

6.7 From IH 108 Table 7.1:

Curve 10: Q95(1) percentage of 10.0% is closest to Q95(1) of 10.7% given by soil

Percentile	% Mean Flow	Flow (m ³ /s)
2	428.96	0.0425
5	303.93	0.0301
50	52.46	0.0052
80	21.25	0.0021
90	13.75	0.0014
95	10.00	0.0010
99	5.89	0.0006

Table 5: Flow duration

6.8 Flow duration curve is shown in Figure 8.

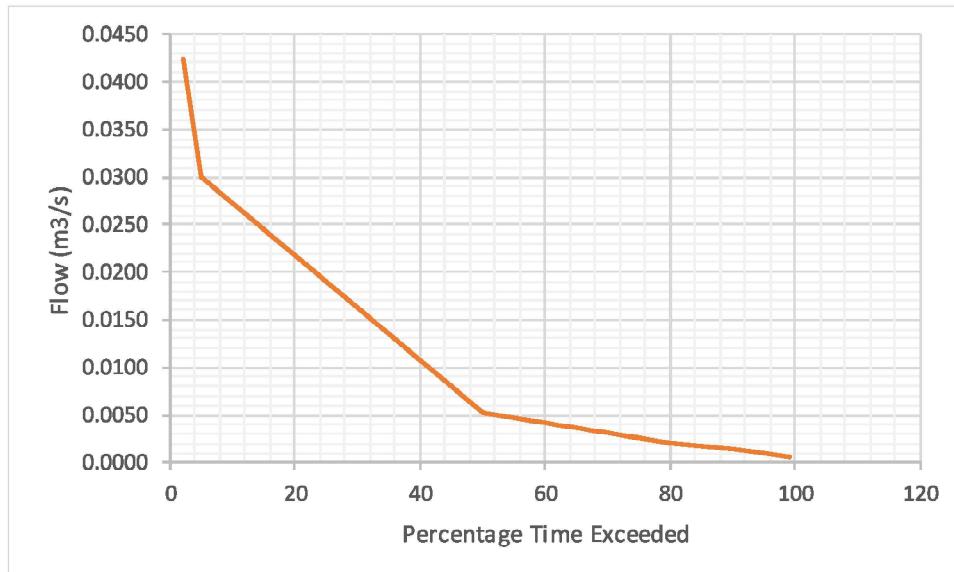


Figure 8: Flow Duration Curve

- 6.9 The Q95(1) flow of 0.001 m³/s is too low to be run in the hydraulic model, and so a Manning's equation calculation has been undertaken on a typical cross section to determine the typical water level. The typical cross section is shown in Figure 9.

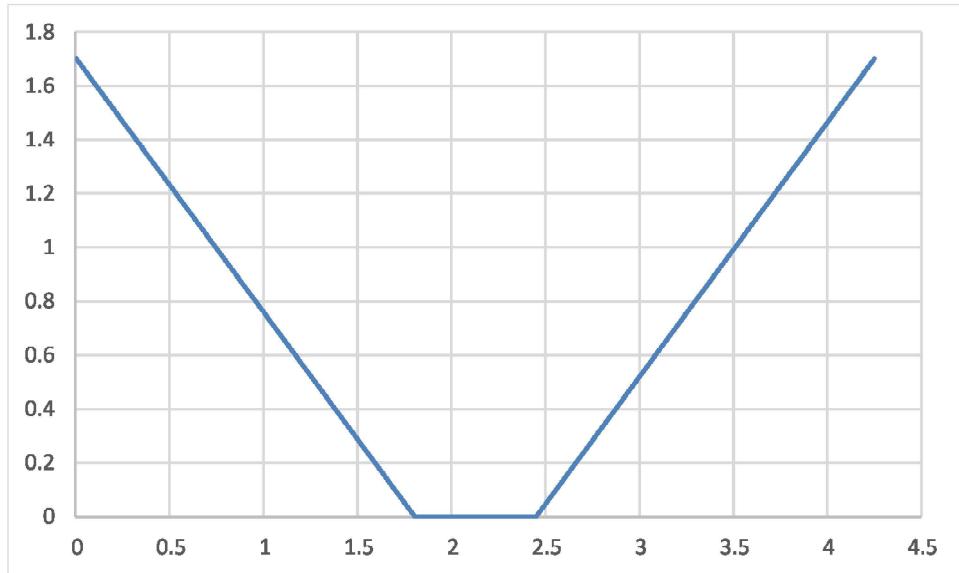


Figure 9: Typical cross section

6.10 Manning's equation is as follows:

$$Q = \frac{AR^{2/3}\sqrt{S}}{n}$$

where Q is flow, A is area of flow, R is hydraulic radius and S is gradient.

6.11 Using the average gradient of 0.025 and a Manning's roughness coefficient of 0.06, Manning's equation yields:

$$A = \frac{Qn}{R^{2/3}\sqrt{S}}$$

$$A = \frac{0.01 \times 0.06}{0.011^{2/3}\sqrt{0.025}}$$

$$A = 0.008 \text{ m}^3$$

6.12 The flow area of 0.008m³ corresponds to a depth in the typical channel cross section of 0.012m. It is therefore recommended that the invert levels of surface water outfalls be set at 300mm above this level.

7.0 CONCLUSIONS

- 6.1 The hydraulic assessment has indicated that peak water levels in the watercourses remain largely within banks for events up to the 1% AEP plus climate change.
- 6.2 A thorough sensitivity analysis of key parameters has been undertaken and has shown that the model results are not significantly affected by changes in those parameters.
- 6.3 A low flow analysis was undertaken to determine the Q95(1) flow. The Q95(1) flow was calculated to be 0.001m³/s.
- 6.4 A Manning's equation calculation provided a typical depth in the channel of 0.012m. It is recommended that the invert levels of the surface water outfalls be set at 300mm above the Q95(1) water level.

BIBLIOGRAPHY & REFERENCES

National Planning Policy Framework, CLG (2012).

Planning Practice Guidance, CLG (2014)

Institute of Hydrology Report No. 108 (1992)

Web-based References

Bingmaps – <http://www.bing.com/Maps/>

British Geological Survey – <http://www.bgs.ac.uk/opengeoscience/home.html>

Chronology of British Hydrological Events – www.dundee.ac.uk/

CIRIA – <http://www.ciria.org/>

Cranfield University – <http://www.landis.org.uk/soilscapes/>

Environment Agency – www.environment-agency.gov.uk/

FloodProBE – <http://www.floodprobe.eu/>

Flood Forum – <http://www.floodforum.org.uk/>

Flood London – <http://www.floodlondon.com/>

Flood Resilience Group – <http://www.floodresiliencegroup.org/frg/>

Fylde Borough Council – <http://www.fylde.gov.uk/>

Google Maps – <http://maps.google.co.uk/>

Lancashire County Council – <http://www.lancashire.gov.uk/home/2010/classic/index.asp>

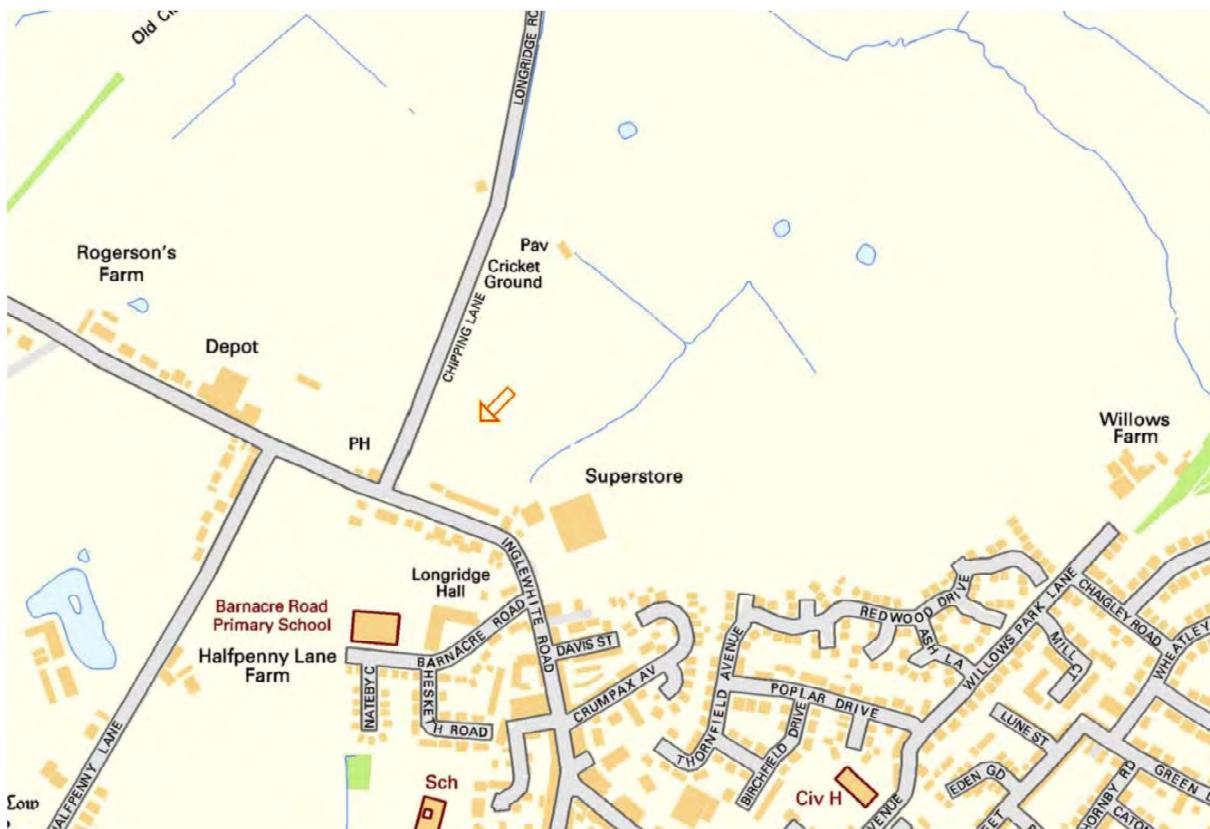
Streetmap – <http://www.streetmap.co.uk/>

United Utilities - <http://www.unitedutilities.com/default.aspx>

APPENDIX A: LOCATION PLAN

This page has been left intentionally blank

This page has been left intentionally blank



OS X (Eastings) 360073

OS Y (Northings) 437980

Nearest Post Code PR3 2NA

Lat (WGS84) N53:50:12 (53.836529)

Long (WGS84) W2.36.30 (-2.608205)

Lat,Long 53.836529,-2.608205

Nat Grid SD600379 / SD6007337980

This page has been left intentionally blank

APPENDIX B: INDICATIVE PLANNING LAYOUT

This page has been left intentionally blank

This page has been left intentionally blank



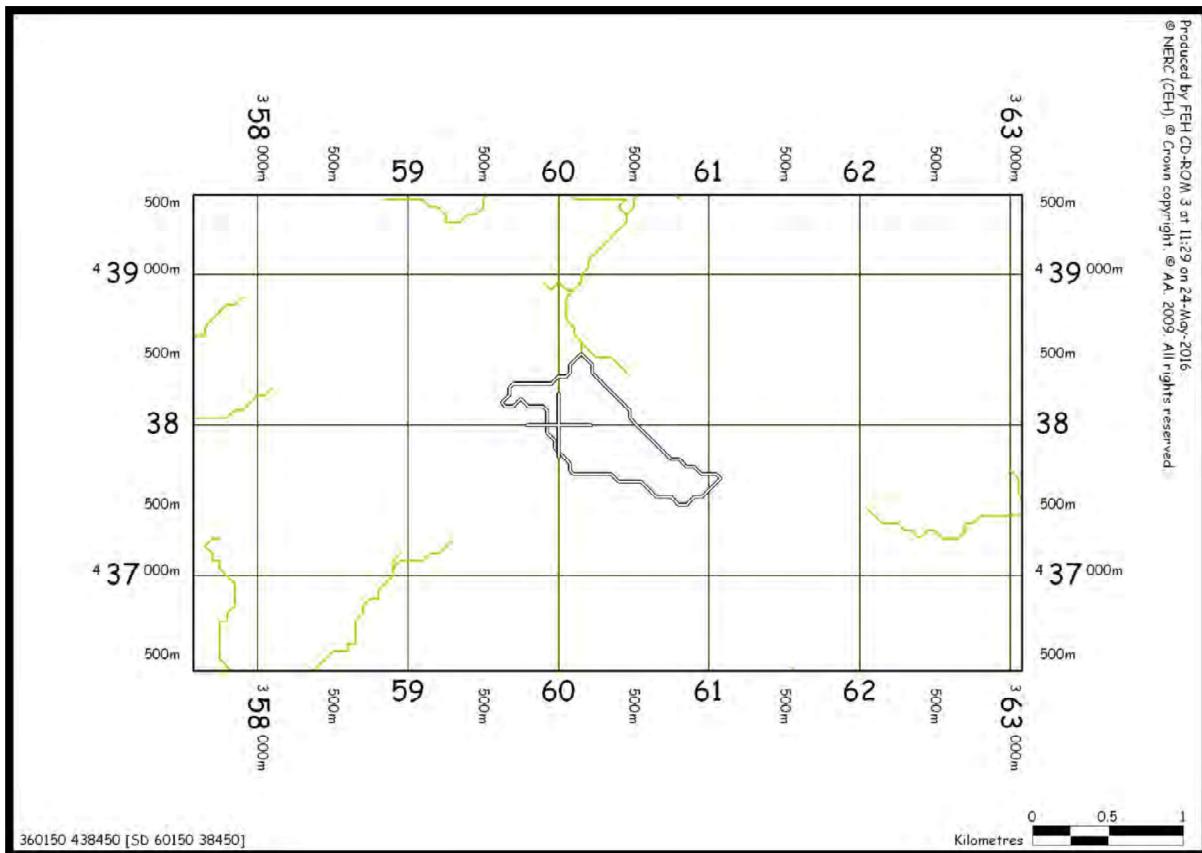
This page has been left intentionally blank

APPENDIX C: FEH CATCHMENT DATA & DESCRIPTIONS

This page has been left intentionally blank

This page has been left intentionally blank

Higgin Brook catchment and catchment characteristics



AREA	0.52	URBLOC1990	1.515
ALTBAR	115	C	-0.025
ASPBAR	325	D1	0.40671
ASPVAR	0.65	D2	0.33211
BFIHOST	0.417	D3	0.41529
DPLBAR	0.77	E	0.29629
DPSBAR	22.3	F	2.45864
FARL	1	C(1 km)	-0.025
LDP	1.58	D1(1 km)	0.404
PROPWET	0.51	D2(1 km)	0.33
RMED-1H	10.5	D3(1 km)	0.417
RMED-1D	39.7	E(1 km)	0.296
RMED-2D	51.6	F(1 km)	2.453
SAAR	1200		
SAAR4170	1137		
SPRHOST	35.03		
URBCONC1990	0.964		
URBEXT1990	0.1643		

This page has been left intentionally blank

APPENDIX D: REVITALISED FLOOD HYDROGRAPH METHOD OUTPUTS [PEAK FLOW ESTIMATES]

This page has been left intentionally blank

This page has been left intentionally blank

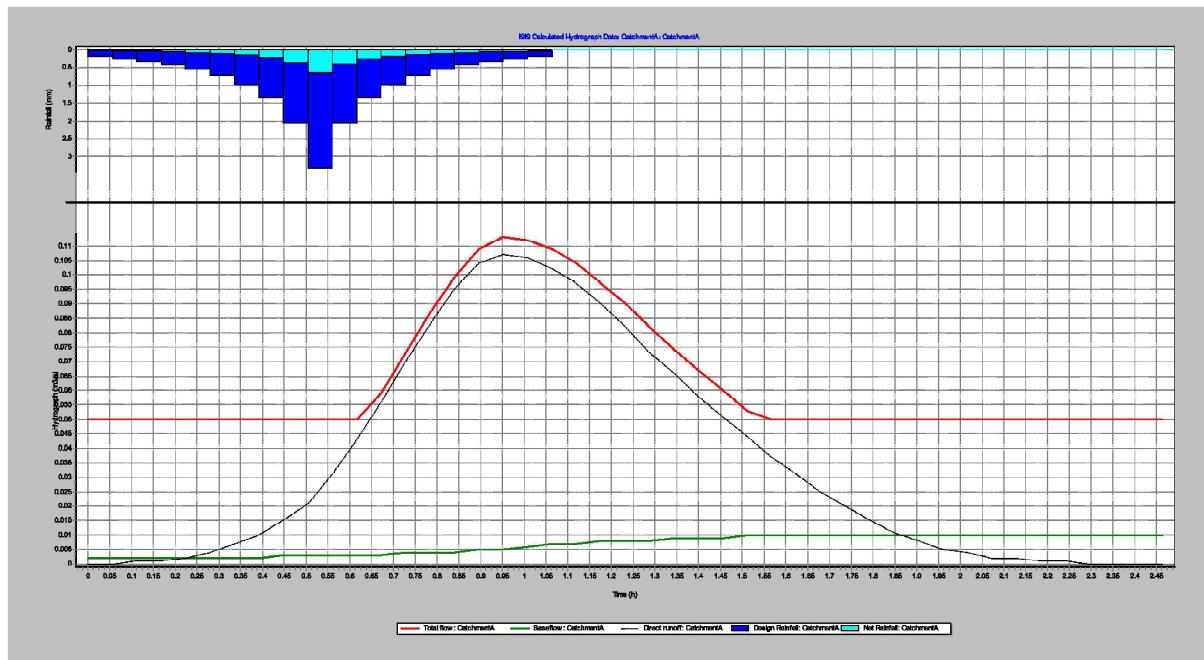


Figure D.1 Sub-catchment A 1 in 5 year (20% AEP) flow hydrograph

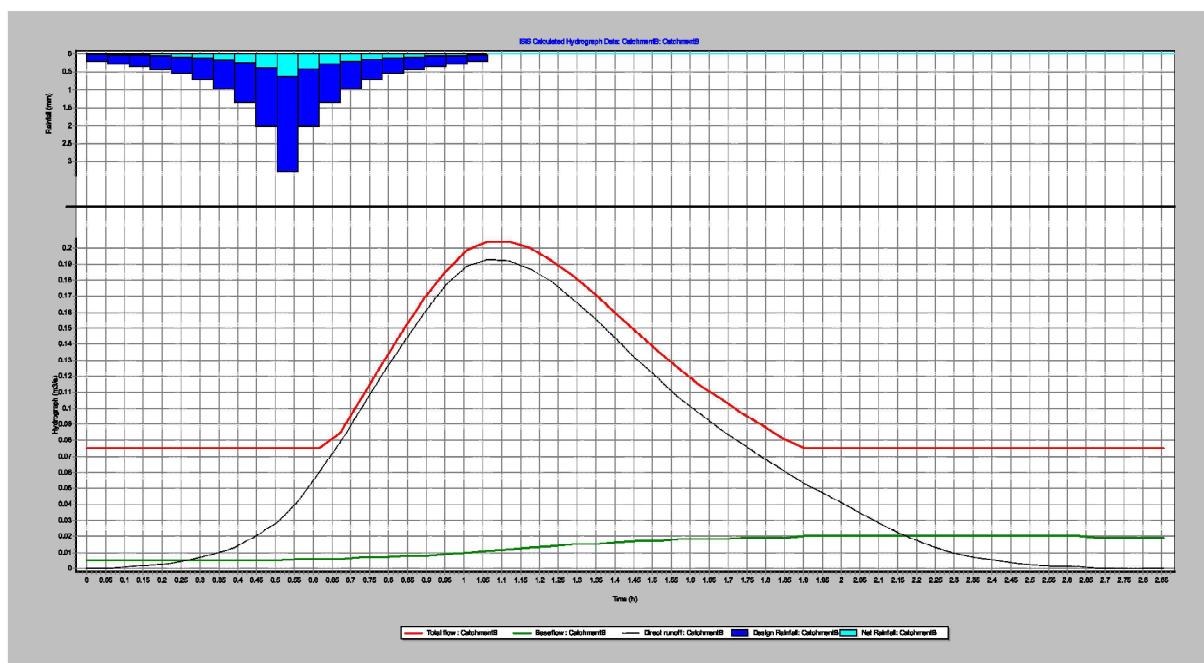


Figure D.2 Sub-catchment B 1 in 5 year (20% AEP) flow hydrograph

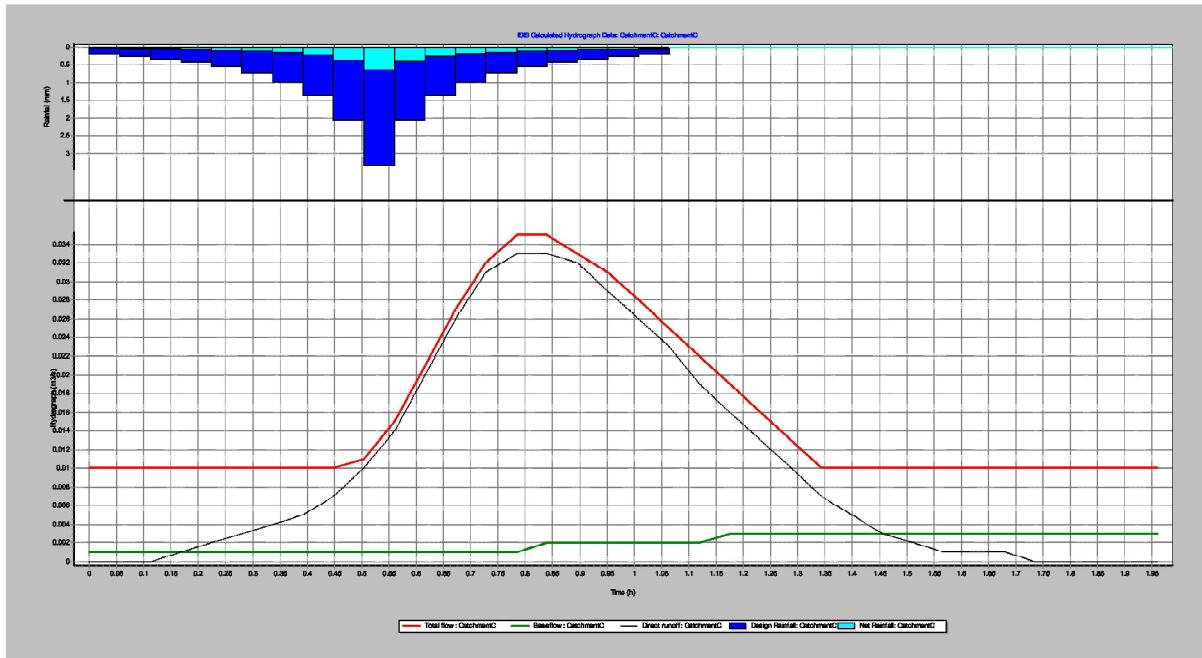


Figure D.3 Sub-catchment C 1 in 5 year (20% AEP) flow hydrograph

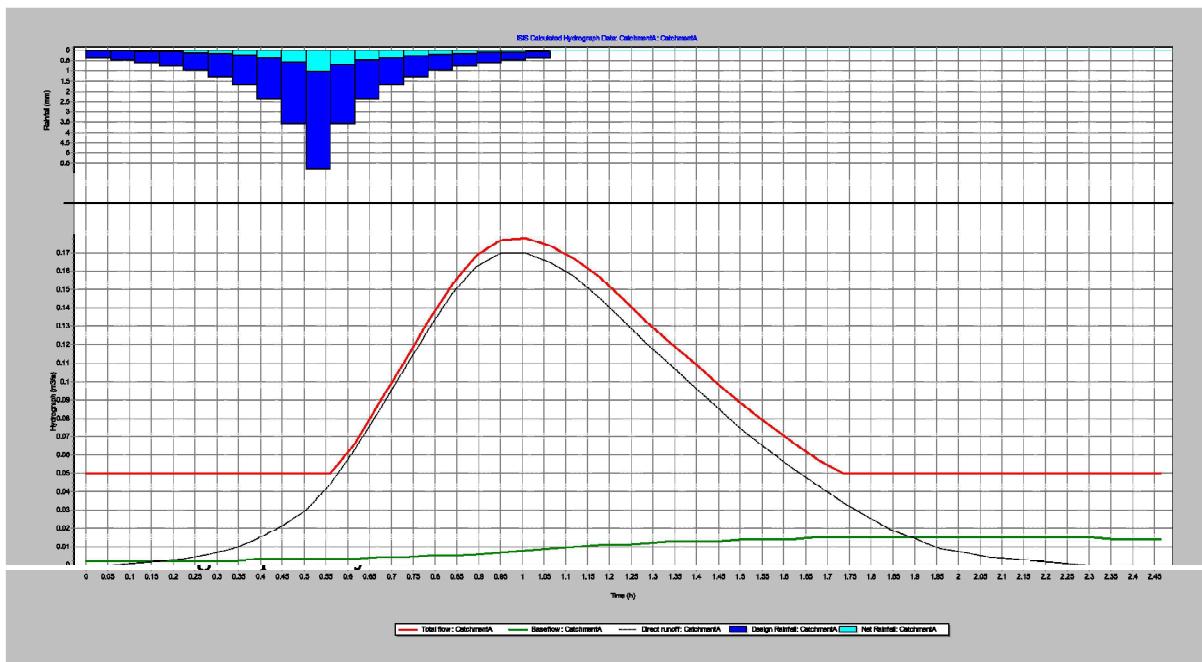


Figure D.4 Sub-catchment A 1 in 30 year (3.3% AEP) flow hydrograph

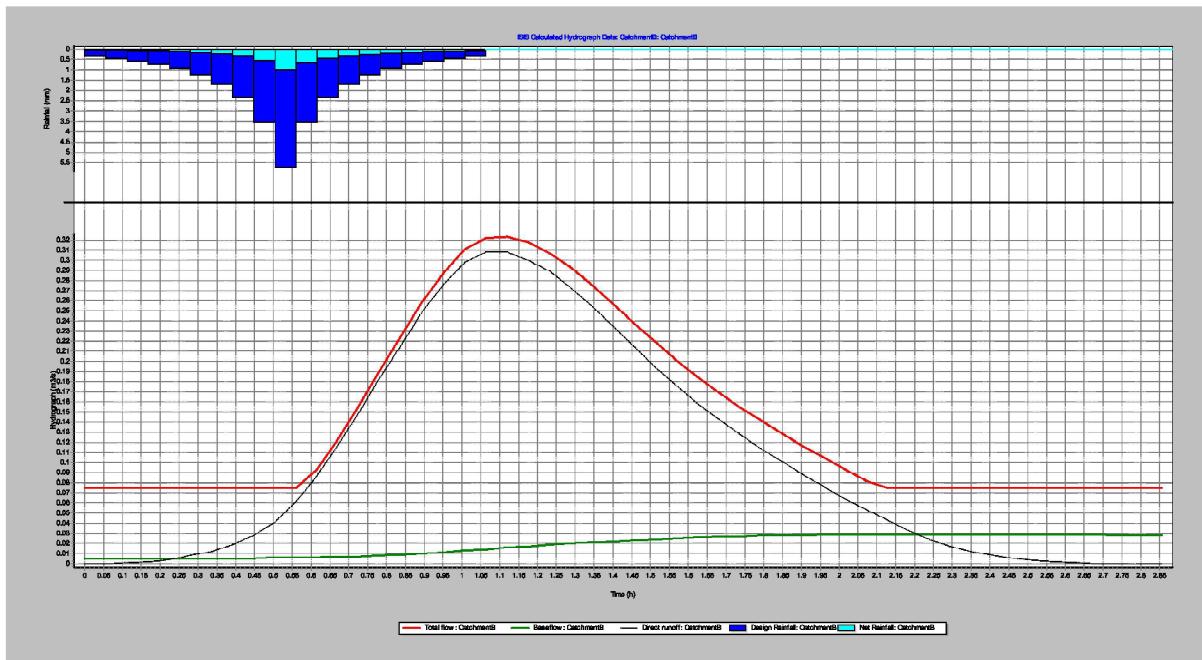


Figure D.5 Sub-catchment B 1 in 30 year (3.3% AEP) flow hydrograph

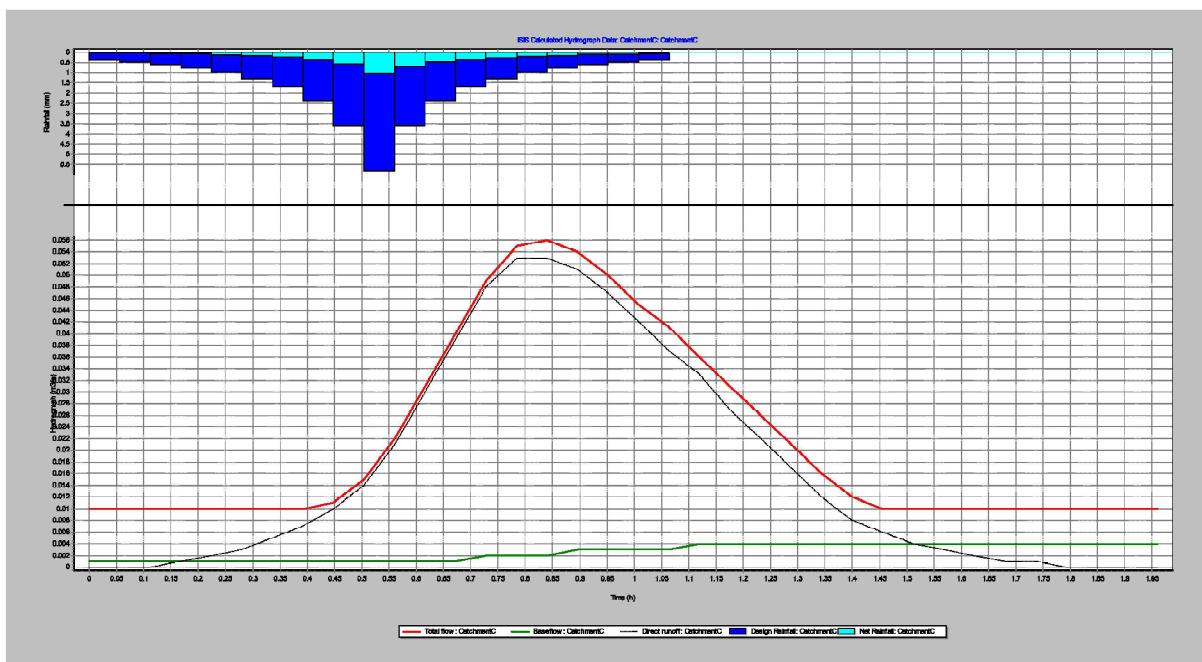


Figure D.6 Sub-catchment C 1 in 30 year (3.3% AEP) flow hydrograph

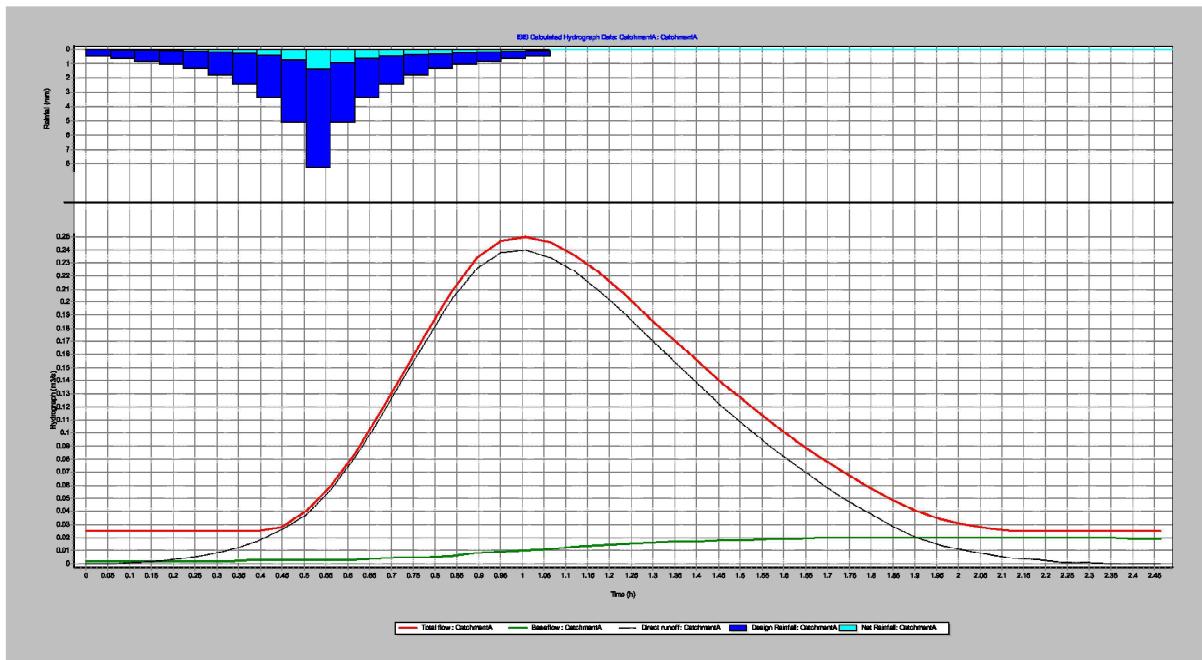


Figure D.7 Sub-catchment A 1 in 100 year (1% AEP) flow hydrograph

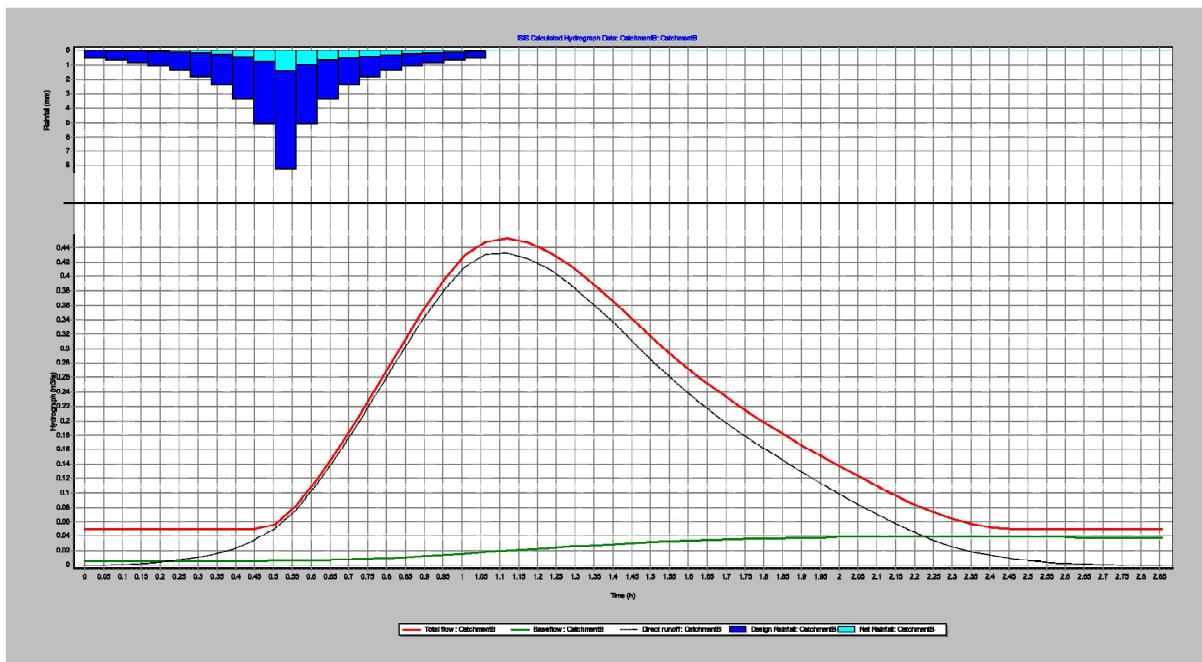


Figure D.8 Sub-catchment B 1 in 100 year (1% AEP) flow hydrograph

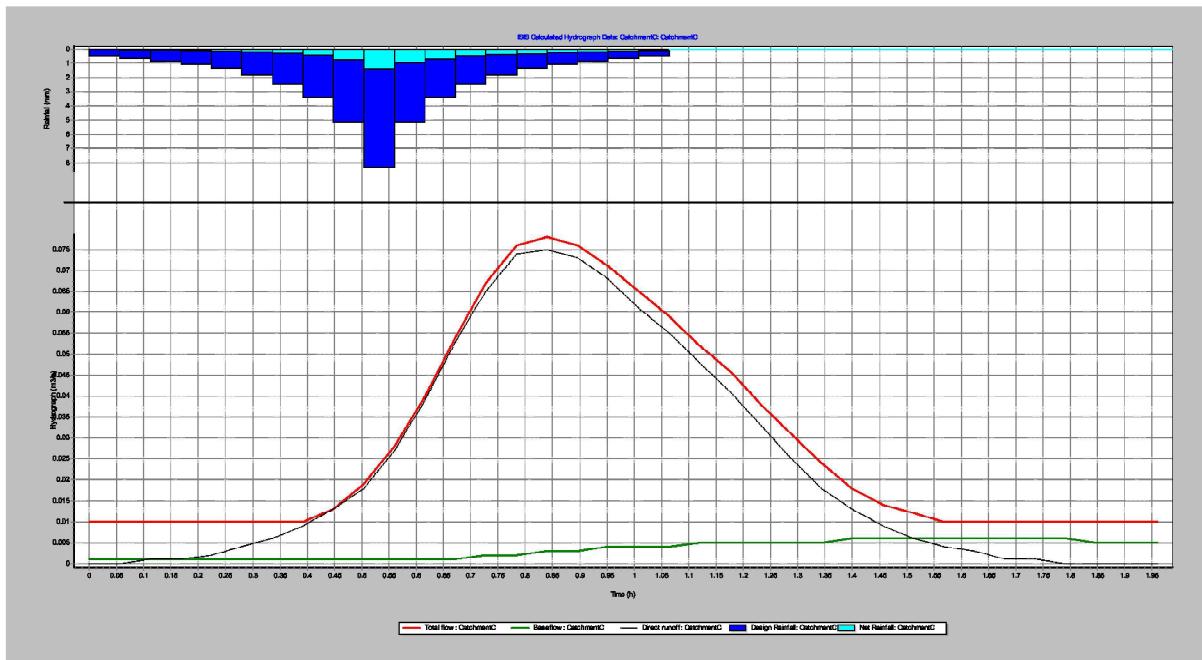


Figure D.9 Sub-catchment C 1 in 100 year (1% AEP) flow hydrograph

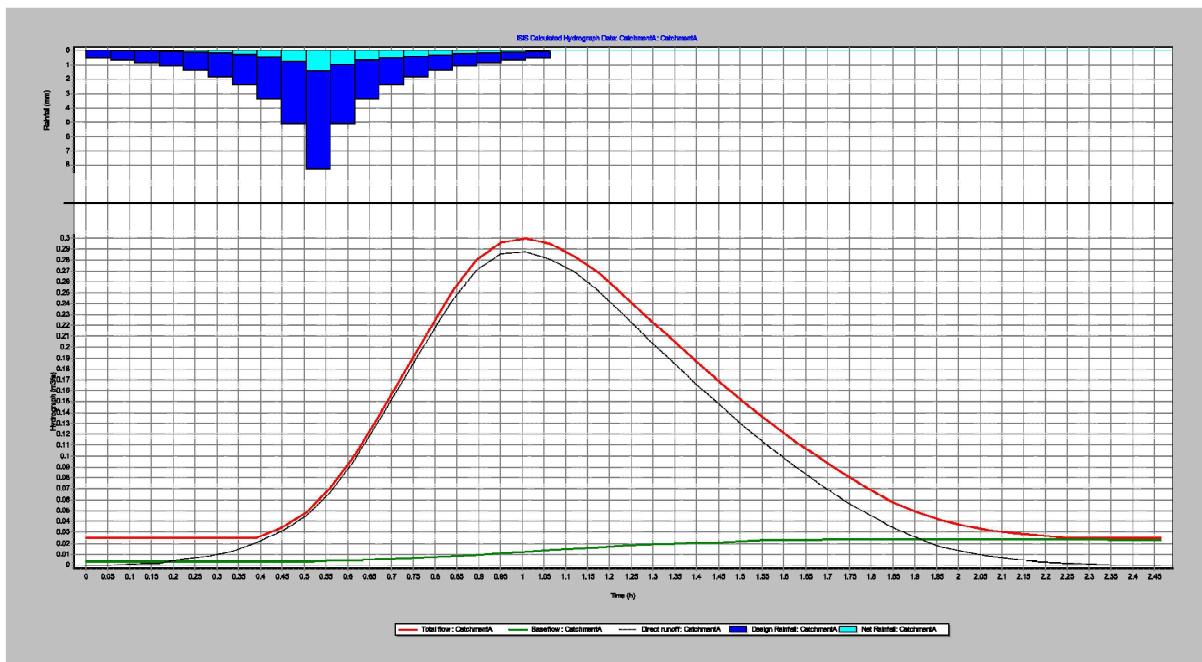


Figure D.9 Sub-catchment A 1 in 100 year (1% AEP) plus climate change flow hydrograph

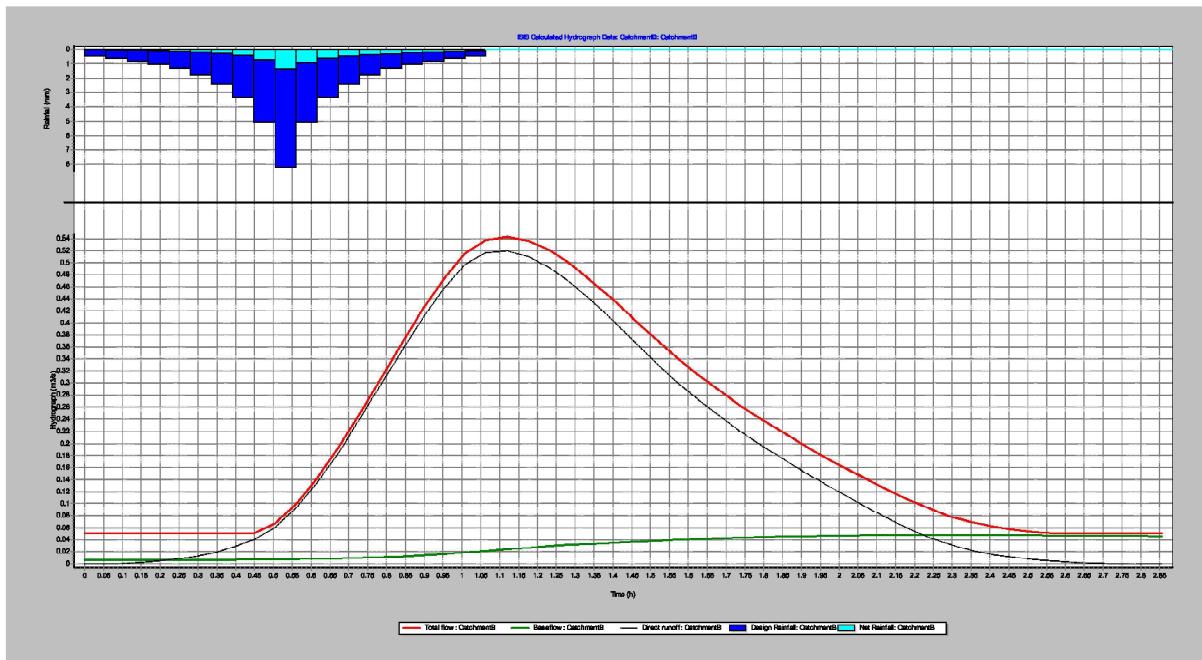


Figure D.9 Sub-catchment B 1 in 100 year (1% AEP) plus climate change flow hydrograph

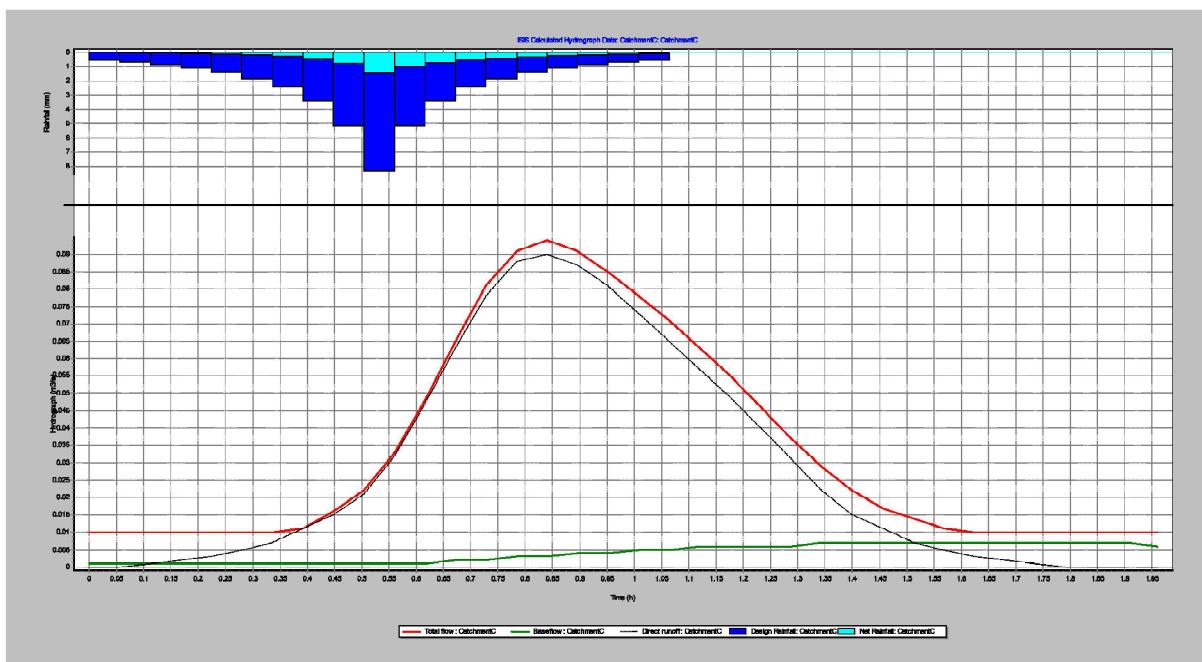


Figure D.10 Sub-catchment C 1 in 100 year (1% AEP) plus climate change flow hydrograph

APPENDIX E: ISIS OUTPUTS: EXISTING SCENARIO SCHEMATIC, LONG-SECTION AND CROSS-SECTIONS

This page has been left intentionally blank

This page has been left intentionally blank

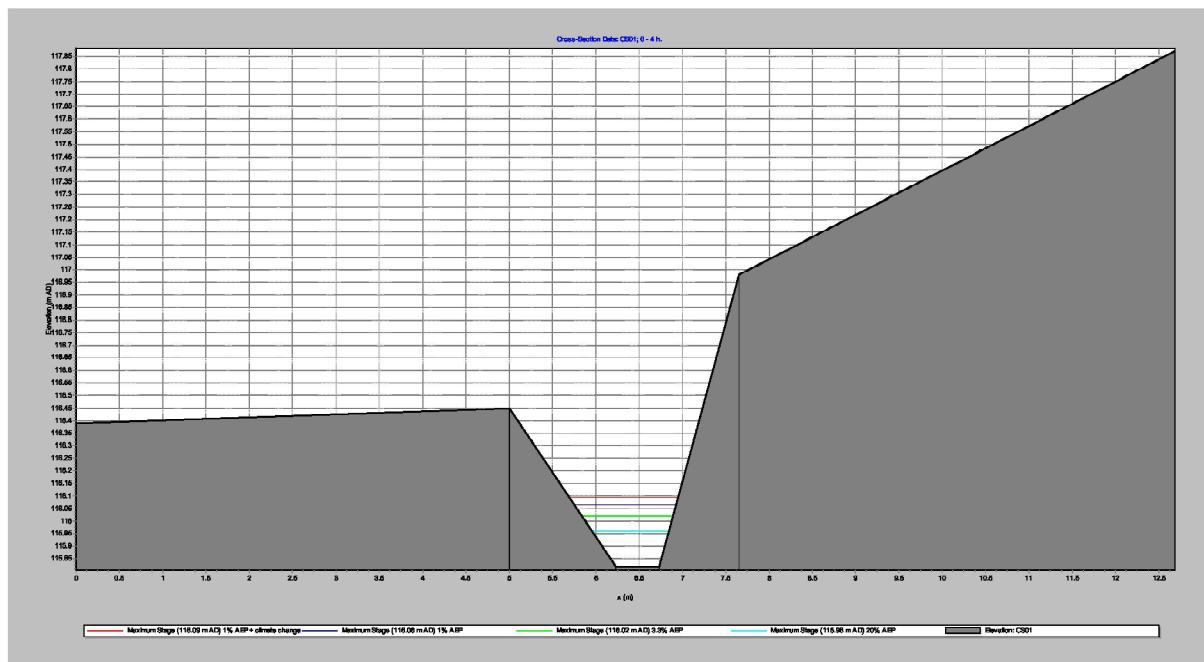


Figure E.1 Peak levels at cross section CS01

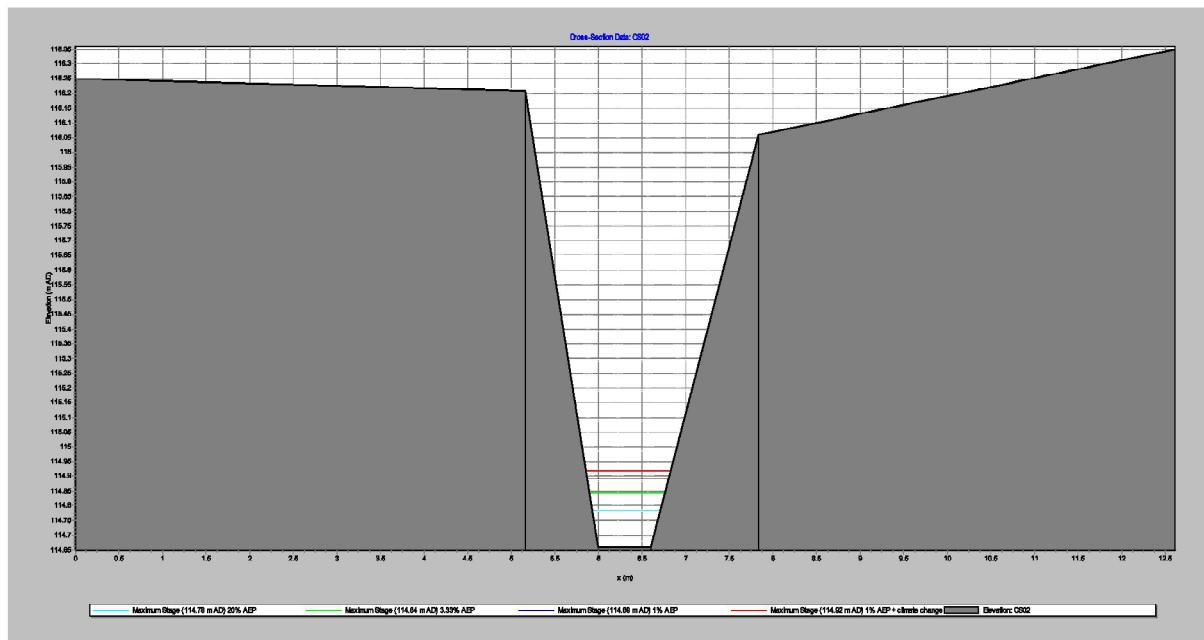


Figure E.2 Peak levels at cross section CS02

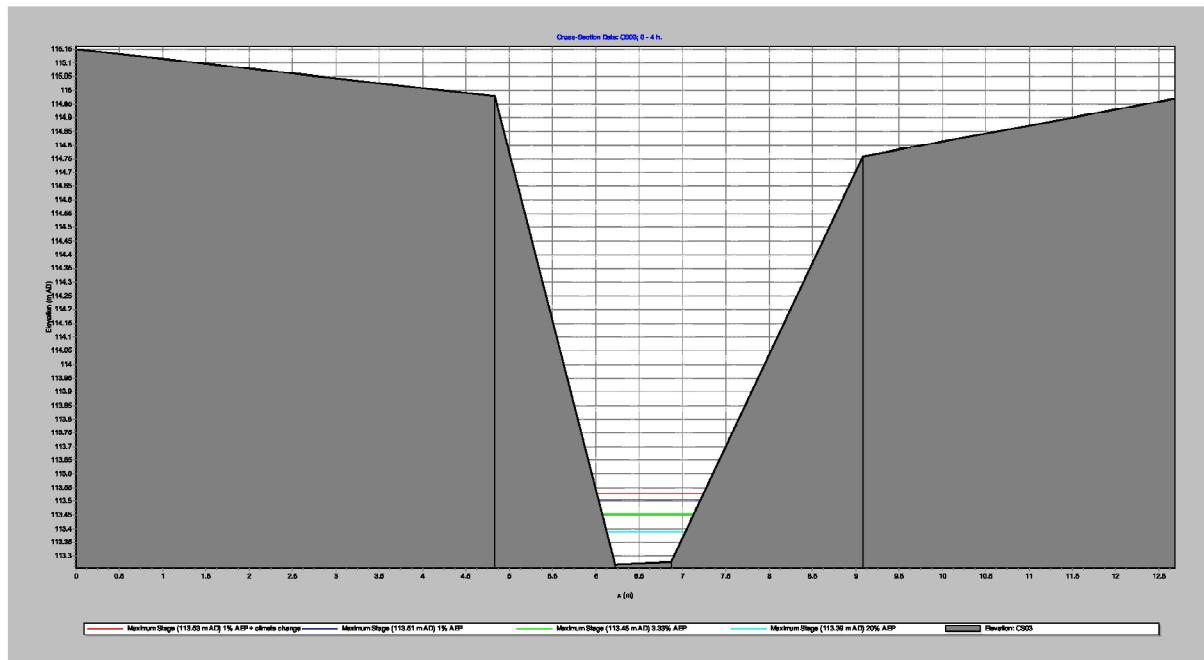


Figure E.3 Peak levels at cross section CS03

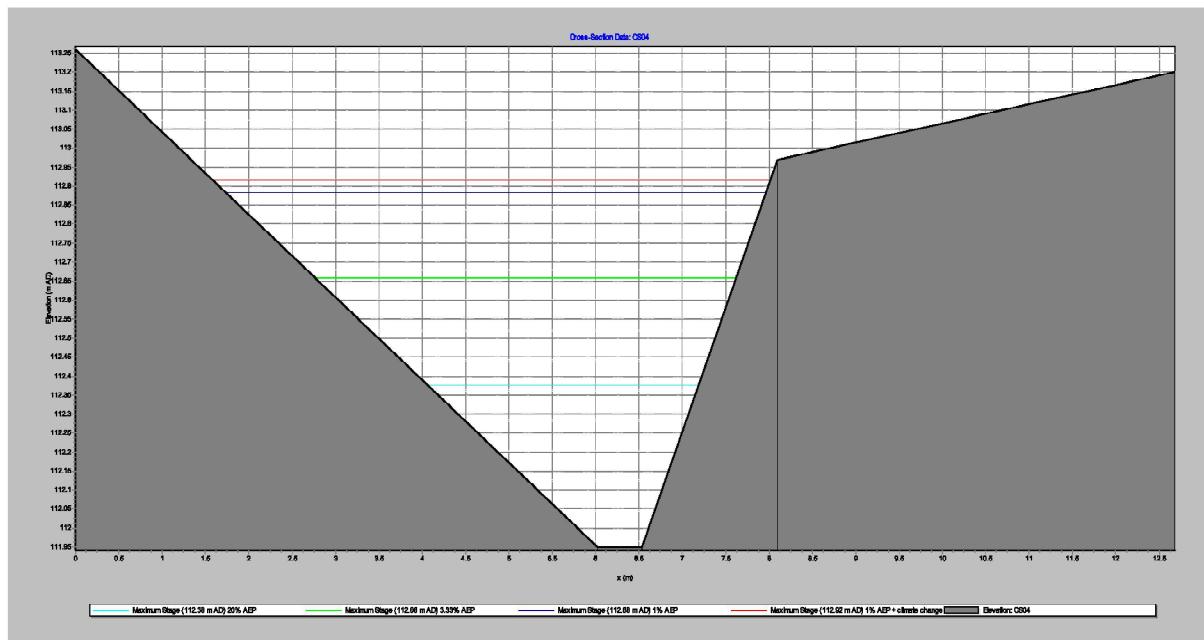


Figure E.4 Peak levels at cross section CS04

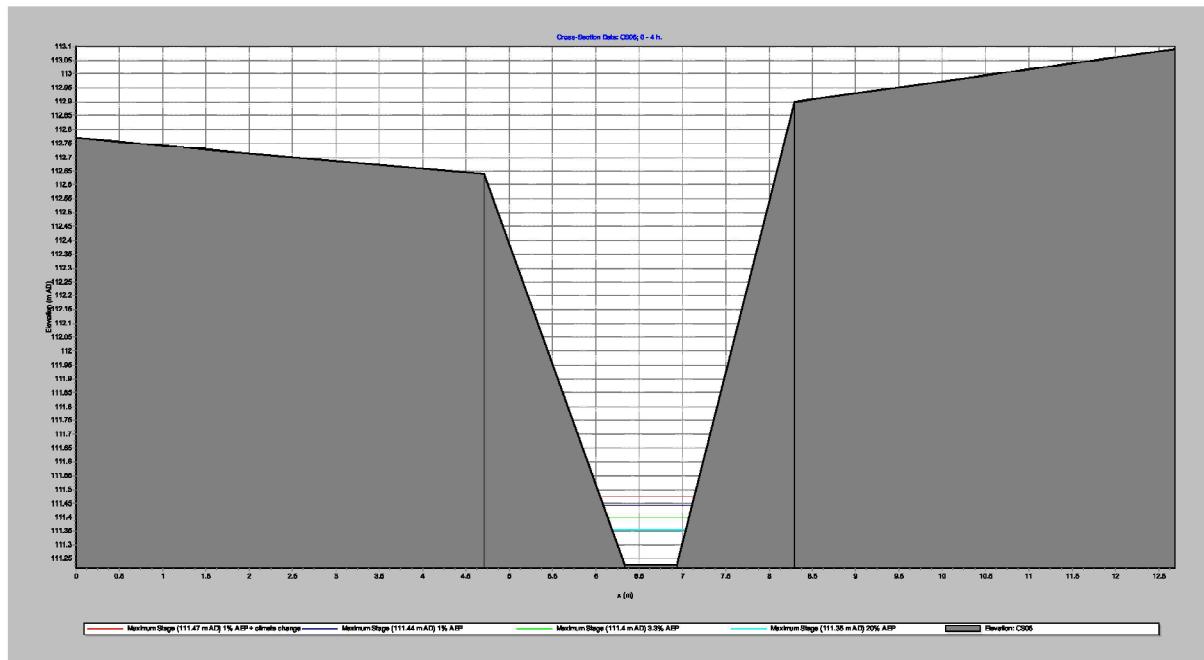


Figure E.5 Peak levels at cross section CS05

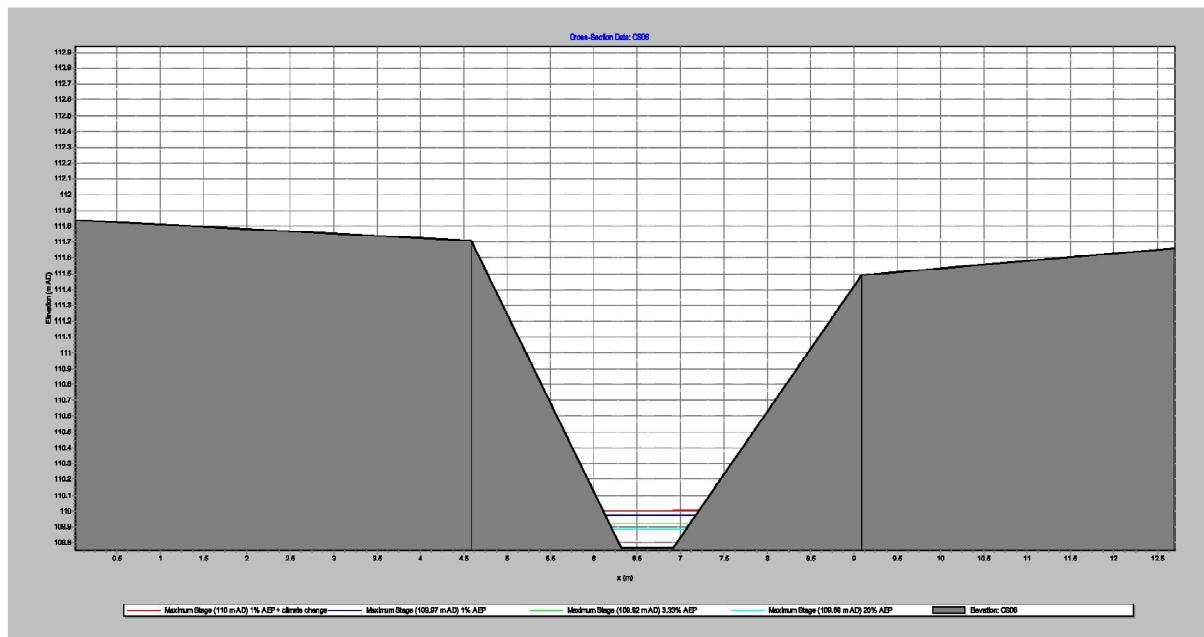


Figure E.6 Peak levels at cross section CS06

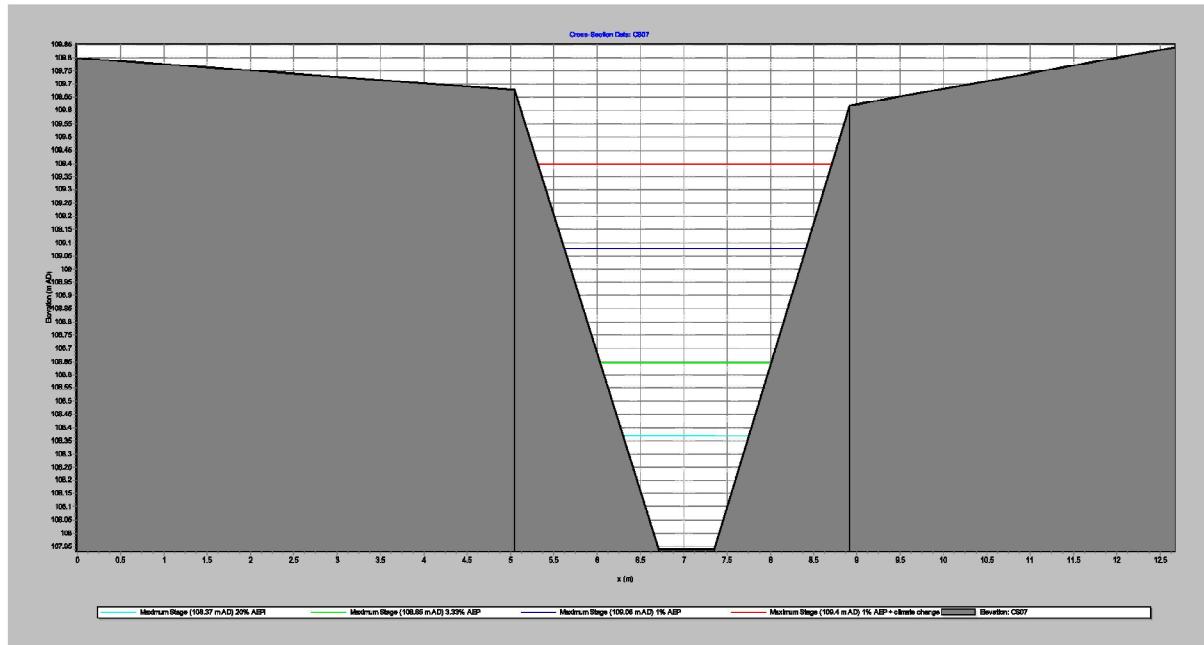


Figure E.7 Peak levels at cross section CS07

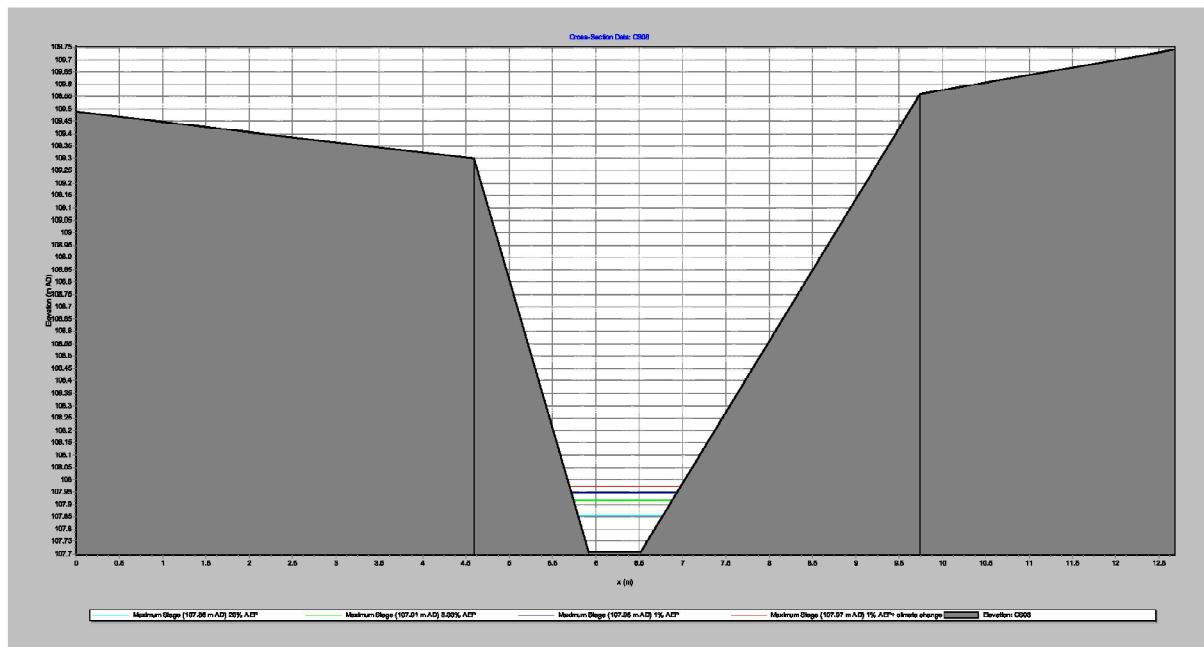


Figure E.8 Peak levels at cross section CS08

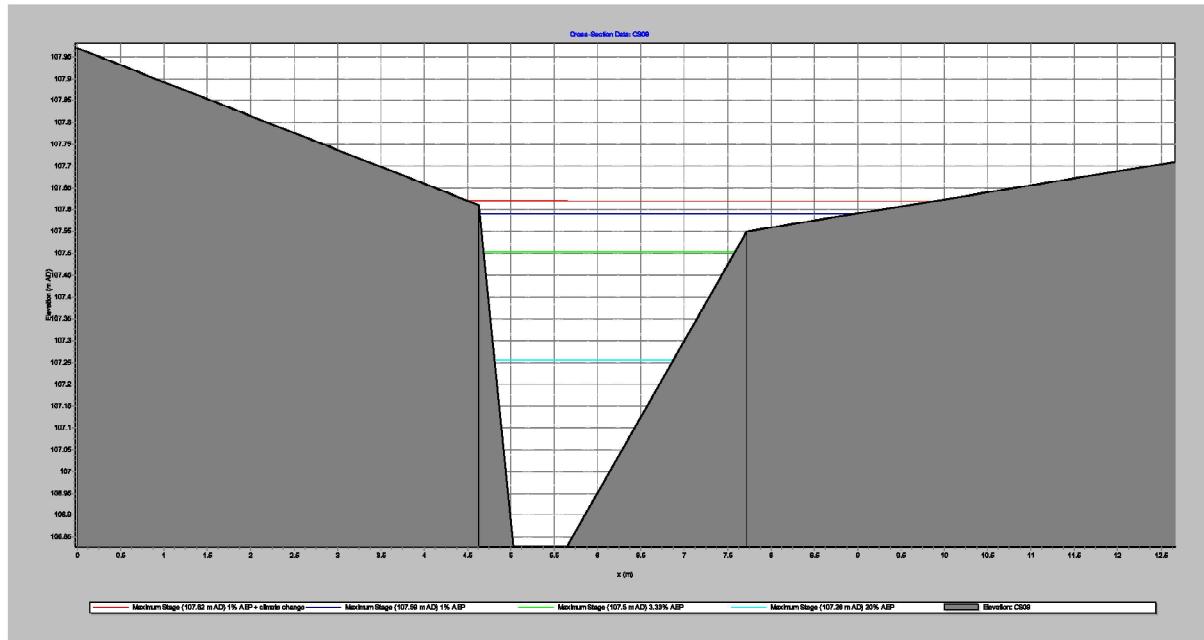


Figure E.9 Peak levels at cross section CS09

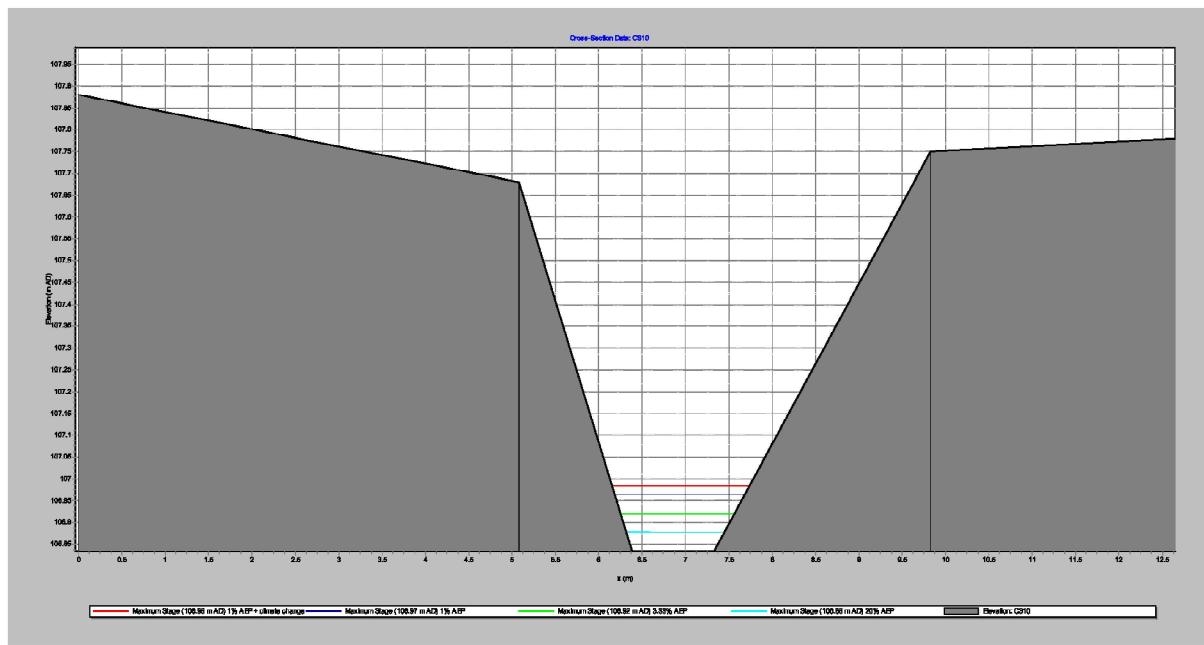


Figure E.10 Peak levels at cross section CS10

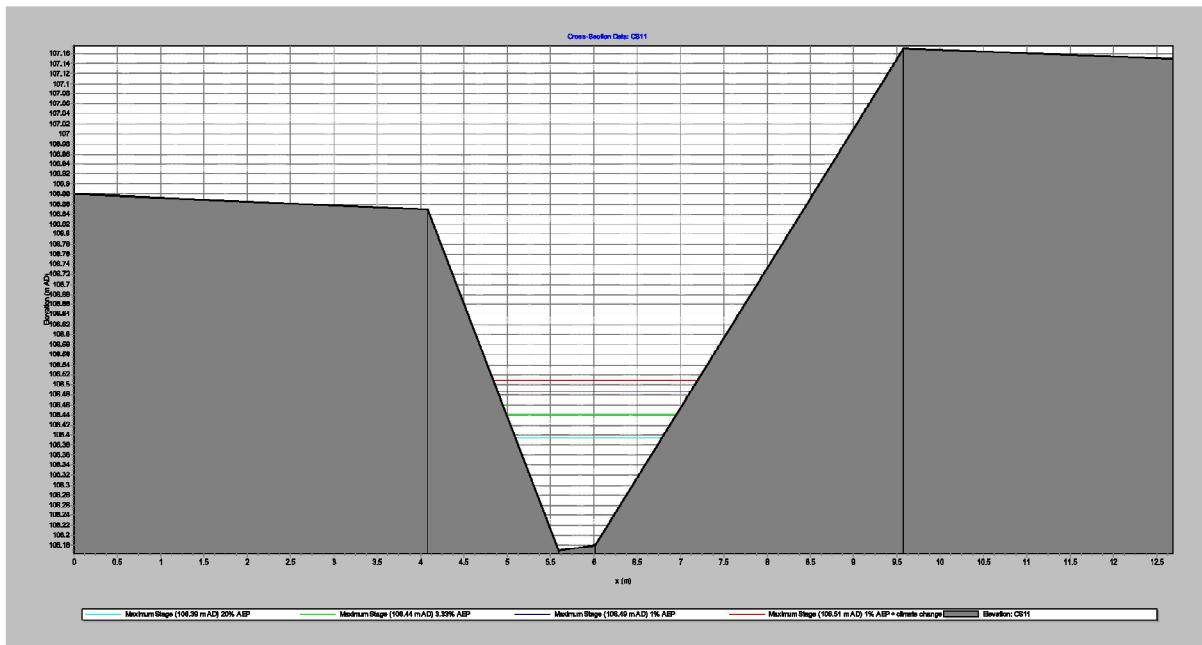


Figure E.11 Peak levels at cross section CS11

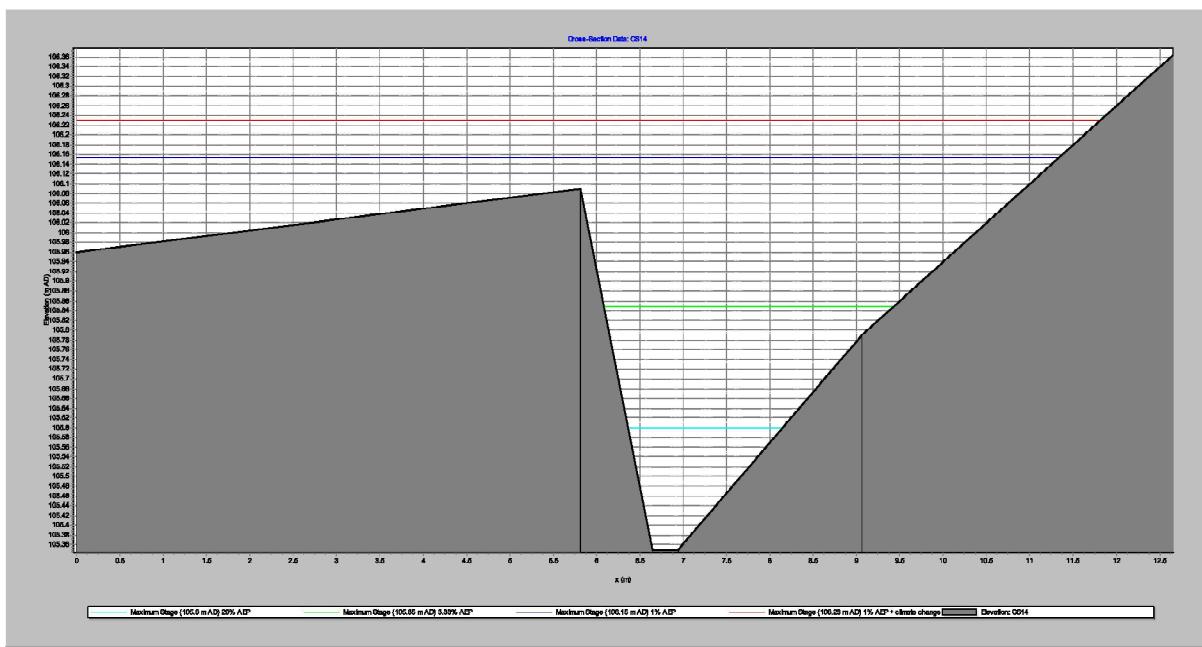


Figure E.12 Peak levels at cross section CS14

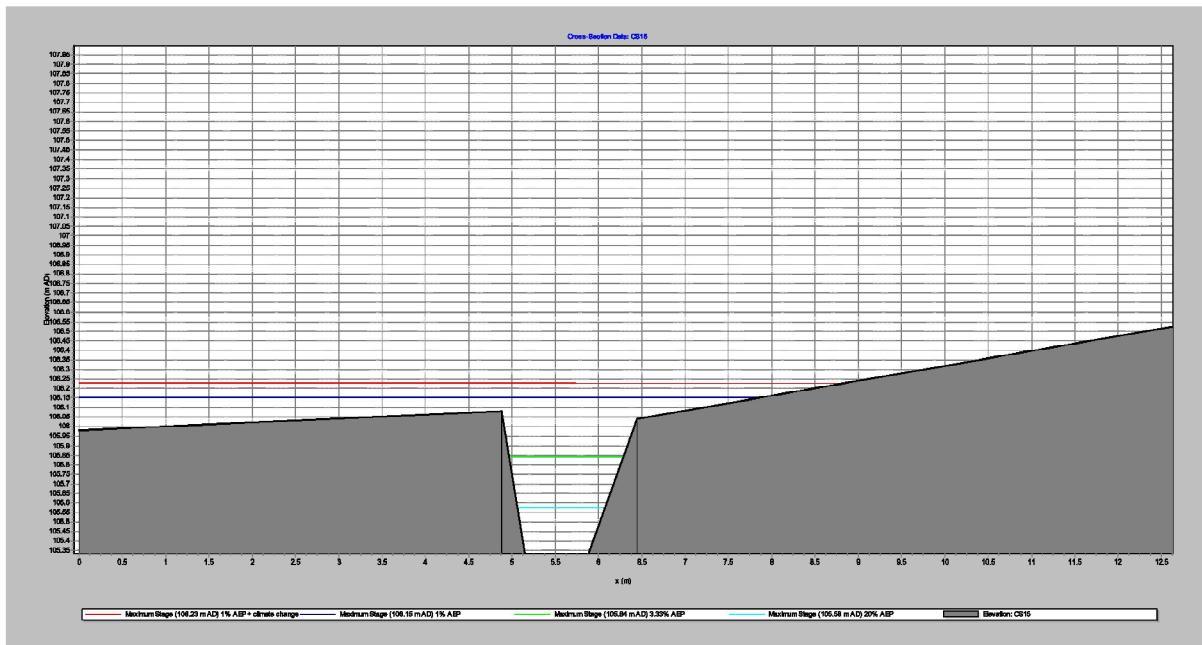


Figure E.13 Peak levels at cross section CS15

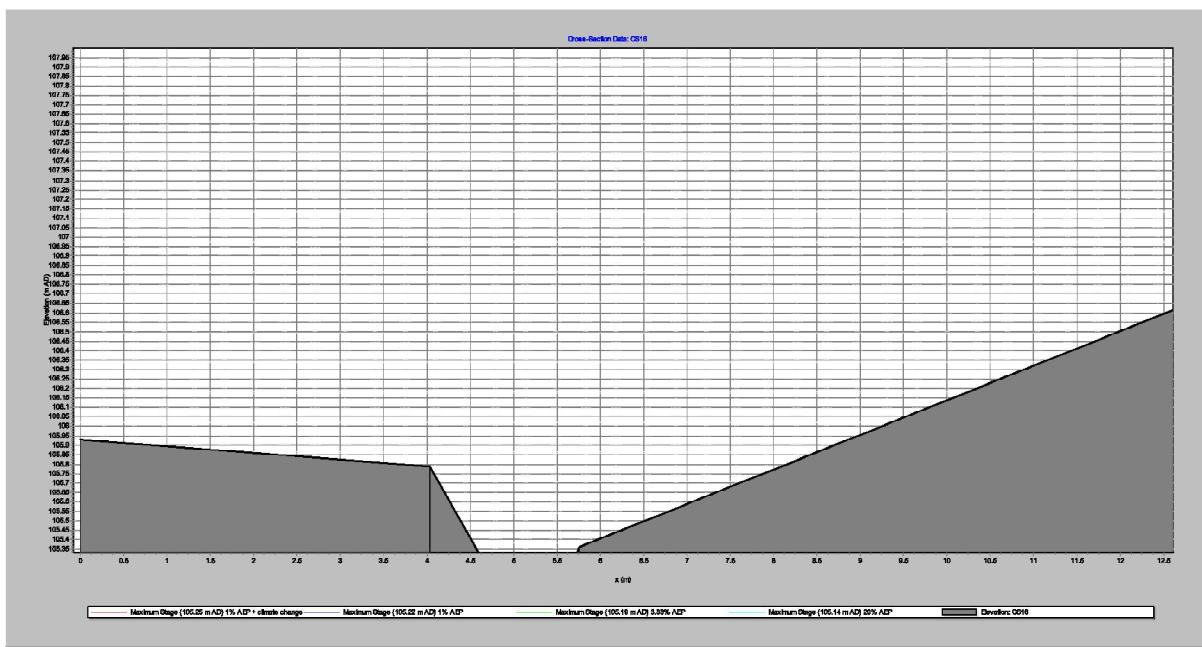


Figure E.14 Peak levels at cross section CS16

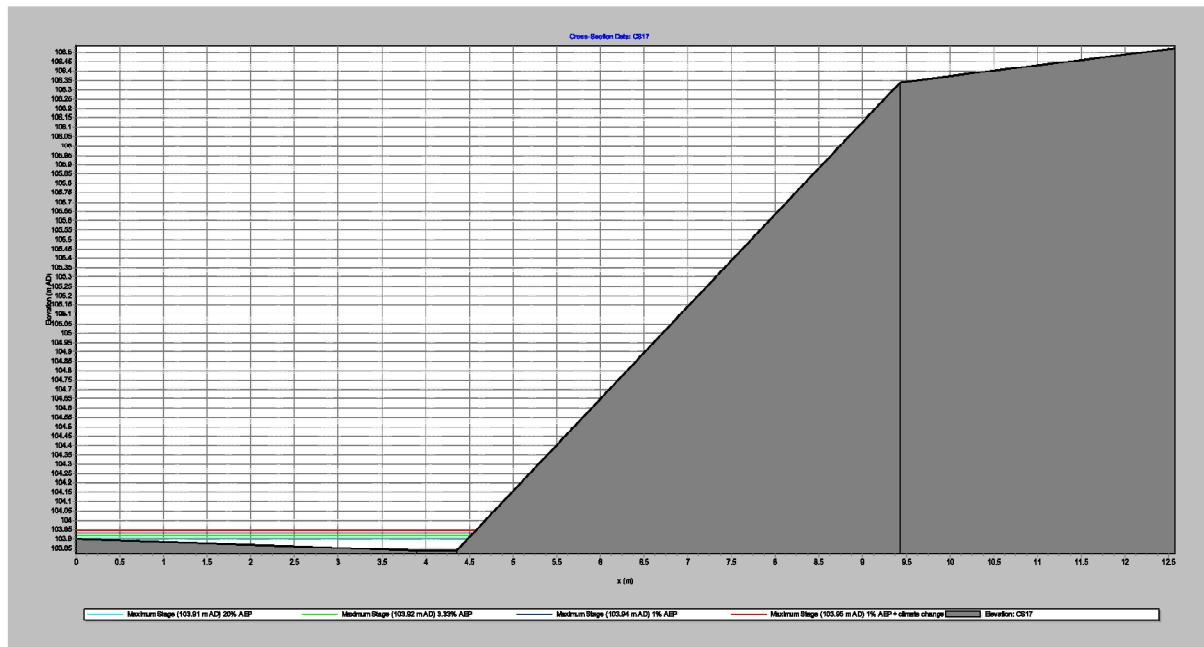


Figure E.15 Peak levels at cross section CS17

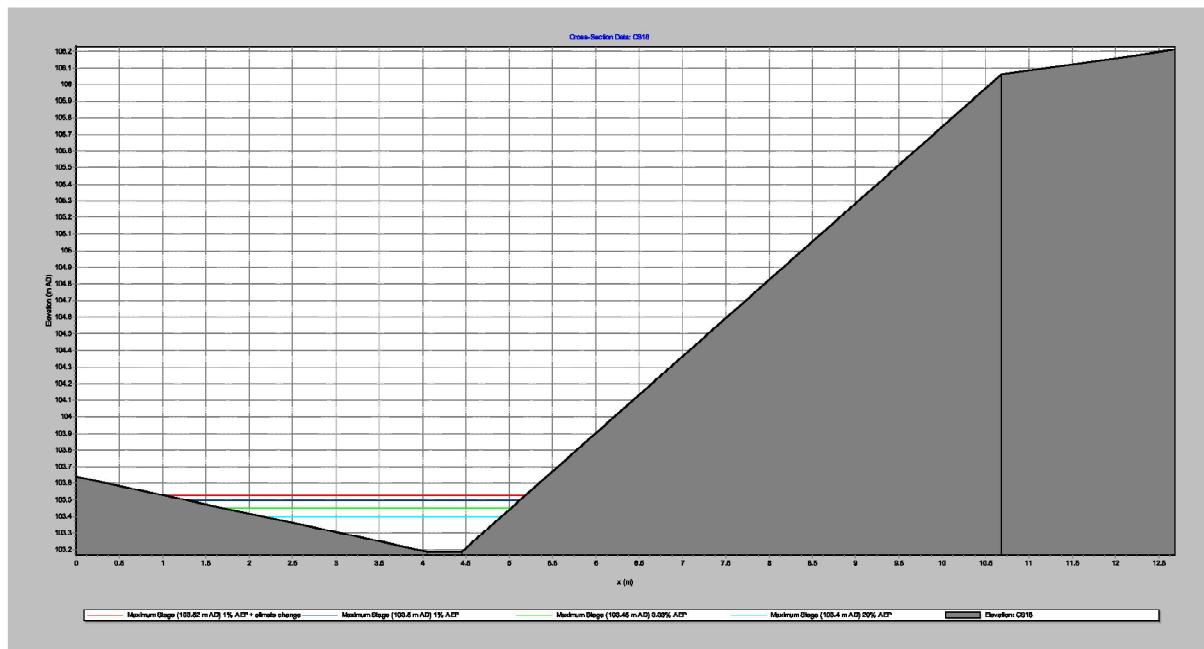


Figure E.16 Peak levels at cross section CS18

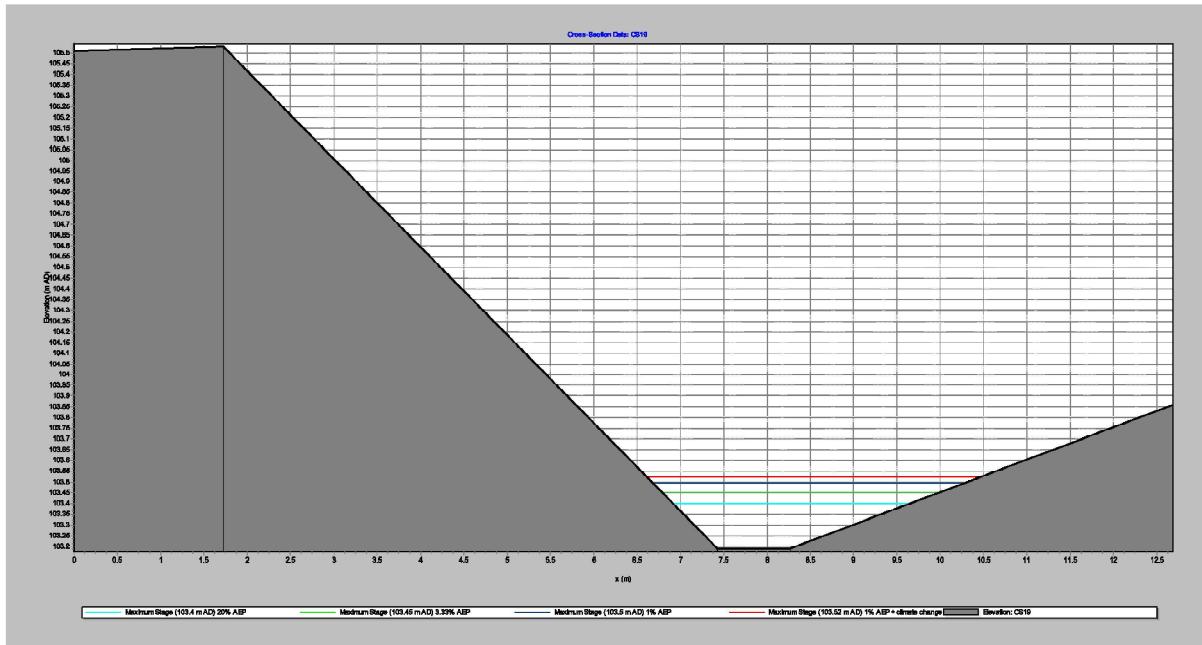


Figure E.17 Peak levels at cross section CS19

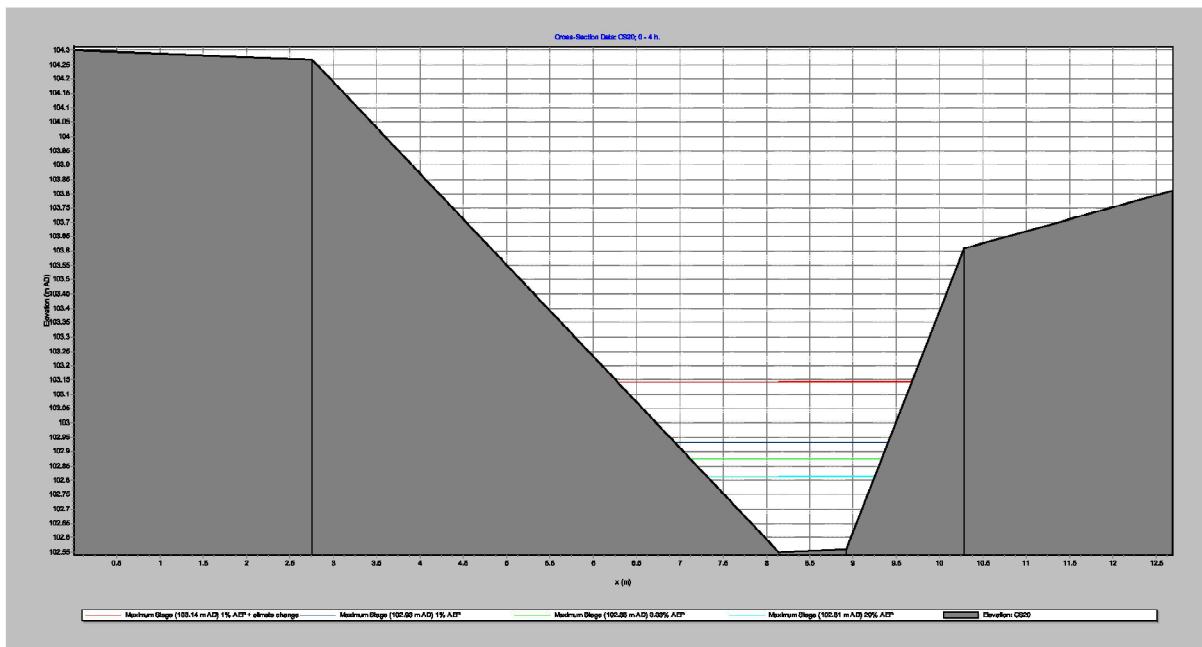


Figure E.18 Peak levels at cross section CS20

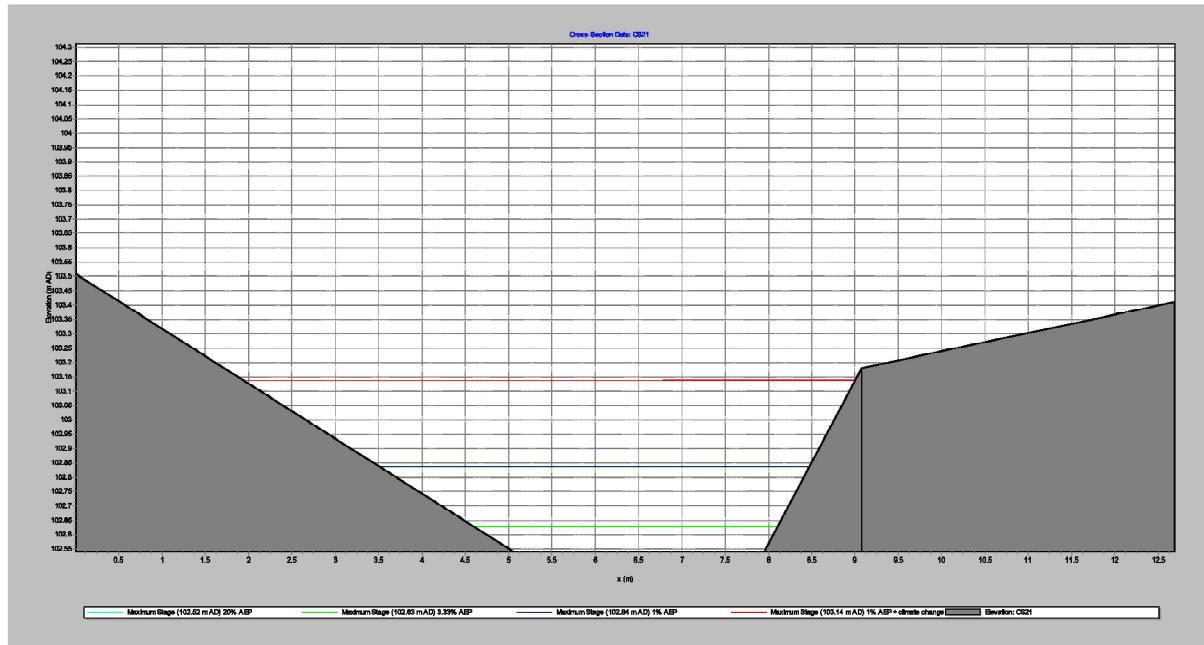


Figure E.19 Peak levels at cross section CS21

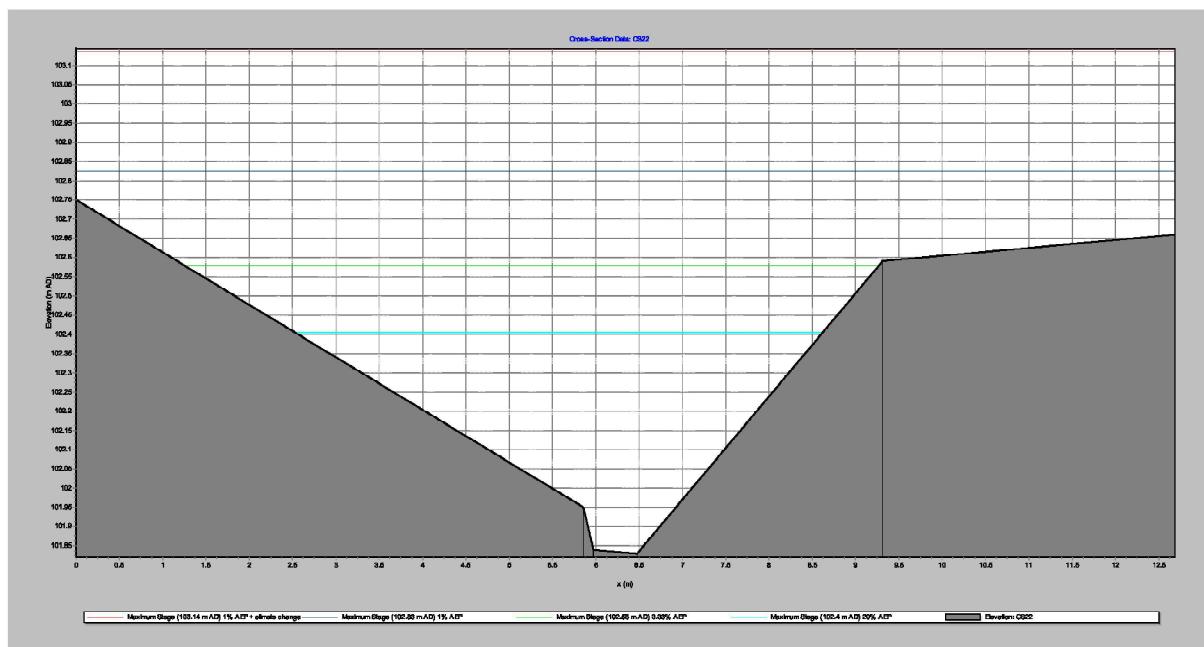


Figure E.20 Peak levels at cross section CS22

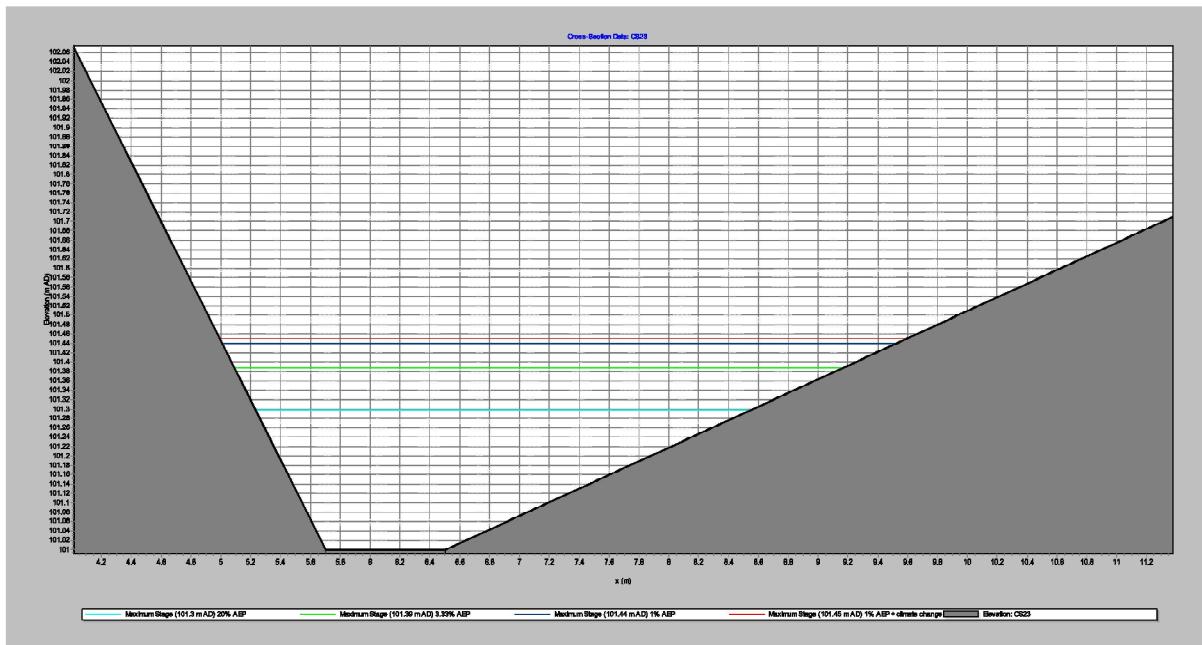


Figure E.21 Peak levels at cross section CS23

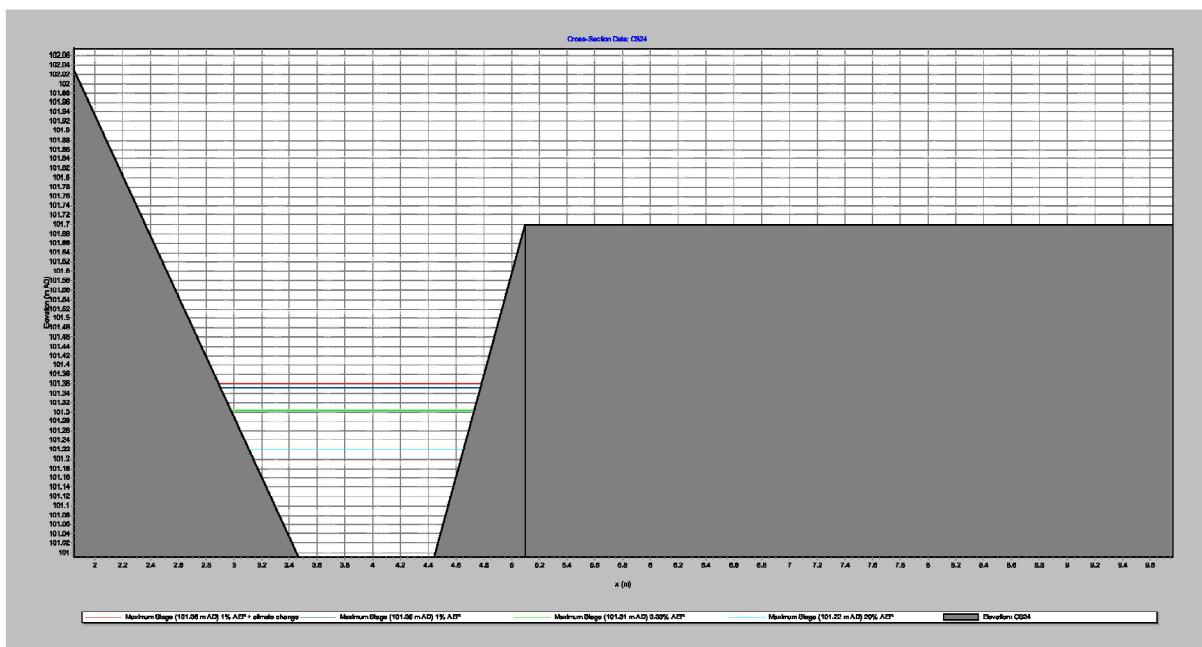


Figure E.22 Peak levels at cross section CS24

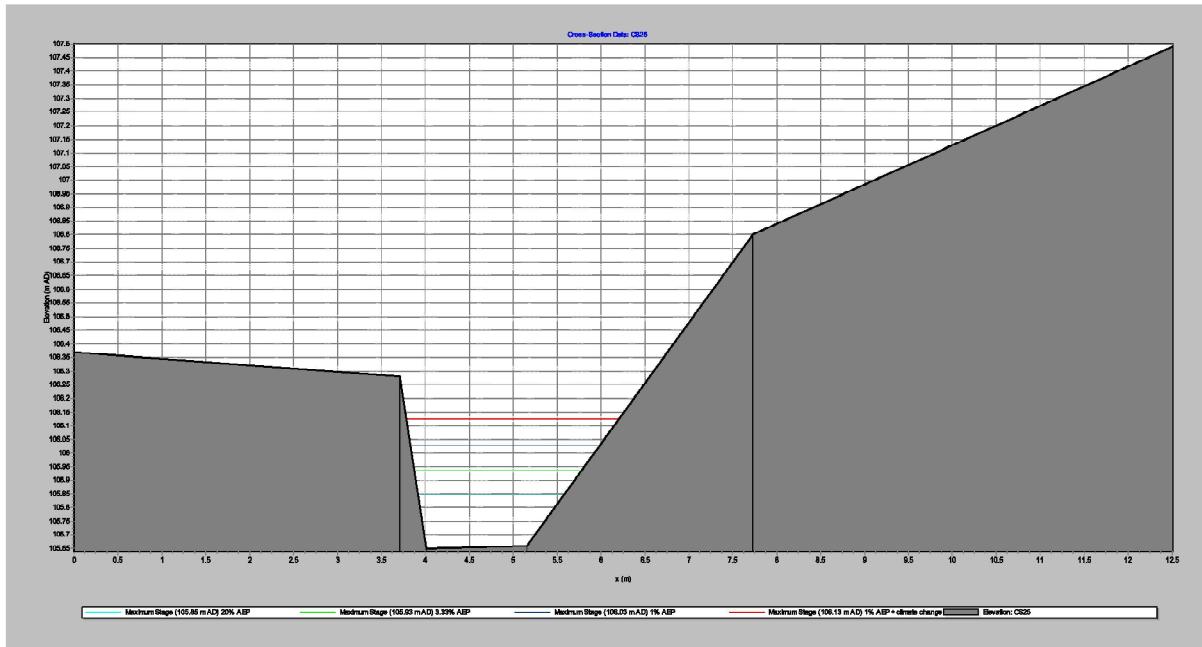


Figure E.23 Peak levels at cross section CS25

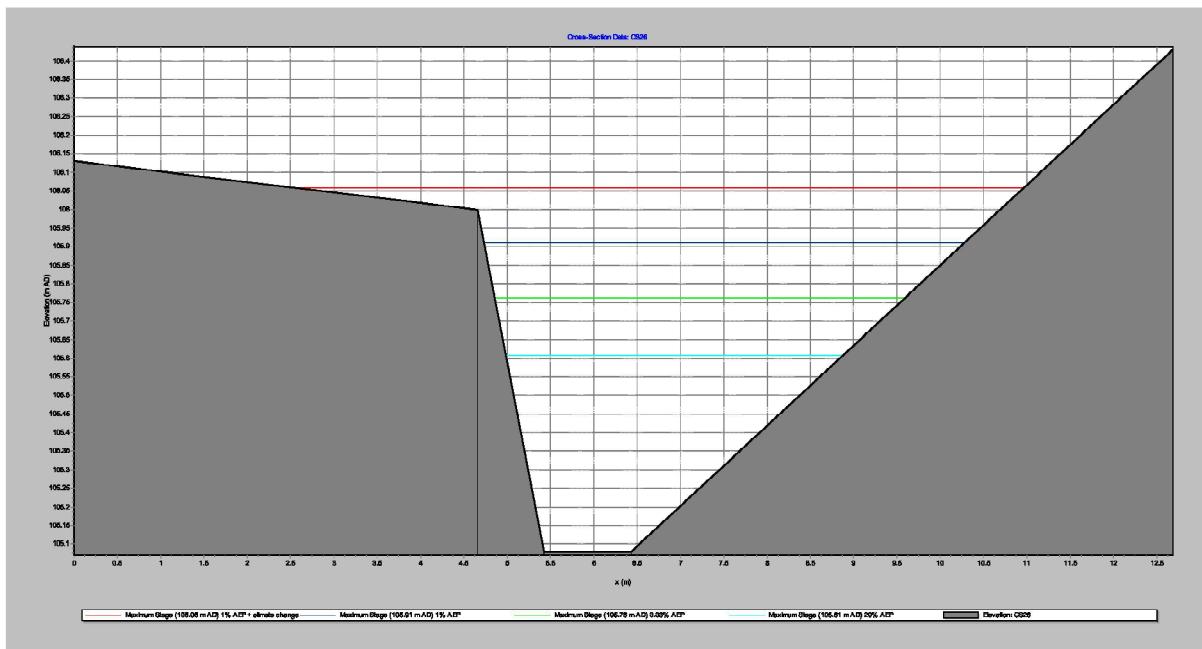


Figure E.24 Peak levels at cross section CS26

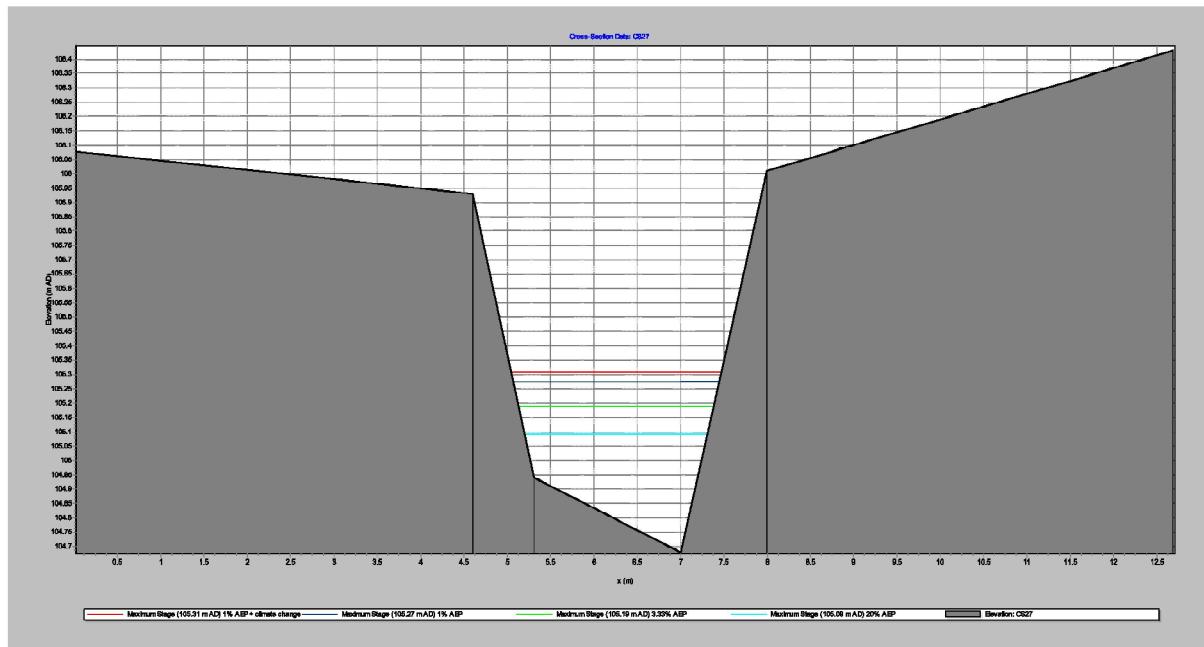


Figure E.25 Peak levels at cross section CS27

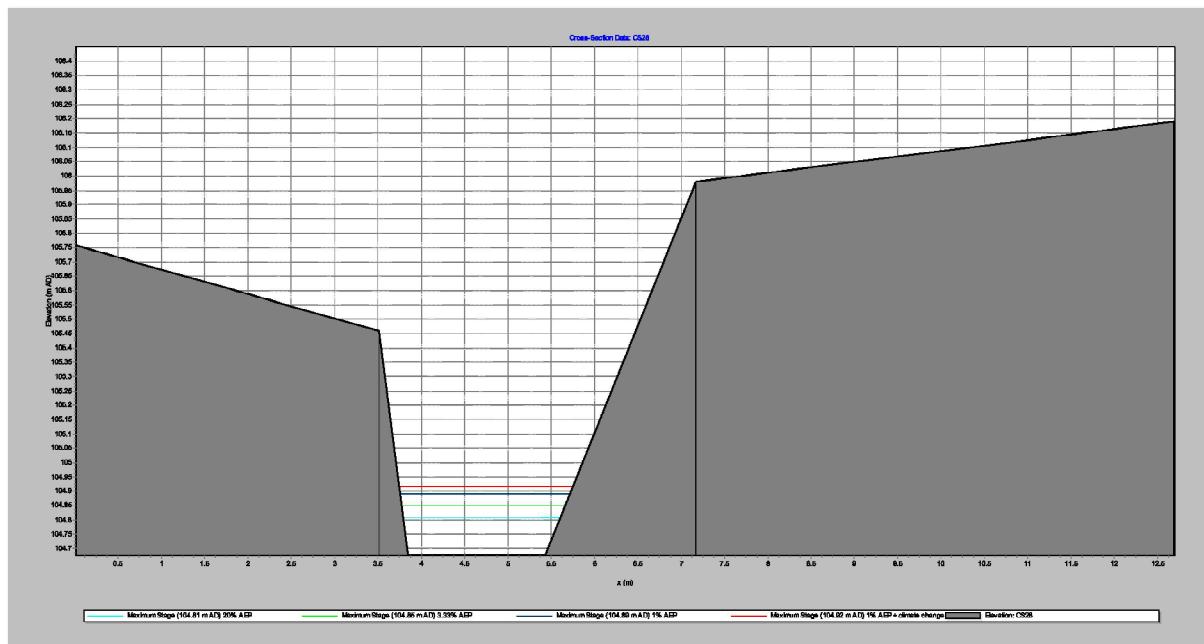


Figure E.26 Peak levels at cross section CS28

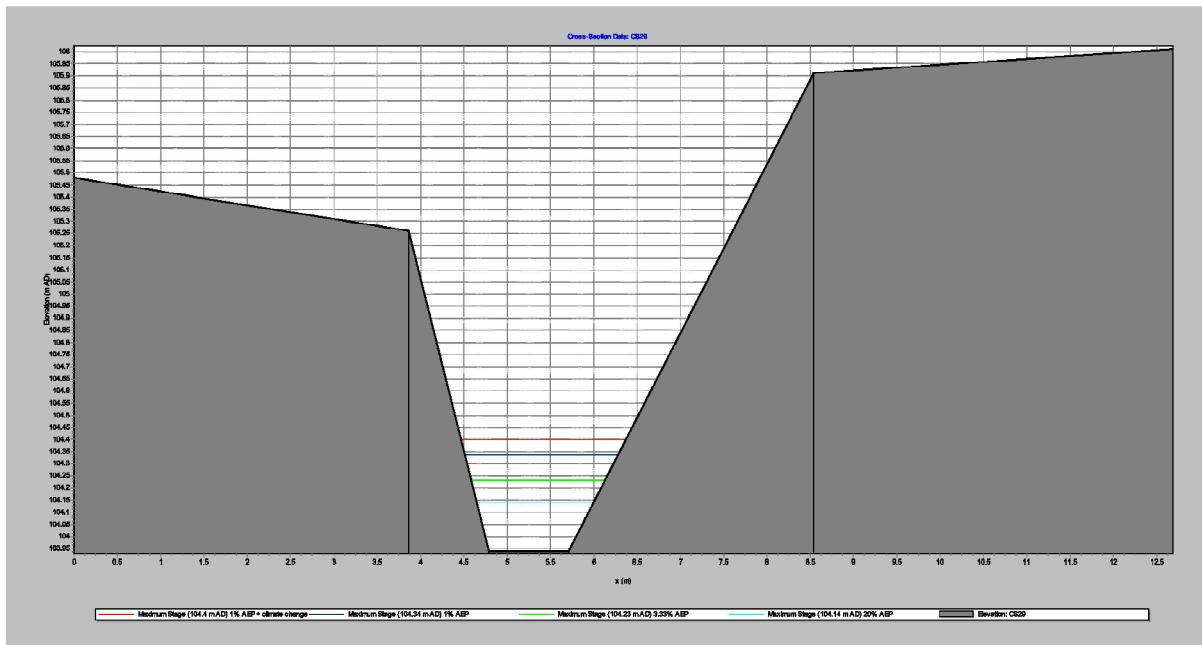


Figure E.27 Peak levels at cross section CS29

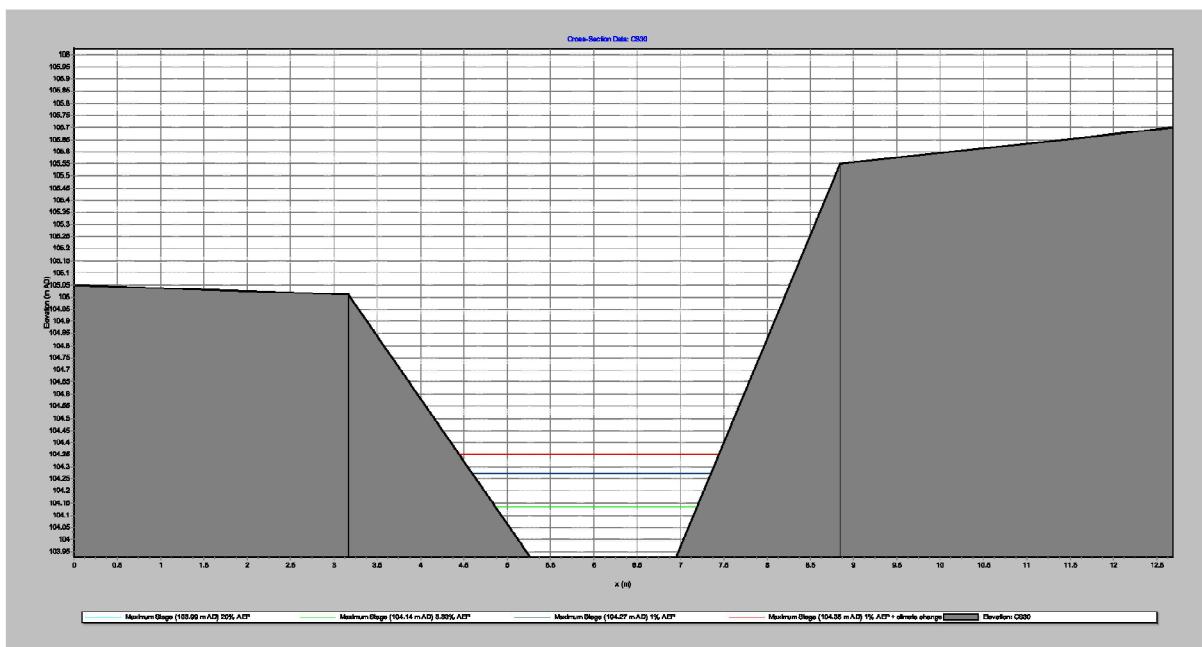


Figure E.28 Peak levels at cross section CS30