Eric Wright Construction Ltd.

Clitheroe Care Home Drainage Strategy

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Clitheroe Care Home Drainage Strategy



Document History

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Revision	Purpose Description	Originated	Authorised	Date



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1 Introduction

1.1 **Commission**

JP Structural Design were appointed by Eric Wright Construction Ltd. to produce a Drainage Strategy for a new build care home with access and car parking area at Clitheroe, Lancashire.

1.2 Limitations

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The findings of this Strategy have been based on data available at the time of the study and on the review of available information that has been undertaken to date. They relate to the current proposed layout as outlined in **Appendix A**. Should the proposed end use of the site change after the completion of this assessment, then the findings of this report will need to be reviewed and updated accordingly.



2 Existing Site and Proposed Development

2.1 Existing Site

The site is located on land off Dyke Nook, Clitheroe, Lancashire. The centre of the site is at National Grid Reference SD748407. The proposed site covers an area of approximately 0.990 ha. See **Appendix B** for the Site Information Drawing.

The site is roughly triangular in shape and it is located to the south east of Clitheroe. The site is part of a wider proposed residential development within the area which will surround the site once completed. The site is currently agricultural land, there is a hedgerow in the north and a tree present in the centre of the site, there are no structures present.

A review of the topographical survey shows that the site falls in a northern direction. The survey indicates that the site has a high point of 103.850m (Above Ordnance Datum) AOD located in the south west of the site. The low point is located in the north west of the site at 101.300m AOD, with levels of 102.600m AOD and 103.280m AOD in the north east and south east corners respectively. Additionally, there is a 1.3-1.5m high earthworks mound located 15-20m from the west of the site boundary. The topographical survey of the site is provided in **Appendix C**.

2.2 Ground Conditions

The surface geology of the site has been reviewed from the British Geological Survey (BGS) online geology maps. The geology map indicates that the site is superficially underlain by "Till, Devensian - Diamicton. Sedimentary superficial deposit formed between 116 and 11.8 thousand years ago during the Quaternary period." The bedrock geology is described as "Clitheroe Limestone Formation and Hodder Mudstone Formation - Mudstone. Sedimentary bedrock formed between 346.7 and 337 million years ago during the Carboniferous period."

According to the Soilscapes soils dataset (<u>http://www.landis.org.uk/soilscapes/</u>), soil conditions at the site and within the surrounding area are slowly permeable seasonally wet acid loamy and clayey soils with impeded drainage.

E3P carried out soakaway testing on 19 December 2022 in accordance with BRE365 to determine infiltration rates of the site. A summary of the findings are provided in **Appendix D**. The testing was undertaken in three locations on site. The E3P report states the following:

"Within all three locations, soakaway testing failed, with the water level failing to soakaway below 75% effective storage. As such, soakaway drainage is not unlikely to be suitable on the site."

No groundwater was encountered within any of the exploratory holes.



2.3 Existing Waterbodies

There is an unnamed watercourse to the west of the site that discharges to Pendleton Brook.

There are no other water bodies within the immediate vicinity of the site.

2.4 Existing Drainage

Public sewer records obtained from United Utilities (UU) (refer to **Appendix E**) indicate that there are public sewers serving the existing residential areas to the north and west of the site but that there are no public sewers within the immediate vicinity of the site.

The site is undeveloped greenfield, given site topography and ground conditions, surface water runoff would be expected to flow overland in a north west direction.

The Coopers Chartered Consulting Engineers drawings 6263 / sp-a-03-1, 2, 3 and 4 for the Spine Road of Phase A of the wider development have been reviewed as part of this assessment (refer to **Appendix F**). The drawings indicate the following:

- A system comprising 300mm, 375mm and 450mm diameter surface water sewers situated within the proposed spine road as shown on the existing drainage layout in **Appendix G**. The system appears to discharge to the unnamed watercourse to the west of the site that ultimately discharges to Pendleton Brook.
- A system comprising 150mm, 225mm and 300mm diameter foul water sewers situated within the proposed spine road as shown on the existing drainage layout in **Appendix G**. The system appears to discharge to the 225mm / 300mm diameter public combined sewer situated within Littlemoor Road to the west of the site.

2.5 Existing Flood Risk

The Environment Agency Flood Map for Planning (Rivers and Sea) indicates the site is located in flood zone I. Flood zones refer to the probability of river and sea flooding. Table I of the NPPG defines flood zones as follows:

- Flood zone I: Low Probability. Land having a less than I in 1,000 annual probability of river or sea flooding.
- Flood zone 2: Medium Probability. Land having between a 1 in 100 and 1 in 1,000 annual probability of river flooding or between a 1 in 200 and 1 in 1,000 annual probability of sea flooding.
- Flood zone 3a: High Probability. Land having a 1 in 100 or greater annual probability of river flooding or a 1 in 200 or greater annual probability of sea flooding.
- Flood zone 3b: Functional Floodplain. Land where water has to flow or be stored in times of flood.

The management of surface water run-off generated by the post development site will be the principal flood risk associated with this scheme, therefore reduction of any flood risk is discussed in Section 3 of this report.



2.6 **Proposed Development**

The scheme consists of the construction of a new 2 storey 68 bed care home over in Clitheroe. The development will include a new building, landscaped areas and vehicular parking area. See **Appendix A** for proposed site layout.



3 **Drainage Proposals**

3.1 Foul Drainage

The proposed peak foul discharge generated by the development has been calculated based upon an occupancy rate of 1.5 persons per 1 bedroom, (allowing for residents and support staff) which equates to 102 persons. Therefore, allowing a typical usage of 350 l/person/day over a 24-hour day with a peak factor of 6, generates a peak foul flow of 2.5 l/s.

As the existing site is greenfield the peak foul flows from the proposed development will be greater than the existing.

It is proposed to discharge foul flows from the site to the existing manhole F10 via the existing 150mm diameter spur connection currently present in the south of the site as indicated on the preliminary drainage layout presented in **Appendix H**.



3.2 Surface Water Drainage

It is acknowledged that the satisfactory collection, control and discharge of storm water is now a principle planning and design consideration. Part H of the Building Regulations 2002 recommends that surface water run-off shall discharge to one of the following, listed in order of priority:

- a) an adequate soakaway or some other adequate infiltration system, or where that is not reasonably practicable,
- b) a watercourse, or, where that is not reasonably practicable,
- c) a sewer.

It is necessary to identify the most appropriate method of controlling and discharging surface water. The design should seek to improve the local run-off profile by using systems that can either attenuate run-off and reduce peak flow rates or positively impact on the existing flood profile.

3.2.1 Ground Infiltration Techniques

As detailed in Section 2, infiltration testing has been undertaken by E3P in accordance with the guidelines in BRE365. However, within all three locations, soakaway testing failed. As such the disposal of surface water via infiltration has been discounted for the site.

3.2.2 Discharge To Watercourse

It is therefore proposed to discharge surface water runoff from the site to the existing manhole SII via the existing 150mm diameter spur connection currently present in the south of the site. This is indicated on the preliminary drainage layout presented in **Appendix H.** The existing system discharges to the unnamed watercourse to the west of the site that ultimately discharges to Pendleton Brook.



3.3 **Surface Water Calculations**

3.3.1 **Proposed Discharge Rate**

The Coopers Chartered Consulting Engineers drawings 6263 / sp-a-03-1, 2, 3 and 4 for the Spine Road of Phase A of the wider development have been reviewed as part of this assessment (refer to **Appendix F**). The plans indicate the surface water sewers situated within the proposed spine road have been designed to accommodate a proposed discharge rate of 11 l/s from this development. Accordingly, it is proposed to restrict runoff from impermeable surfaces of the development to 11 l/s.

3.3.2 Attenuation Storage

Attenuation storage will be provided to restrict surface water runoff generated across roofs and hardstanding.

The attenuation storage facility has been modelled using Causeway Flow (refer to **Appendix I**). The required storage volume has been sized to store the 1:100 rainfall event including a 40% increase in rainfall intensity to allow for climate change in accordance with Environment Agency guidance.

Based on a peak discharge rate of 11 l/s, a total storage volume of 259.9 m³ would be required. It is proposed that this storage is provided in a geo-cellular attenuation tank with a plan area of 171 m² and a depth of 1.6 m.

The proposed attenuation tank is to be located within the car park area to the south of the proposed building.

Additional storage is provided in the proposed network of pipes and manholes. A preliminary surface water drainage layout is provided in **Appendix H**.



4 **Planning Requirements**

4.1 Water Quality

The proposed sump gullies and proposed vortex flow control catch pit manhole are deemed to be sufficient to remove the suspended sediments from the roof and car park surface water for a development of this size and nature.

4.2 Exceedance Events

Flooding is not expected to occur in the 1:100 year rainfall event plus 40% climate change for the site. It is anticipated that flood flows from rainfall events above this will be directed towards the access road and car parking areas respectively, with the site being profiled to ensure that flood flows are directed away from built development.

4.3 **O&M** Manuals

During the detail design stage & construction, full details of the final design will be submitted and included in the O&M manuals to ensure the drainage system is regularly maintained with particular regards to the surface water system. This will include manufacturer's guidelines for maintenance and replacement and full details of the flow control device as well as means to operate the drain down features in a blockage situation.



5 **C**onclusions

The proposed development is not expected to be affected by general objections in respect to draining the site. There will be suitable conditions imposed to ensure that the drainage proposals are designed and constructed in accordance with relevant statutory requirements, including Building Regulations 2010 and the requirements of Lancashire Lead Local Flood Authority.



Appendix A – Proposed Layout



BOUNDARY TREATMENTS KEY: LOW HEIGHT STONE WALL 1800MM HIGH METAL HOOP TOP RAILINGS 1800MM HIGH CLOSE BOARDED TIMBER FENCE EXISTING GREEN MESH FENCE TO SCHOOL BOUNDARY HEDGING NEW 1800MM HIGH MESH FENCING NOTE: PLAN PRODUCED USING ALMAGATED INFORMATION FROM PLANNING APPLICATIONS. 21/0957 22/0116 21/0951 AND SUBJECT TO CHANGE. PLEASE SEE DRAWING BY SUMO SERVICES SU-MO-10800 FOR LEVEL INFORMATION WITHIN RED LINE BOUNDARY. NOTE: For final landscaping proposals refer to TEL Landscape drawings ref MR22-142/101 & MR22-142/102. DRAWING REVISIONS REV DESCRIPTION DATE BY A Drawings updated to suit general comments from client. Amendments include riser positions being indicated, additional doors being added & re-congiguration of the servery and dining 18.01.23 JA TE Service road redcued to single lane at a width of 3.7m. Additional path added at the side of service road for staff access. 10.02.23 Close boarded fencing to the east boundary omitted and replaced with metal mesh fencing. Change made to suit client comments. 20.02.23 Proposed sub-station location indicated on. 07.03.23 JA drawing in green hatch with annotation & dimensions to suit. Footpath location amended 16.03.23 General amendments made including wall vent locations indicatively shown on drawing. PV panel numbers updated as guided by M&E Consultant. Landscaping co-ordinated to suit TEL Landscape drawings. NOTES DO NOT SCALE, USE FIGURED DIMENSIONS ONLY ALL DIMENSIONS TO BE VERIFIED ON SITE PRIOR TO THE COMMENCEMENT OF . ANY WORK OR THE PRODUCTION OF ANY SHOP DRAWING. ALL DISCREPANCIES TO BE REPORTED TO THE ARCHITECT. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELATED ARCHITECTS AND ENGINEER'S DRAWINGS AND ANY OTHER RELEVANT INFORMATION. THIS DRAWING IS COPYRIGHT © OF DWA ARCHITECTS (LONDON) LTD. DV ARCHITECTS DWA Architects (London) Ltd Cyclops House Link Business Park York United Kingdom YO10 3JB 01904 544 400 dwa@dwa-architects.co.uk www.dwa-architects.co.uk CLIENT Eric Wright Group PROJECT Proposed Care Home, Standen Central Site, Clitheroe North DRAWING TITLE Proposed Site Plan DWA PROJECT NO. DWA DWG NO. REV F. G5709 192 283 DRAWING STATUS PLANNING DRAWN BY CHECKED BY SCALE DATE 21.12.22 As indicated @ A1 TE . JA BIM JOB NO. 220018 10m 20m 30m -50m SUITABILITY CODE SUITABILITY DESCRIPTION VISUAL SCALE 1:500 DOCUMENT NAME (BS EN ISO 19650) 220018-DWA-XX-XX-DR-A-192-F-Proposed Site Plan



Appendix B – Site Information Drawing



NOTE

I. DO NOT SCALE FROM THIS DRAWING, WORK FROM FIGURED DIMENSIONS ONLY. ALL DIMENSIONS ARE IN MILLIMETRES AND ALL LEVELS ARE IN METRES ABOVE ORDNANCE DATUM U.N.O.

2. NO DEVIATION FROM THE DETAILS SHOWN ON THIS DRAWING WILL BE ALLOWED WITHOUT THE PRIOR PERMISSION IN WRITING.

3. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL RELEVANT ARCHITECTS, ENGINEERS AND SPECIALIST DRAWINGS AND SPECIFICATIONS.

 SITE CO-ORDINATES
 374889

 OS X (Eastings)
 374889

 OS Y (Northings)
 440726

 Nearest Post Code
 BB7 1FZ

 Nat Grid
 SD748407 / SD7488940726

 AREAS
 SITE BOUNDARY

 9,900m² (0.990 ha)

 ADDITIONAL IMPERMEABLE AREA

 5,440m² (0.544 ha)

 JR
 RH
 MM
 27.03.2023

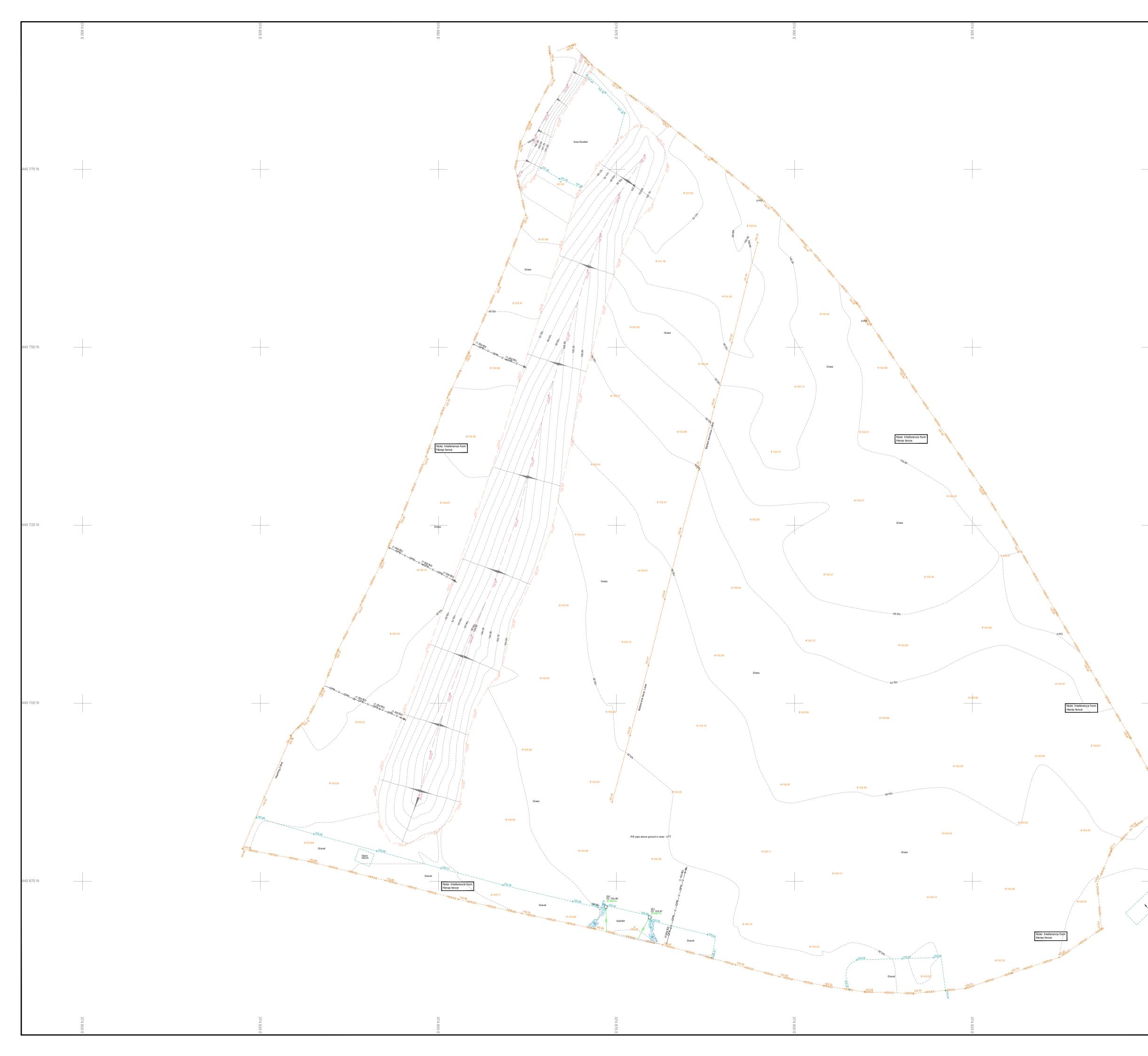
 BY
 CHK
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 DATE
 SITE LAYOUT AMENDED P02 REV DESCRIPTION PS CIVIL + STRUCTURAL ENGINEERS +44 (0)1244 893430 | www.jpstructural.co.uk | info@jpstructural.co.uk PURPOSE OF ISSUE STATUS STAGE 2 **S2** PROJECT CLITHEROE CARE HOME TITLE DRAINAGE SCHEME SITE INFORMATION CLIENT

ERIC WRIGHT CONSTRUCTION LTD

DRAWN BY	CHECKED BY	APPROVED BY	
JR	RH	MM	
DATE	SCALE (@ AI)	PROJECT NUM	BER
08.02.2023	AS SHOWN	113001	
DRAWING NUMBER	^{REV} P02		



Appendix C – Topographic Survey



		TOPOGR Barrier (symbol - sized)	APHIC LEGEND	Pipe into Ground (symbol)
	●BB ●B	Belisha Beacon (symbol) Bollard (symbol)	OPO RWP	Post (symbol) Rain Water Pipe (symbol)
		Borehole (symbol) British Telecoms IC (sized)	ORS ♦RE ⑤	Road Sign (symbol) Rodding Eye (symbol) Sapling (symbol)
	⊗ ¤ctv	Building (incomplete detail) Cable into Ground (symbol) Cable TV Box (symbol)	OSP OSL 50.00 +	Sign Post (symbol) Spot Light (symbol) Spot Height
	CL 50.00	CCTV Camera (symbol) Cover Level in metres Direction of Flow (Drainage)	□SC □ŞV	Stop Cock (symbol) Stop Valve (symbol)
	DB ER	Distribution Board (symbol) Earth Rod (symbol)	●TP ■B	Survey Station (symbol) Telephone Pole (symbol) Traffic Bollard (symbol)
	EP ES	Electric Cabinet (sized) Electric Pole (symbol) Electric Sign (symbol)		ree (dimensions in metres - e.g.) iameter at 1mh: (bole) oread:
	[®] ∰ → ■ FP	mbankment Slope (symbol) Feeder Pillar (sized)	Maple Sp	oread: eight: becies: Traffic Light (symbol)
	□ FH● FP● FL	Fire Hydrant (sized) Flag Pole (symbol) Flood Light (symbol)		Unknown Valve (symbol) Vent Pipe (symbol)
	GV Single Double	Gas Valve (symbol) Gate	●WP □WM	Wall Waste Pipe (symbol) Water Meter (symbol)
	GL 50.00 □ GU □ IC	Ground Level in metres Gulley (sized) Inspection Cover (sized)	□WV	Water Valve (symbol) Building Canopy
40 775 N	IL 50.00	Invert Level in metres Junction Box - BT (sized) nction Box - Comms (sized)	Drainage Channel	Crash Barrier Drainage Channel
	□ JB ●LP ●LB	Junction Box - Elec (sized) Lamp Post (symbol) Light Bollard (symbol)	<u> </u>	Drop Kerb (level in metres) Edge Detail (level in metres) Footpath (level in metres)
	●LIG □ MH	Light in ground (symbol) Manhole (sized)	— / — / — — — HERAS— — — 50.00	Fence Heras Fence Kerb channel (m)
]⊖ PL 50.00 Ø100	Manhole Capped Port Parapet Level (m) Pipe Diameter (mm)	OHS	Kerb top (m) Overhead Service Line Pedestrian Railing
		UTILITY		r coostnan rtaining
	UTILITY LINET —— BT —— —— CATV ——	YPES British Telecom Cable Television	G	Gas Heating Pipes
	CCTV C	Closed Circuit Television Communications	HV MSR	HV Cables Multiple Services Route
	DR CW FW	Drainage Drainage - Combined Water Drainage - Foul Water	OIL OXY SL	Oil Oxygen Street Lighting
	SW ED E	Drainage - Storm Water Empty Duct Electric	TSL GPR U U	Traffic Signal Loop Unknown found by GPR Unknown
	FH FO	Fire Hydrant Main Fibre Optic Fuel	UC UPL W	Unknown Cable Unknown Pipeline Water
	Assumed Service	Route - See GENERAL NOTES n of service and colour as above	ASR	XXX —ASR—XXX ——
	AOC	EY INFORMATION Area of Concern	UTS	Unable to Survey
	PREVIOUS	Contracted Survey Area Previous Survey Area Post processed GPR Area	UTL II⊃ II⊠	Unable to Lift (cover) Unable to Trace Unable to Trace (due to blockage)
	(O.60d)	Depth to Top of Service (m) ABREVIATIONS use	止 山	Unable to Trace Further End of Trace
40 750 N		B2P) 0.65d = Depth in metres,		,
	GENERAL NOTE		d by a utility s	urvey and statutory plans,
		should be undertaken w - HSG47 Avoiding Danger		ion and in accordance with d Services
		utility information is provided ith positional referencing.	d. Above ground ut	ility information may be shown
		nue outside of the survey a oses only and may not repre		rks outside of the area are for of the sub-surface utilities.
				positively confirmed with the ervice routes (ASR) have been
	highlighted with a	background yellow colour fo	r visual enhancem	
	measurement fror	n the surface and as such ca	annot be guarantee	d
	utility/feature and	is recorded as (x.xxd) i.e. (1.25d) - (depth to	s indicative to the top of the top of service) and should not e rather than the service itself.
	pipes/ducts may	have been detected using	g threading and t	oth is not shown. Drains and he depth indicated could be on is indicative to the centre of
	the utility/feature	and should not be taken as e	exact.	
	may become illeg is given in respec	ble quickly depending on gr	ound, weather and markings and that	traffic conditions. No warranty they are a complete
		the sub-surface utilities, ther for the survey results.	efore, this drawing	should be used as the
	been detected us	ing non-invasive technolog	ies only and the p	the sub-surface. Utilities have performance can be adversely MO's control, therefore, some
40 725 N	utilities may be un does not warrant	detectable. While SUMO us that 100% detection will be	es reasonable end achieved and that	leavours to detect all utilities, it approximate depth penetration
	of the technologie	s SUMO uses will not be gre	eater than two metr	es.
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