd.
Depot Road
Newmarket
CB8 0AL
Tel: 01638 606070

Email: info@chemtest.co.uk

# **Final Report**

Report Number: 15-06828 Issue-1

Initial Date of Issue: 31-Mar-2015

Client: HSP Consulting Engineers Limited

Lawrence House

Meadowbank Way

Client Address: Eastwood

Nottinghamshire

NG16 3SB

Contact(s): Luke Bradley

Project: C2099 - Clitheroe

Quotation No.: Date Received: 25-Mar-2015

Order No.: Date Instructed: 25-Mar-2015

No. of Samples: 15

Turnaround: (Wkdays) 5 Results Due Date: 31-Mar-2015

Date Approved: 31-Mar-2015

Approved By:

**Details:** Keith Jones, Technical Manager



# **Results Summary - Soil**

#### Project: C2099 - Clitheroe

Client: HSP Consulting Engineers Limited		Chem	ntest Jo	b No.:	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828
Quotation No.:	С	hemtes	st Samp	le ID.:	120111	120112	120113	120114	120115	120116	120117	120118
Order No.:		Clien	t Sample	e Ref.:								
		Clier	nt Samp	le ID.:	WS1	WS2	WS3A	WS6	WS7	WS8	WS5	WS5
			Sample		SOIL							
		7	op Dep	th (m):	0.5	0.5	0.1	0.5	0.5	0.6	0.5	0.1
			tom Dep									
			Date Sar	mpled:	17-Mar-15							
Determinand	Accred.	SOP	Units	LOD								
Moisture	N	2030	%	0.02	17	26	44	21	19	24	22	31
Soil Colour	N				Brown							
Other Material	N				Stones							
Soil Texture	N				Clay	Clay	Sand	Clay	Clay	Clay	Clay	Clay
рН	M	2010			6.9	7.2	5.7	7.4	6.5	7.5	7.0	6.2
Boron (Hot Water Soluble)	M	2120	mg/kg	0.4	< 0.40	0.41	1.9	0.66	0.51	0.52	< 0.40	1.8
Sulphate (2:1 Water Soluble) as SO4	M	2120	g/l	0.01	0.017	0.032	0.082	0.036	0.023	0.085	0.028	0.21
Total Sulphur	M	2175	%	0.01	< 0.010		0.070	< 0.010		< 0.010		
Sulphur (Elemental)	M	2180	mg/kg	1	< 1.0	< 1.0	4.2	4.0	< 1.0	< 1.0	< 1.0	6.5
Cyanide (Total)	M	2300	mg/kg	0.5	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50
Cyanide (Free)	M	2300	mg/kg	0.5	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50
Sulphide (Easily Liberatable)	M	2325	mg/kg	0.5	1.6	1.7	1.3	1.8	1.4	0.96	1.3	1.1
Sulphate (Acid Soluble)	M	2430	%	0.01	0.021		0.17	0.046		0.080		
Arsenic	M	2450	mg/kg	1	54	10	13	4.2	8.8	6.9	15	9.9
Cadmium	M	2450	mg/kg	0.1	< 0.10	0.21	0.77	0.41	0.85	0.42	2.3	0.50
Chromium	M	2450	mg/kg	1	28	29	33	19	26	23	29	25
Copper	M	2450	mg/kg	0.5	54	20	32	4.1	14	11	23	21
Mercury	M	2450	mg/kg	0.1	0.15	< 0.10	0.20	< 0.10	< 0.10	< 0.10	< 0.10	0.13
Nickel	M	2450	mg/kg	0.5	15	31	22	8.6	21	16	55	18
Lead	M	2450	mg/kg	0.5	85	27	100	23	45	31	56	65
Selenium	M	2450	mg/kg	0.2	0.52	0.25	0.72	0.27	0.38	0.23	1.0	0.34
Zinc	M	2450	mg/kg	0.5	75	80	130	68	130	95	280	100
Chromium (Hexavalent)	N	2490	mg/kg	0.5	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50	< 0.50
Organic Matter	M	2625	%	0.4	0.90	1.7	15	1.5	1.1	1.7	1.1	7.8
Aliphatic TPH >C5-C6	N		mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aliphatic TPH >C6-C8	N	2675	mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aliphatic TPH >C8-C10	М		mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aliphatic TPH >C10-C12	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C12-C16	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C16-C21	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C21-C35	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aliphatic TPH >C35-C44	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Total Aliphatic Hydrocarbons	M	2675	mg/kg	5	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0	< 5.0



# **Results Summary - Soil**

#### Project: C2099 - Clitheroe

Client: HSP Consulting Engineers Limited		Chen	ntest Jo	b No.:	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828
Quotation No.:	Chemtest Sample ID.:			120111	120112	120113	120114	120115	120116	120117	120118	
Order No.:	Client Sample Ref.: Client Sample ID.: Sample Type:			-	-		-			-		
				WS1	WS2	WS3A	WS6	WS7	WS8	WS5	WS5	
				SOIL								
		Top Depth (m):		0.5	0.5	0.1	0.5	0.5	0.6	0.5	0.1	
	Bottom Depth(m): Date Sampled:											
				17-Mar-15								
Determinand	Accred.	SOP	Units									
Aromatic TPH >C5-C7	N	2675	ma/ka	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aromatic TPH >C7-C8	N	2675	mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aromatic TPH >C8-C10	M	2675	mg/kg	0.1	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Aromatic TPH >C10-C12	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C12-C16	M	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Aromatic TPH >C16-C21	M	2675	mg/kg	1	< 1.0	< 1.0	3.9	< 1.0	< 1.0	< 1.0	< 1.0	2.6
Aromatic TPH >C21-C35	M	2675	mg/kg	1	< 1.0	< 1.0	11	< 1.0	< 1.0	< 1.0	< 1.0	11
Aromatic TPH >C35-C44	N	2675	mg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Total Aromatic Hydrocarbons	M	2675	mg/kg	5	< 5.0	< 5.0	15	< 5.0	< 5.0	< 5.0	< 5.0	15
Total Petroleum Hydrocarbons	M	2675	mg/kg	10	< 10	< 10	15	< 10	< 10	< 10	< 10	15
Naphthalene	M	2700	mg/kg	0.1	< 0.10	< 0.10	0.44	< 0.10	< 0.10	< 0.10	< 0.10	0.48
Acenaphthylene	M	2700	mg/kg	0.1	< 0.10	< 0.10	0.34	< 0.10	< 0.10	< 0.10	< 0.10	0.39
Acenaphthene	M		mg/kg	0.1	< 0.10	< 0.10	0.16	< 0.10	< 0.10	< 0.10	< 0.10	0.34
Fluorene	M	2700	mg/kg	0.1	< 0.10	< 0.10	0.15	< 0.10	< 0.10	< 0.10	< 0.10	0.45
Phenanthrene	M	2700	mg/kg	0.1	< 0.10	< 0.10	1.3	< 0.10	< 0.10	< 0.10	< 0.10	3.5
Anthracene	M	2700	mg/kg	0.1	< 0.10	< 0.10	0.33	< 0.10	< 0.10	< 0.10	< 0.10	< 0.10
Fluoranthene	M	2700	mg/kg	0.1	< 0.10	< 0.10	3.1	< 0.10	< 0.10	< 0.10	< 0.10	5.5
Pyrene	M	2700	mg/kg	0.1	< 0.10	< 0.10	3.2	< 0.10	< 0.10	< 0.10	< 0.10	5.5
Benzo[a]anthracene	M	2700	mg/kg		< 0.10	< 0.10	1.6	< 0.10	< 0.10	< 0.10	< 0.10	2.6
Chrysene	M	2700	mg/kg		< 0.10	< 0.10	2.3	< 0.10	< 0.10	< 0.10	< 0.10	3.3
Benzo[b]fluoranthene	M	2700	mg/kg	0.1	< 0.10	< 0.10	1.9	< 0.10	< 0.10	< 0.10	< 0.10	3.5
Benzo[k]fluoranthene	M	2700	mg/kg		< 0.10	< 0.10	0.34	< 0.10	< 0.10	< 0.10	< 0.10	2.0
Benzo[a]pyrene	M	2700	mg/kg	0.1	< 0.10	< 0.10	1.7	< 0.10	< 0.10	< 0.10	< 0.10	2.3
Indeno(1,2,3-c,d)Pyrene	M	2700	mg/kg	0.1	< 0.10	< 0.10	0.54	< 0.10	< 0.10	< 0.10	< 0.10	0.78
Dibenz(a,h)Anthracene	M	2700	mg/kg		< 0.10	< 0.10	0.44	< 0.10	< 0.10	< 0.10	< 0.10	0.43
Benzo[g,h,i]perylene	M	2700	mg/kg	0.1	< 0.10	< 0.10	0.45	< 0.10	< 0.10	< 0.10	< 0.10	0.47
Total Of 16 PAH's	M	2700	mg/kg	2	< 2.0	< 2.0	18	< 2.0	< 2.0	< 2.0	< 2.0	32
Benzene	M	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Toluene	M	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Ethylbenzene	M	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
m & p-Xylene	M	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
o-Xylene	M	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Methyl Tert-Butyl Ether	M	2760	μg/kg	1	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0



# **Results Summary - Soil**

#### Project: C2099 - Clitheroe

Client: HSP Consulting Engineers Limited	Chemtest Job No.:			15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	15-06828	
Quotation No.:	Chemtest Sample ID.:				120111	120112	120113	120114	120115	120116	120117	120118
Order No.:	Client Sample Ref.:											
	Client Sample ID.:				WS1	WS2	WS3A	WS6	WS7	WS8	WS5	WS5
	Sample Type:				SOIL							
	Top Depth (m):				0.5	0.5	0.1	0.5	0.5	0.6	0.5	0.1
	Bottom Depth(m):											
	Date Sampled:				17-Mar-15							
Determinand	Accred.	SOP	Units	LOD								
Total Phenols	M	2920	mg/kg	0.3	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30	< 0.30



#### **Report Information**

#### Key

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- M MCERTS and UKAS accredited
- N Unaccredited
- S This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
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- This analysis has been subcontracted to an unaccredited laboratory
- I/S Insufficient Sample
- U/S Unsuitable Sample
- N/E not evaluated
  - < "less than"
  - "greater than"

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVCOs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at our Coventry laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

#### **Sample Deviation Codes**

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container

#### Sample Retention and Disposal

All soil samples will be retained for a period of 60 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: customerservices@chemtest.co.uk



# **Final Report**



Chemtest Ltd.
Depot Road
Newmarket
CB8 0AL
Tel: 01638 606070

Email: info@chemtest.co.uk

Report Number: 15-06833 Issue-1

Initial Date of Issue: 31-Mar-2015

Client: HSP Consulting Engineers Limited

Lawrence House

Meadowbank Way

Client Address: Eastwood

Nottinghamshire

NG16 3SB

Contact(s): Luke Bradley

Project: C2099 - Clitheroe

Quotation No.: Date Received: 25-Mar-2015

Order No.: Date Instructed: 25-Mar-2015

No. of Samples: 2

Turnaround: (Wkdays) 5 Results Due Date: 31-Mar-2015

Date Approved: 31-Mar-2015

Approved By:

**Details:** Darrell Hall, Laboratory Director



# **Results Summary - 2 Stage WAC**

#### Project: C2099 - Clitheroe

Chemtest Job No: 15-06833 Chemtest Sample ID: 120134							Landfill Wa	aste Acceptar Limits	ce Criteria
Sample Ref: Sample ID: WS2 Top Depth(m): 0.5 Bottom Depth(m): Sampling Date: 17-Mar-2015 Determinand	SOP	Accred.	Units				Inert Waste Landfill	Stable Non- reactive Hazardous waste in non- hazardous	Hazardous Waste Landfill
Total Organic Carbon	2625	U	%			0.92	3	5	6
Loss on Ignition	2610	U	%			4.3			10
Total BTEX	2760	U	mg/kg			< 0.01	6		
Total PCBs (7 congeners)	2815	U	mg/kg			< 0.10	1		
TPH Total WAC (Mineral Oil)	2670	U	mg/kg			< 10	500		
Total (of 17) PAHs	2700	N	mg/kg			< 2.0	100		
рН	2010	U				7.1		>6	
Acid Neutralisation Capacity	2015	N	mol/kg			0.003		To evaluate	To evaluate
Eluate Analysis			2:1 mg/l	8:1 mg/l	2:1 mg/kg	Cumulative 10:1 mg/kg		s for complia S EN 12457-3	_
Arsenic	1450	U	< 0.001	0.002	< 0.050	< 0.050	0.5	2	25
Barium	1450	U	0.003	0.008	< 0.50	< 0.50	20	100	300
Cadmium	1450	U	0.00068	0.0035	< 0.010	0.033	0.04	1	5
Chromium	1450	U	0.004	0.011	< 0.050	0.11	0.5	10	70
Copper	1450	U	0.003	0.011	< 0.050	< 0.050	2	50	100
Mercury	1450	U	< 0.0005	< 0.0005	< 0.001	< 0.005	0.01	0.2	2
Molybdenum	1450	U	< 0.001	< 0.001	< 0.050	< 0.050	0.5	10	30
Nickel	1450	U	0.002	0.006	< 0.050	0.054	0.4	10	40
Lead	1450	U	0.001	0.005	< 0.010	0.048	0.5	10	50
Antimony	1450	U	< 0.001	< 0.001	< 0.010	< 0.010	0.06	0.7	5
Selenium	1450	U	0.001	< 0.001	< 0.010	< 0.010	0.1	0.5	7
Zinc	1450	U	0.033	0.14	< 0.50	1.3	4	50	200
Chloride	1220	U	4.6	1.3	< 10	14	800	15000	25000
Fluoride	1220	U	0.19	0.14	< 1.0	1.4	10	150	500
Sulphate	1220	U	4.9	< 1.0	< 10	< 10	1000	20000	50000
Total Dissolved Solids	1020	N	48	17	93	180	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.030	< 0.30	< 0.50	1	-	-
Dissolved Organic Carbon	1610	U	31	18	60	190	500	800	1000

Soild Information	
Dry mass of test portion/kg	0.175
Moisture (%)	23

Leachate Test Information							
Leachant volume 1st extract/l	0.296						
Leachant volume 2nd extract/l	1.4						
Eluant recovered from 1st extract/l	0.084						



# **Results Summary - 2 Stage WAC**

#### Project: C2099 - Clitheroe

Chemtest Job No: 15-06833 Chemtest Sample ID: 120135							Landfill Wa	aste Acceptan Limits	ce Criteria
Sample Ref: Sample ID: WS4 Top Depth(m): 0.1 Bottom Depth(m): Sampling Date: 17-Mar-2015 Determinand	SOP	Accred.	Units				Inert Waste Landfill	Stable Non- reactive Hazardous waste in non- hazardous	Hazardous Waste Landfill
Total Organic Carbon	2625	U	%			3.5	3	5	6
Loss on Ignition	2610	U	%			9.4			10
Total BTEX	2760	U	mg/kg			< 0.01	6		
Total PCBs (7 congeners)	2815	U	mg/kg			< 0.10	1		
TPH Total WAC (Mineral Oil)	2670	U	mg/kg			37	500		
Total (of 17) PAHs	2700	N	mg/kg			34	100		
рН	2010	U				7.2		>6	
Acid Neutralisation Capacity	2015	N	mol/kg			0.003		To evaluate	To evaluate
Eluate Analysis			2:1 mg/l	8:1 mg/l	2:1 mg/kg	Cumulative 10:1 mg/kg		s for compliar S EN 12457-3	_
Arsenic	1450	U	0.002	0.001	< 0.050	< 0.050	0.5	2	25
Barium	1450	U	0.014	0.019	< 0.50	< 0.50	20	100	300
Cadmium	1450	U	0.0021	0.0015	< 0.010	0.015	0.04	1	5
Chromium	1450	U	0.005	0.007	< 0.050	0.064	0.5	10	70
Copper	1450	U	0.01	0.012	< 0.050	< 0.050	2	50	100
Mercury	1450	U	< 0.0005	< 0.0005	< 0.001	< 0.005	0.01	0.2	2
Molybdenum	1450	U	< 0.001	< 0.001	< 0.050	< 0.050	0.5	10	30
Nickel	1450	U	0.005	0.004	< 0.050	< 0.050	0.4	10	40
Lead	1450	U	0.01	0.012	0.019	0.12	0.5	10	50
Antimony	1450	U	< 0.001	< 0.001	< 0.010	< 0.010	0.06	0.7	5
Selenium	1450	U	0.001	0.001	< 0.010	0.011	0.1	0.5	7
Zinc	1450	U	0.077	0.05	< 0.50	0.51	4	50	200
Chloride	1220	U	4.1	2	< 10	21	800	15000	25000
Fluoride	1220	U	0.13	0.085	< 1.0	< 1.0	10	150	500
Sulphate	1220	U	15	2.2	29	28	1000	20000	50000
Total Dissolved Solids	1020	N	60	19	120	210	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.030	< 0.30	< 0.50	1	-	-
Dissolved Organic Carbon	1610	U	83	22	160	250	500	800	1000

Soild Information						
Dry mass of test portion/kg	0.175					
Moisture (%)	18					

Leachate Test Information							
Leachant volume 1st extract/l	0.313						
Leachant volume 2nd extract/l	1.4						
Eluant recovered from 1st extract/l	0.086						



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If you require extended retention of samples, please email your requirements to: customerservices@chemtest.co.uk



# **Appendix VI**



Project Number C2099

Project Name Land off ChatburnRoad, Clitheroe

				Detecti	on Lim	it				
		<0.1	<0.1	<0.1	<0.1	<1	<1			
Hole Number	Gas Flow Rate. (I/hr)	Methane. (%LEL)	Methane. (%vol)	Oxygen. (%vol)	Carbon Dioxide. (%vol)	Carbon Monoxide. (ppm)	Hydrogen Sulphide. (ppm)		Depth of Installation. (mbgl)	Depth of Groundwater (mbgl)
CP1	<0.1	<0.1	<0.1	19.2	0.5	<1	1		4.22	4.16
CP2	<0.1	<0.1	<0.1	20.1	0.0	13	2		5.75	2.00
CP3	1.2	<0.1	<0.1	20.2	0.4	5	2		6.65	1.10
WS2	<0.1	<0.1	<0.1	17.6	1.6	<1	<1		2.55	DRY
WS6	<0.1	<0.1	<0.1	20.8	0.2	8	<1		1.82	0.62
WS8	<0.1	<0.1	<0.1	20.1	0.8	5	1	Ш	1.94	1.80
								Ш		
								Ц		
		1						Ц		

Date	Not	es:	Barometric Pressure,	006
27.03.2015	Engineer	DRS	mbar	986
			Pressure Trend	Rising
	Equipment	GFM430	Air Temperature	8°C



Project Number C2099

Project Name Land off ChatburnRoad, Clitheroe

			Detection Limit							
		<0.1	<0.1	<0.1	<0.1	<1	<1			
Hole Number	Gas Flow Rate. (I/hr)	Methane. (%LEL)	Methane. (%vol)	Oxygen. (%vol)	Carbon Dioxide. (%vol)	Carbon Monoxide. (ppm)	Hydrogen Sulphide. (ppm)		Depth of Installation. (mbgl)	Depth of Groundwater (mbgl)
CP1	<0.1	<0.1	<0.1	19.6	0.5	13	1		8.97	1.40
CP2	< 0.1	<0.1	<0.1	18.0	<0.1	13	0		9.58	2.05
CP3	< 0.1	<0.1	<0.1	17.9	1.1	0	0		9.57	0.90
WS2	< 0.1	< 0.1	<0.1	16.3	2.2	5	0		2.55	DRY
WS6	<0.1	<0.1	<0.1	20.3	0.6	0	0		1.82	0.76
WS8	<0.1	<0.1	<0.1	19.2	1.2	5	3		1.94	1.38
								Ш		
								Ш		
								Ц		

Date	Not	es:	Barometric Pressure,	1020
16.04.2015	Engineer HJD		mbar	1020
			Pressure Trend	Rising
	Equipment	GFM430	Air Temperature	13°C



Project Number C2099

Project Name Land off ChatburnRoad, Clitheroe

				Detecti	on Lim	it				
		<0.1	<0.1	<0.1	<0.1	<1	<1			
Hole Number	Gas Flow Rate. (I/hr)	Methane. (%LEL)	Methane. (%vol)	Oxygen. (%vol)	Carbon Dioxide. (%vol)	Carbon Monoxide. (ppm)	Hydrogen Sulphide. (ppm)		Depth of Installation. (mbgl)	Depth of Groundwater (mbgl)
CP1	<0.1	<0.1	<0.1	19.7	0.6	<1	1		4.20	1.47
CP2	< 0.1	<0.1	<0.1	20.4	0.0	11	2		5.75	2.10
CP3	< 0.1	<0.1	<0.1	12.1	2.9	8	1		6.55	1.35
WS2	< 0.1	< 0.1	<0.1	16.9	2.5	5	<1		2.55	DRY
WS6	< 0.1	<0.1	<0.1	19.6	1.2	8	<1		1.95	1.44
WS8	< 0.1	<0.1	<0.1	20.7	0.5	8	2		1.80	0.85
	<u> </u>							L		
								L		

Date	No	tes:	Barometric Pressure,	006
24.04.2015	Engineer	DRS	mbar	996
			Pressure Trend	Falling
	Equipment	GFM430	Air Temperature	13°C



Project Number C2099

Project Name Land off ChatburnRoad, Clitheroe

			Detection Limit						
		<0.1	<0.1	<0.1	<0.1	<1	<1		
Hole Number	Gas Flow Rate. (I/hr)	Methane. (%LEL)	Methane. (%vol)	Oxygen. (%vol)	Carbon Dioxide. (%vol)	Carbon Monoxide. (ppm)	Hydrogen Sulphide. (ppm)	Depth of Installation. (mbgl)	Depth of Groundwater (mbgl)
CP1	<0.1	<0.1	<0.1	20.8	<0.1	<1	2	4.18	0.52
CP2	<0.1	<0.1	<0.1	18.1	0.3	<1	3	5.72	1.82
CP3	<0.1	<0.1	<0.1	10.0	4.4	13	2	6.55	1.06
WS2	< 0.1	<0.1	<0.1	18.0	2.1	5	<1	2.54	2.46
WS6	<0.1	<0.1	<0.1	21.0	<0.1	8	2	1.96	0.95
WS8	<0.1	<0.1	<0.1	19.8	0.5	<1	13	1.82	0.70

Date	No	tes:	Barometric Pressure,	1001	
08.05.2015	Engineer	DRS	mbar	1001	
			Pressure Trend	Falling	
	Equipment	GFM430	Air Temperature	12°C	



Project Number C2099

Project Name Land off ChatburnRoad, Clitheroe

				Detecti	on Limi	t				
		<0.1	<0.1	<0.1	<0.1	<1	<1			
Hole Number	Gas Flow Rate. (I/hr)	Methane. (%LEL)	Methane. (%vol)	Oxygen. (%vol)	Carbon Dioxide. (%vol)	Carbon Monoxide. (ppm)	Hydrogen Sulphide. (ppm)		Depth of Installation. (mbgl)	Depth of Groundwater (mbgl)
CP1	<0.1	<0.1	<0.1	20.6	0.2	8	<1		4.20	1.27
CP2	<0.1	<0.1	<0.1	18.0	<0.1	<1	<1		5.73	1.97
CP3	<0.1	<0.1	<0.1	13.4	2.4	<1	<1		6.55	0.85
WS2	< 0.1	< 0.1	<0.1	17.2	2.3	<1	3		2.33	2.46
WS6	<0.1	<0.1	<0.1	20.8	0.1	<1	5		1.77	0.73
WS8	<0.1	<0.1	<0.1	19.1	1.0	<1	<1		1.95	1.27
								Ш		
								Ш		
								Ш		

Date	Not	es:	Barometric Pressure,	1000	
26.05.2015	Engineer	DRS	mbar	1008	
			Pressure Trend	Steady	
	Equipment	GFM430	Air Temperature	15°C	



Project Number C2099

Project Name Land off ChatburnRoad, Clitheroe

			Detection Limit							
		<0.1	<0.1	<0.1	<0.1	<1	<1			
Hole Number	Gas Flow Rate. (I/hr)	Methane. (%LEL)	Methane. (%vol)	Oxygen. (%vol)	Carbon Dioxide. (%vol)	Carbon Monoxide. (ppm)	Hydrogen Sulphide. (ppm)		Depth of Installation. (mbgl)	Depth of Groundwater (mbgl)
CP1	<0.1	<0.1	<0.1	13.9	2.1	2	1		4.17	1.42
CP2	< 0.1	<0.1	<0.1	18.4	<0.1	<1	1		5.76	2.13
CP3	<0.1	< 0.1	<0.1	13.1	4.2	2	2		6.37	1.40
WS2	< 0.1	< 0.1	<0.1	18.5	3.3	2	1		2.54	2.43
WS6	< 0.1	<0.1	<0.1	20.1	0.8	<1	2		1.75	0.86
WS8	<0.1	<0.1	<0.1	19.7	1.4	<1	1		1.93	1.46
								Ц		
								Ц		

Date	Not	es:	Barometric Pressure,	1007	
24.06.2015	Engineer	DRS	mbar	1007	
			Pressure Trend	Steady	
	Equipment	GFM430	Air Temperature	17°C	

# Gas Testing Summary



Project Number	C2099
Project Name	Land off Chatburn Road,
Client	Clitheroe Oakmere Homes

	Methane (%LEL)							
CP1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1		
CP2	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1		
CP3	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1		
WS2	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1		
WS6	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1		
WS8	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1		

	Methane (%Vol)								
CP1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1			
CP2	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1			
CP3	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1			
WS2	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1			
WS6	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1			
WS8	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1			

	Oxygen. (%vol)						
CP1	19.2	19.6	19.7	20.8	20.6	13.9	
CP2	20.1	18	20.4	18.1	18	18.4	
CP3	20.2	17.9	12.1	10	13.4	13.1	
WS2	17.6	16.3	16.9	18	17.2	18.5	
WS6	20.8	20.3	19.6	21	20.8	20.1	
WS8	20.1	19.2	20.7	19.8	19.1	19.7	

# Gas Testing Summary



Project Number	C2099
Project Name	Land off Chatburn Road,
Client	Clitheroe Oakmere Homes

	Carbon Dioxide. (%vol)							
CP1	0.5	0.5	0.6	<0.1	0.2	2.1		
CP2	0	<0.1	0	0.3	<0.1	<0.1		
CP3	0.4	1.1	2.9	4.4	2.4	4.2		
WS2	1.6	2.2	2.5	2.1	2.3	3.3		
WS6	0.2	0.6	1.2	<0.1	0.1	0.8		
WS8	0.8	1.2	0.5	0.5	1	1.4		

	Carbon Monoxide. (ppm)					
CP1	<1	13	<1	<1	8	2
CP2	13	13	11	<1	<1	<1
CP3	5	0	8	13	<1	2
WS2	<1	5	5	5	<1	2
WS6	8	0	8	8	<1	<1
WS8	5	5	8	<1	<1	<1

	Hydrogen Sulphide. (ppm)					
CP1	1	1	1	2	<1	1
CP2	2	0	2	3	<1	1
CP3	2	0	1	2	<1	2
WS2	<1	0	<1	<1	3	1
WS6	<1	0	<1	2	5	2
WS8	1	3	2	13	<1	1

# Gas Testing Summary



Project Number	C2099
	Land off Chatburn Road,
	Clitheroe Oakmere Homes

	Gas Flow Rate (I/hr)						
CP1	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	
CP2	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	
CP3	1.2	<0.1	<0.1	<0.1	<0.1	<0.1	
WS2	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	
WS6	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	
WS8	<0.1	<0.1	<0.1	<0.1	<0.1	<0.1	

Atmospheric Pressure Range						
986	1020	996	1001	1008	1007	

Max Methane Concentration (%vol)	0.1
Max Carbon Dioxide Concentration (%vol)	4.4
Max Carbon Monoxide Concentration (ppm)	13
Max Hydrogen Sulphide Concentration (ppm)	13
Max Flow Rate (I/hr)	1.2
Methane Gas Screening Value	0.0012
Carbon Dioxide Gas Screening Value	0.0528
Carbon Monoxide Gas Screening Value	0.156
Hydrogen Sulphide Gas Screening Value	0.156
Maximum Gas Screening Value	0.0528
Characteristic Situation 1	PASS
Characteristic Situation 2	PASS
Characteristic Situation 3	PASS
Characteristic Situation 4	PASS
Characteristic Situation 5	PASS
Characteristic Situation 6	PASS

TEST DATE AND CONDITIONS				
Date	17/06/15			
Atmospheric Pressure	1010mB			
Ambient Temp	24.3°C			
Environics Serial No.	2633			

GAS DATA LTD	1
Pegasus House	
Seven Stars Estate	
Wheler Rd	
Coventry	
CV34LB	
Tel 02476303311 Fax 024	76307711

#### **GFM430-1 FINAL INSPECTION & CALIBRATION CHECK CERTIFICATE**

PRESSURE CHECKS							
Calibration Pressure			Instrun	nent Pressure	Channels	s Read	
Pressure @	Applied	Atmospheric	tol.				
	Pressure	rApl ( mB)	(mB)				
All Potts	Cunent Atmospheric	1011	+/-2.0				
Ap Port	+800mB(a)	797	+/-5.0		I		
(Internal)	+1200mB(a)	1199	+/-5.0				

FLOW CHECKS							
Calibra	tion Flow	Inst	trument Flo	w Channels Read			
Applied	Applied	Flow	tol.	Differential Pressure	tol.		
Flow (I/hour)	Pressure (Pa)	fFlow <b>l</b> (1/hour)	( l/hour)	fDpl (Pa)	(Pa)		
-30.0	319	-30.1	+/-3.0	-325	+/-50		
-3.0	-15	-3.1	+/-1.0	-16	+/-6		
0.0	0	0.0	0.0	0	0.0		
+3.0	14	3.0	+/-0.5	13	+/-3		
+15.0	115	15.1	+/-1.5	115	+/-20		
+30.0	320	30.0	+/-3.0	320	+/-50		
+60.0	981	60.8	+/-6.0	994	+/-130		
+90.0	1931	91.3	+/-9.0	1989	+/-250		

TEMPERATURE CHECK					
Calibration Temperature	Instrument Temperature Channel Read				
Applied Equivalent	Temperature	tol.			
Temperature (°C)	rTempl ( °C)	(oc)			
-10.0	-10.5	+/- 2.0			
0.0	0.0	+/- 1.0			
30.0	30.0	+/- 1.0			
60.0	60.0	+/- 1.0			
100.0	99.5	+/- 1.0			

#### **Notes:**

The instrument identified by the serial number stated above has been tested by Gas Data personnel for calibration accuracy on the date and under the ambient conditions stated. Gas Data Ltd internal BS EN ISO900I:2008 compliant workshop procedures were followed to apply known calibration test gases, gas flow rates, pressures and temperatures of the values stated. The results displayed on the instrument at each stage are recorded above.

Gas Data Ltd is certified to BS EN ISO9001:2008. Certificate NQA 8374. Valid until 20 /03 /2016

TEST DATE AND CONDITIONS				
Date	17/06/15			
Atmospheric Pressure	IOIOmB			
Ambient Temp	24.3°c			
Environics Serial No.	2633			

# GAS DATA LTD Pegasus House Seven Stars Estate Wheler Rd Coventry CV34LB Tel 02476303311 Fax 02476307711

#### **GFM430-1 FINAL INSPECTION & CALIBRATION CHECK CERTIFICATE**

INSTRUMENT DETAILS					
Serial No	Customer				
10152	HSP Consulting Engineers Limited				

INSTRUMENT CHECKS						
Keyboard	./	Pump Flow	500cc/min			
Display Contrast	./	Pump Flow (a) -200mB	450cc/min			
Clock Set / Running	./	S/W Version	G430.0024/0013			
Labels Fitted	./	Recalibration Date	17/06/16			

GAS CHECKS							
Calibration Gas		Instrument Gas Channels Read					
Gas Type	Applied	plied CH4 tol. CO2 tol. 02				02	to!.
	Cone.	(%)	(%vol.)	(%)	(%vol.)	(%)	(%vol.)
N2	100%	0.0	0.0	0.0	0.0	0.0	+0.1
CH4	5%	5.1	+1-0.3	0.0	0.0	0.0	+0.1
	60%	60.1	+/-3.0	0.0	0.0	0.0	+0.1
CO2	5%	0.0	0.0	4.9	+/-0.3	0.0	+0.1
	40%	0.0	0.0	40.3	+/-3.0	0.0	+0.1
Air (20.9% 02, 400ppm CO2)	100%	0.0	0.0	0.1	+0.1	20.9	+1-0.5

OPTIONAL GAS CHECKS								
Calibration Gas				Instrume	nt Gas Char	nnels Read		
Gas	Applied	Label	H2S	со				tol.
Type	Cone.	Range	2000ppm	2000ppm				(% vol.)
N2	100%		0	0				0.0
H2S	1500ppm		1500	0				+/- 5.0
со	1000ppm		35	1000				+/- 5.0
								+/- 5.0
								+/- 5.0
								+/- 5.0

# TEST DATE AND CONDITIONS Date / t ,6 · (t:, Atmospheric Pressure (QIQ mB Ambient Temp 20 / 00 Environics Serial No.

<b>&amp;</b> SDATALTD	
egasus House	
eSen Stars Estate	
h <b>w</b> er Rd.	
Coventry	
CV3 4LB	
etr: 024 76 303311	Fax: 024 76 30771 I

#### **GFM430-1 OUTWARD INSPECTION & QUALITY CHECK SHEET**

	IN	STRUMENT DETA	ILS		
SO Number   Instrument Type   In		Instrument Serial	Job Number(s)		
l N		mher+ SW Version	I (D I 1-		_
P:J lcl -h	C.,F-tvtu?{)	mber+ SW Yersion			
Calibration	Technician	•••••	DATE	.!.?.	f(!5
	6			n	, ,S
Inspection '	Technician		DAT	E <b>1</b> 7	7 7
	INSTRUMENT CHECKS	Pass (P), Fail (F) or not aoolicable (NA)	INSTRUMENT LIST	PACKING	Tick if included
Function	bust Caps Fitted	,)	n str ument		11
h'ests	Keyboard Test (All Keys) Backlight Test	- <u>r.</u> ;	Leather Case Lustrument Strap		V
	Dackingin 163t		1	\.\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	
	lock Set / Running	[)	IAC Battery Charger (	UK)	V
	omms Test	r	v\C Battery Charger (	EURO)	У
	Pump Flow Test (In & Out)	/J	IAC Battery Charger (	US)	7
	overall Leak Test (30mB)	NA	AC Batte1y Charger (AUS)		\/
	Batte1yCharge Test		Gas Sample Pipe	, and the second	
	Service Date set to?	t,7 6⋅ (.f	=any Case		)(*
2 hannel Test	Data Logging Enabled?	INR	Spares Pot		Y'
	Verify CH4/LEL	r_,)	Allen Key		,),(
	Verify CO2		Flow Sample Pipe		V
	Verify 02	J	Temperature Probe		У
	Verify LEL	J	Vane Anemometer		χ-,
	Verify 1st Option gas H'/""	U US	B Cable		t
	Verify 2nd Option gas	u	USB Memory stick		y.
	Verify 3rd Option gas	r\IA S	iteMan Software	Ver	y
	Verify 4th Option gas	Nft	Internal Filter Pack	Qty	y
	Verify atmosp heric pressure Verify static pressure		External Filter Pack	Qty	V
	Verify static pressure	ΪĴΑ	field Guide	У	
	Verify differential pressure	<b>[</b> [.)	bperation Manual (hard copy) y		
	Verify flow		Extra He ms:		
	Verify temperature probe input	V			
	Verify vane anemometer input	_'lJ			
DataBase	Jobcard(s) completed and signed	[.)	1		
Checks	Jobcard(s) booked off database		1		
	2 alibration ce1tificate completed	{-J			
	Complete & print QI record	NA-	Comments:		
Label Checks	No. of Calibration label fitted	,∖-: <b>J</b> ., ,,,,)H	7		
	Warranty label fitted	V			
H2S Range	H2S Range from Sales Order	'/ ( L.')	†		
	H2S Range from Cal Cert	j JI)	1		
	Over-range value correct?	μ	1		