

# soiltechnics

environmental and geotechnical consultants

Proposed residential development  
Land east of Chipping Lane  
Longridge, Preston

Ground Investigation Report  
(Phase 2)

Ivy Mill Business Centre, Crown Street, Failsworth, Manchester M35 9BG

**t:** 0161 9470270

**e:** [mail@soiltechnics.net](mailto:mail@soiltechnics.net)

**w:** [www.soiltechnics.net](http://www.soiltechnics.net)

## Aerial photograph of site



Approximate Phase 2 site boundaries edged in pink

# **1 Executive summary**

## **General**

We recommend the following executive summary is not read in isolation to the main report which follows.

## **Site description, history and development proposals**

The site comprised three open grassed fields separated by mature hedgerows and sporadic trees, positioned on the north-western outskirts of Longridge, Preston. It is understood that the land is currently used by livestock for grazing. Localised ponding of surface water was evident across the site. Higgin Brook is also recorded adjacent to the north-western boundary. Historically the site has remained undeveloped farm land. We understand the scheme in its entirety will comprise the construction of up to 363 dwellings within what is termed Phases 1 and 2 (refer to Drawing 02a for details), with associated landscaping, gardens, hardstanding and access roads. This report refers to the Phase 2 area only in which 245 dwellings are proposed.

## **Ground conditions encountered**

Near surface soils generally comprised Topsoil (to depths of between 0.2m and 0.4m) overlaying cohesive Devensian Till deposits to beyond depths of investigation (>3.2m). Till comprised low to very high strength brown mottled grey and orange brown/grey, slightly silty to silty, slightly sandy, slightly gravelly clay in the initial 1m-1.5m below surface level. Below such depths deposits generally exhibit an increase in shear strength and trend towards a brown mottled grey, dark brown and reddish brown colour with varying amounts of silt, sand and gravel. Made Ground was also present in four locations and extended to depths between 1.2m and 2.0m. It is considered possible these soils are associated with some general infilling of depressions on the site and possibly historic ponds.

## **Foundation solution**

Traditional strip/trench fill type foundations considered suitable for the site, located at a minimum depth of 0.9m. Foundations will require deepening locally due to the presence of Made Ground, extending to depths of between 1.5m and 2.3m in such areas. CBR value of 2.17% considered representative of near surface soils. Buried concrete at the site would be classified as DS-1 AC-1s based on to sulphate levels in Made Ground and Devensian Till. Infiltration testing indicates that the near surface Devensian Till deposits are impermeable. Severe instability in foundation/service trenches is unlikely. No groundwater recorded at the site. Refer to Section 7 for further details and recommendations on additional investigations.

## **Chemical and gaseous contamination**

We have not identified any significant chemical or gaseous contamination at the subject site, therefore, remediation is not considered necessary. We recommend that hardness values within surface waters of Higgin Brook are determined to enable a more detailed risk assessment to be completed in relation to water receptors. It is unlikely that protected water supply pipes will be required at the site.

## 2 Introduction

2.1	Objectives
2.2	Client instructions and confidentiality
2.3	Site location and scheme proposals
2.4	Report format and investigation standards
2.5	Status of this report
2.6	Report distribution

### 2.1 Objectives

- 2.1.1 This report describes a ground investigation carried out for the Phase 2 area of a proposed residential development located on land east of Chipping Lane, Longridge, Preston PR3 2NA.
- 2.1.2 The objective of the ground investigation was to establish ground conditions at the site, sufficient to identify possible foundation solutions for the development and provide parameters necessary for the design and construction of foundations.
- 2.1.3 The investigation included an evaluation of potential chemical and gaseous contamination of the site leading to the production of a risk assessment in relation to contamination.
- 2.1.4 A Phase 1 Desk Study Assessment has been previously undertaken for the site by Curtins Consulting Ltd (ref EB1355/GL/3692 Revision A dated April 2014). A copy of their report is presented in Appendix L. We understand that we have the benefit of using such information and have provided a summary of the data in Section 3 of this report. This will also form a basis for our interpretative chemical and gaseous contamination assessments presented in Sections 8 and 9 respectively.
- 2.1.5 The investigation has also been produced to support a planning application for the site (ref 3/2014/0764) by satisfying National Planning Policies Framework sections 120 and 121.

### 2.2 Client instructions and confidentiality

- 2.2.1 The investigation was carried out in February 2016 and reported in April 2016 acting on instructions received from our client Barratt Homes (Manchester).
- 2.2.2 This report has been prepared for the sole benefit of our above named instructing client, but this report, and its contents, remains the property of Soiltechnics Limited until payment in full of our invoices in connection with production of this report.
- 2.2.3 Our original investigation proposals were outlined in our correspondence to Barratt Homes of 20<sup>th</sup> January 2016. The investigation generally followed our original investigation proposals. The investigation process was also determined to maintain as far as possible the original investigation budget costs.

## **2.3 Site location and scheme proposals**

- 2.3.1 The National Grid reference for the site is 360447, 437970. A plan showing the location of the site is presented on Drawing 01, with the extent of the development phases presented on Drawing 02a.
- 2.3.2 We understand the scheme in its entirety will comprise the construction of up to 363 dwellings within what is termed Phases 1 and 2 (refer to Drawing 02a for details), with associated landscaping, gardens, hardstanding and access roads. This report refers to the Phase 2 area only in which 245 dwellings are proposed.
- 2.3.3 We have received layout drawings of the proposed scheme with the layout presented on Drawing 03.

## **2.4 Report format and investigation standards**

- 2.4.1 Sections 2 to 6 of this report describe the factual aspects of the investigation with Section 7 presenting an engineering assessment of the investigatory data. Section 8 provides a risk assessment of chemical contamination based on readily available historic records, inspection of the soils and laboratory testing. Section 9 provides a similar risk assessment in relation to gaseous contamination with Section 10 providing a risk assessment relating to construction materials likely to be in contact with the ground. Section 11 discusses issues relating to classification of waste soils for disposal and reuse.
- 2.4.2 This investigation integrates both contamination and geotechnical aspects. The investigation was carried out generally, and where practical following the recommendations of BS EN 1997:2 2007 '*Eurocode 7 – Geotechnical Design – Part 2: Ground Investigation and Testing*'. The investigation process also followed the principles of BS10175: 2011 '*Investigation of potentially Contaminated Sites – Code of Practice*'. The following elements, defined in BS10175, have been completed and incorporated in this report.
- a) Phase I Preliminary investigation (desk study) – review of existing Phase 1 report undertaken by Curtins Consulting Ltd and site reconnaissance undertaken by Soiltechnics Ltd
  - b) Phase II Exploratory and main (intrusive) investigations
- 2.4.3 The extent and result of the preliminary investigation (desk study) undertaken by Curtins Consulting Ltd, in addition to site reconnaissance undertaken by Soiltechnics Ltd, is reported in Section 3. Fieldwork combined the exploratory investigation and main investigation stages into one phase with the extent of these works described in Sections 4 and 6 of this report. Any supplementary investigations deemed necessary are identified in Section 12. Section 13 provides information on any remedial strategy and specification if required.

## 2.5 Status of this report

2.5.1 This report is final based on our current instructions.

2.5.2 This investigation has been carried out and reported based on our understanding of best practice. Improved practices, technology, new information and changes in legislation may necessitate an alteration to the report in whole or part after publication. Hence, should the development commence after expiry of one year from the publication date of this report then we would recommend the report be referred back to Soiltechnics for reassessment. Equally, if the nature of the development changes, Soiltechnics should be advised and a reassessment carried out if considered appropriate.

## 2.6 Report distribution

2.6.1 This report has been prepared to assist in the design and planning process of the development and normally will require distribution to the following parties, although this list may not be exhaustive:

**Table summarising parties likely to require information contained in this report**

Party	Reason
Client	For information/reference and cost planning.
Developer/Contractor/project manager	To ensure procedures are implemented, programmed and costed.
Planning department	Potentially to discharge planning conditions.
Environment Agency	If ground controlled waters are affected and obtain approvals to any remediation strategies.
Independent inspectors such as NHBC/Building Control	To ensure procedures are implemented and compliance with building regulations.
Project design team	To progress the design.
Principal Designer (PD)	To advise in construction risk identification and management under the Construction (Design and Management) Regulations.
<b>Table 2.6</b>	

### 3 Desk study information and site observations

3.1	General
3.2	Description of the site
3.3	Injurious and invasive weeds and asbestos
3.4	History of the site
3.5	Geology and geohydrology of the area
3.6	Landfill and infilled ground
3.7	Radon
3.8	Flood risk
3.9	Enquiries with Statutory Undertakers
3.10	Enquiries with Local Authority Building Control and Environmental Health Officers

#### 3.1 General

- 3.1.1 A Phase 1 Detailed Desk Top Study has been previously undertaken for the site by Curtins Consulting Ltd (reference EB1355/GL/3692, revision A, issue 01, dated 14<sup>th</sup> April 2014). A copy of their report is presented in Appendix L. We understand that we have the benefit of using such information and have provided a summary of the data in following paragraphs, together with our own site observations. It should be noted that we have tailored the information to suite the current site boundary for the Phase 2 development area, which is shown in a slightly different position in the Curtins report.

#### 3.2 Description of the site

- 3.2.1 The site is positioned on the north-western outskirts of Longridge, Preston, at an elevation of between approximately 106m and 117m AOD and with the topography of the site falling in a north-westerly direction. The site, approximately 7.7Ha in size, comprised three open grassed fields separated by mature hedgerows and sporadic trees between approximately 2m and 15m in height. It is understood that the land is currently used by livestock for grazing. Localised ponding of surface water was evident across the site. Higgin Brook is recorded along part of the north-western site boundary and flows in a north-easterly direction. Each parcel of land onsite was accessed via a gate within the hedge line. Manhole covers were also present in south-western corner of the site. An underground services access area was also located along the southern boundary.
- 3.2.2 The site was bound to the north, west (Phase 1 area) and east by further open grassed fields. Residential housing bordered the site to the south. A supermarket was located off the south-western corner of the site.
- 3.2.3 A plan showing existing site features and location of exploratory points is presented as Drawing 02b.



### **3.3 Injurious and invasive weeds and asbestos**

#### **3.3.1 Injurious and invasive weeds**

3.3.1.1 The following weeds are controlled under the Weeds Act 1959:

- Common ragwort
- Spear thistle
- Creeping (or field) thistle
- Broad-leaved dock
- Curled dock

3.3.1.2 Whilst it is not an offence to have the above weeds growing on your land, you must:

- Stop them spreading to agricultural land, particularly grazing areas or land used for forage, like silage and hay
- Choose the most appropriate control method for the your site
- Not plant them in the wild

3.3.1.3 Should you allow the spread of these weeds to another parties land, Natural England could serve you with an Enforcement Notice. You can also be prosecuted if you allow animals to suffer by eating these weeds.

3.3.1.4 In addition to the above, you must not plant in the wild or cause certain invasive and non-native plants to grow in the wild as outlined in the Wildlife and Countryside Act 1981. It is an offence under section 14(2) of the act to '*plant or otherwise cause to grow in the wild*' any plants listed in schedule 9, part II. This can include moving contaminated soil or plant cuttings. The offence carries a fine or custodial sentence of up to two years. The most commonly found invasive, non-native plants include:

- Japanese knotweed
- Giant hogweed
- Himalayan balsam
- Rhododendron ponticum
- New Zealand pigmyweed

3.3.1.5 You are not legally obliged to remove these plants or to control them. However, if you allow Japanese knotweed to spread to another party's land, you could be prosecuted for causing a private nuisance.

3.3.1.6 The presence of such weeds on site may have considerable effects on the cost/timescale in developing the site. Japanese knotweed can cause significant damage to buildings, roads and pavements following development, if untreated prior to development.



- 3.3.1.7 Our investigations exclude surveys to identify the presence of injurious and invasive weeds. We did not observe any obvious evidence the above species; however, we recommend specialists in the identification and procedures to deal with injurious and invasive weeds are appointed prior to commencement of any works on site.

### 3.3.2 Asbestos

- 3.3.2.1 Our investigations exclude surveys to identify the presence or absence of asbestos on site. It should be noted, however, that where intrusive investigations were undertaken we did not observe any obvious evidence of potential asbestos containing materials. This information does not constitute a site-specific risk assessment and we recommend specialists in the identification and control/disposal of asbestos are appointed prior to commencement of any works on site.

- 3.3.2.2 The presence of asbestos on site may have considerable effects on the cost/timescale in developing the site. There is good guidance in relation to asbestos available on the Health and Safety Executive (HSE) website.

## 3.4 History of the site

- 3.4.1 The recent pertinent history of the site, updated from the Curtins summary to reflect the current site boundary, is presented in the following table:

Summary description of site history		
Date	On site	Off site
1847	Open fields including a number of small ponds and marshy areas.	Surrounding land predominantly agricultural. A road is located to the south-west of the site. Quarrying works recorded between 500m and 1000m east of the site.
1893 to 1914	No significant change	Higgin Brook partially borders the site to the north-west. Some development to the west and south of the site including Pitt Street Mills (Corn & Bone) and a smithy are some 90m to the west. An iron and brass foundry was present c.440m to the west of the site.
1932 to 1956	No significant change	The Pitt Street Mills (Corn & Bone) and smithy buildings recorded as a Bobbin works.
1961 to 1967	No significant change	The Bobbin works is no longer recorded and the site has been redeveloped as Ashley Dairy, which borders the site to the south-west. Some residential development has also occurred to the west.
1968 to 1975	No significant change	The iron and brass foundry was labelled as a works. Significant development is occurring to the south of the site.
1975 to 1996	No significant change	No significant changes
2001 to 2013	No significant change	Ashley Dairy has been redeveloped as a superstore.
Table 3.4.1		

### 3.5 Geology and geohydrology of the area

#### 3.5.1 Geology of the area

3.5.1.1 The geology of the area, updated from the Curtins summary to reflect the current site boundary, is presented in the following table:

Summary of geology and likely aquifer-containing strata					
Stratum	Bedrock or superficial	Approximate thickness	Typical soil type	Likely permeability	Aquifer designation
Devensian Till	Superficial	>5m	Clay with silt and sand	Low	Unproductive strata (r)
Pendleside Sandstone Member	Bedrock	Up to 50m	Sandstone with mudstone and siltstones	Low to moderate	Secondary A aquifer (r)
Bowland Shale Formation	Bedrock	Up to 200m	Mudstone, siltstone with sandstones	Low to moderate	Secondary A and secondary undifferentiated aquifers (r)

**Table 3.5.1**

(r) recorded aquifer designation

(a) assumed aquifer designation

3.5.1.2 Unproductive strata are defined as deposits exhibiting low permeability with negligible significance for water supply or river base flow. Unproductive strata are generally regarded as not containing groundwater in exploitable quantities.

3.5.1.3 Secondary A aquifers are predominantly permeable layers capable of supporting water supplies at a local, rather than strategic, scale. In some cases, Secondary A aquifers can form an important source of base flow to rivers.

3.5.1.4 Secondary undifferentiated aquifer is a designation used when it is not possible to attribute fully one of either Secondary A or Secondary B, due to the variable nature of the soils. The unit will therefore be a mix of both, which are defined as follows:

- Secondary A can be defined as: Permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers.
- Secondary B can be defined as: layers which may store limited amounts of ground water. These groundwater stores are generally the water bearing parts of former aquifers.

#### 3.5.2 Water abstractions

3.5.2.1 There are no potable groundwater abstraction licences within 2km of the site. The only surface water abstraction within a 2km radius of the site is associated with field drains located approximately 445m to the south of the site. Details of the water's use are not supplied. There are two groundwater abstractions within a 2km radius of the site.

They are both associated with Singletons Dairy (Mill Farm, Preston) and are located approximately 890m and 975m to the south of the site. The abstracted water is used for general purposes.

- 3.5.2.2 The site is not located within a zone protecting a potable water supply abstracting from a principal aquifer (i.e. a source protection zone).

### 3.5.3 Coal mining and brine extraction

- 3.5.3.1 The site is not recorded to be within an area affected by past or present coal mining, minerals worked in association with coal, or brine extraction (within the Cheshire Brine Compensation District). The site does not lie within a coal mining referral area and, as such, a Coal Authority report is not required.

### 3.5.4 Shallow mining and natural subsidence hazards

- 3.5.4.1 The British Geological Survey presents hazard ratings for shallow mining and natural subsidence hazards. The site has the following ratings:

Table summarising mining and subsidence hazards	
Hazard	Rating
Mining hazard in non-coal mining areas	Highly unlikely
Potential for collapsible ground stability hazard	Very low / no hazard
Potential for compressible ground stability hazard	Moderate / no hazard
Potential for ground dissolution stability hazard	Low / very low
Potential for landslide ground stability hazard	Very low
Potential for running sand ground stability hazard	Low / very low
Potential for shrinking or swelling clay ground stability hazard	Very low
Table 3.5.4	

- 3.5.4.2 The moderate potential for compressible ground stability hazards is likely to be associated with the deposits of Alluvium recorded in northern part of the original site boundary that the Envirocheck encompasses. It should be noted that this area is located approximately 150m to the north-west of the Phase 2 development area.

### 3.5.5 Borehole records

- 3.5.5.1 The British Geological Survey (BGS) retains records of boreholes formed from ground investigations carried out on a nationwide basis. However, there are no BGS borehole records in the vicinity of the site.

## 3.6 Landfill and infilled ground

- 3.6.1 Within a 2km radius of the site, there are no BGS recorded or historical landfill sites; however, there are two registered landfill sites. Lords Delph (Forty Acre Lane, Longridge) is located approximately 645m to the east of the site and has been accepting non-biodegradable waste since at least 1982. Chapel Hill Quarry is located approximately 960m to the south of the site and accepted non-biodegradable waste; in 1992, the site was recorded as dormant.

- 3.6.2 In addition, we have reviewed old Ordnance Survey maps and there is no obvious evidence of significant quarrying in the area, other than a small number of BGS mineral sites, recorded between 400m and 850m of the subject site which exploited the underlying clays and grits. The geological map of the area indicates areas of infilled ground which approximately coincide with such areas.

### **3.7 Radon**

- 3.7.1 Envirocheck uses the British Geological Survey database to review reported radon levels in the area in which the site is located, to establish recommended radon protection levels for new dwellings. The database presented in the Curtins report indicates that the site is located in an area where no protection is considered necessary.
- 3.7.2 Building Research Establishment (BRE) publication BR211 '*Radon: guidance on protective measures for new buildings*' (2007) applies to all new buildings, conversions and refurbishments, whether they be for domestic or non-domestic use.
- 3.7.3 It is noteworthy that the BRE information is based on statistical analysis of measurements made in dwellings, in combination with geological units which are known to emit radon. Therefore there is a risk that actual radon levels at the site will exceed the levels assessed by the BRE. Currently, the only true method of checking actual radon levels is by measurement within a building on the site over a period of several months. It should be noted that it is not currently a requirement of the Building Regulations to test new buildings for radon; however, the BRE recommends testing on completion or occupation of all new buildings (domestic and non-domestic), extensions and conversions. Should you wish to undertake radon monitoring following completion of the development, we can provide proposals.

### **3.8 Flood risk**

- 3.8.1 Based on the information provided within the Curtins report, the site is not located within a fluvial or tidal flood plain. It should be noted that this information does not constitute a site-specific Flood Risk Assessment (FRA) and that a full FRA may be required for the development to support a planning application or to satisfy planning conditions.

### **3.9 Enquiries with Statutory Undertakers**

- 3.9.1 We have been provided with the following Statutory Undertaker (SU) records in order to avoid damaging their apparatus during our fieldwork activities:
- a) BT Openreach
  - b) Electricity North West
  - c) ESP Utilities Group
  - d) National Grid Gas
  - e) United Utilities

- 3.9.2 Copies of these records are presented in Appendix J. These records have been obtained solely for the purposes described above.
- 3.9.3 Normally Statutory Undertakers' drawings record the approximate location of their services. We recommend further on-site investigations be undertaken to confirm the position of the apparatus and thus establish the effect on the proposed development and the necessity or otherwise for the permanent or temporary diversion of the service to allow the construction of the development to safely and successfully proceed.
- 3.9.4 It should be noted that a United Utilities surface water sewer crosses the south-western part of the site and discharges into Higgin Brook. We are not aware that the supply to such services has been capped off-site and, as such, they should be treated as live until further information indicates otherwise.
- 3.9.5 It should be noted that Statutory Undertakers' records normally exclude private services.

### **3.10 Enquiries with Local Authority Building Control and Environmental Health Officers**

- 3.10.1 We have contacted the Local Authority Building Control and are awaiting a response. We will update this report if anything of concern arises.
- 3.10.2 We have contacted the Local Authority Environmental Health Officer, who has confirmed that no gas monitoring is required on this site, due to the limited number of sources and pathways in the area (refer to Section 9 for further details). A copy of their correspondence is presented in Appendix K.

## 4 Fieldwork

---

4.1	General
4.2	Site restrictions
4.3	Exploratory trial pits
4.4	Driven tube sampling
4.5	Dynamic cone penetration testing
4.6	Sampling strategies

---

### 4.1 General

4.1.1 Fieldwork was carried out between the 16<sup>th</sup> and 18<sup>th</sup> February 2016 and comprised the following activities:

- Excavation of twenty five exploratory trial pits.
- Excavation of eight exploratory boreholes using driven tube sampler drilling techniques.
- Dynamic cone penetration testing in three locations.
- Infiltration testing undertaken in two trial pits.

4.1.2 A plan of the site showing observed/existing site features and position of exploratory points is presented on Drawing 02b. The position of exploratory points relative to site development proposals is presented on Drawing 03. The position of exploratory points shown on these plans is approximate only and confirmation of these positions is subject to dimensional surveys, which is considered outside our brief.

4.1.3 The extent of fieldwork activities and position of exploratory points were determined by Soiltechnics Limited.

4.1.4 Exploratory points were positioned to avoid known locations of underground services, to avoid possible location of proposed foundations and to provide a reasonable coverage of the site. Prior to commencement of exploratory excavations an electronic cable locating tool was used to scan the area of the excavation. If we received a response to this equipment then the excavation would be relocated.

4.1.5 All soils exposed in excavations were described in accordance with BS EN ISO 14688 '*Identification and Classification of soil*'.

### 4.2 Site restrictions

4.2.1 No significant site restrictions were encountered during investigations with the exception that trial pit excavations were undertaken using tracked plant due to waterlogged nature of the site and care was taken to avoid the 375mm diameter surface water sewer which cuts across the southern most parcel of land and outfalls into Higgin Brook.

### 4.3 Exploratory trial pits

- 4.3.1 Trial pits TP101 to TP125 were excavated to a maximum depth of 3.3m using a 360° tracked excavator. The excavations were backfilled with excavated material compacted using the back of the excavator bucket. Whilst we attempted to reinstate the excavation to its original condition the soils could not be fully compacted into the trial pit and thus the soils were left proud of the ground surrounding the pit to allow for short-term settlement of the backfill. A Geotechnical Engineer supervised the excavations.
- 4.3.2 Sampling and logging was carried out as trial pit excavations proceeded but trial pits were not entered at depths exceeding 1.2m, or where sides were deemed unstable. The density of granular soils encountered in excavations was gauged by the ease of excavation by spade or penetration of a geological pick.
- 4.3.3 Soil samples for subsequent laboratory determination of concentration of chemical contaminants were taken from the sides of trial pits and stored in new plastic containers, which were labelled and sealed. Samples from below access depth into trial pits were taken as a sub sample from soil contained in the excavator bucket, discarding any soil, which may have been in contact with the bucket. If as a consequence of visual or olfactory evidence, a sample was suspected to be contaminated by organic material, the sample was stored in an amber glass jar with a PTFE sealing washer.
- 4.3.4 Soil samples for subsequent 'classification' laboratory testing were taken from the side of trial pits or from bulk samples taken from the excavator bucket. The sample was placed in a plastic bag and subsequently sealed and labelled. Samples for moisture content determination were placed in sealable tubs and appropriately labelled.
- 4.3.5 Soil samples were obtained to meet quality class 3 to 5 as described in BS EN 1997-2:2007. Sample sizes were appropriate for the laboratory test being considered.
- 4.3.6 A hand held shear vane was used where possible to provide a measure of the undrained shear strength of cohesive soils exposed in excavations. The vane test was carried out in the sides and floor of trial pits by access into the trial pit to depths of 1.2 metres. At depths in excess of 1.2 metres the tests were undertaken using extension rods to a maximum depth of 3.4 metres or by carrying out tests on intact clods of cohesive soils (exceeding 0.3m x 0.3m x 0.3m) removed from the trial pit using the excavator bucket. The apparatus reads to a maximum shear strength of 213kN/m<sup>2</sup> following conversion of the readout or 'division' taken from the instrument. Conversion is either undertaken using the calibration chart or by multiplying the division by the shear strength constant supplied with the instrument. The results are reported in columns to the right of the trial pit record. The shear vane is not a reliable tool for assessing insitu shear strength of stony or sandy cohesive soils.



- 4.3.7 A pocket penetrometer was also used in the cohesive soils encountered. This tool is deemed to measure the apparent ultimate bearing capacity of the soil under test. The pocket penetrometer is calibrated in  $\text{kg}/\text{cm}^2$ . The reading can be approximately converted to equivalent undrained shear strength by multiplying the results by a factor of 50. Tests were carried out in the sides of trial pits when access can be safely achieved otherwise testing was carried out on excavated intact clods. The results are reported in columns to the right of trial pit results. The pocket penetrometer is not covered by British Standards. This tool has the advantage that it can be used to determine the approximate insitu undrained shear strength of stony cohesive soils.
- 4.3.8 A summary of pocket penetrometer and hand held shear vane results obtained from the cohesive soils encountered in exploratory excavations are presented in graphical format on Drawings 04 and 05 respectively.
- 4.3.9 Trial pit records are presented in Appendix C.
- 4.3.10 Soil infiltration tests were carried out in trial pits TP107 and TP114 at depths of between 1.3m and 2.8m. Infiltration tests were carried out following the procedures described in the Building Research Establishment (BRE) Digest 365 (2007) "*Soakaway Design*", with records of test results presented in Appendix E. Water placed in each trial pit did not dissipate and soils are considered to be effectively impermeable.

## **4.4 Driven tube sampling**

- 4.4.1 Boreholes DTS101 to DTS108 were formed using driven tube sampling equipment. Driven tube sampling comprises driving 1m long steel sample tubes which are screw coupled together or coupled to extension rods and fitted with a screw on cutting edge. The sample tubes are of various diameters, generally commencing with 100mm and reducing, with depth, to 50mm and include a disposable plastic liner which is changed between sampling locations in order to limit the risk of cross contamination. On completion of excavation the liner containing the sample is cut open and the soil sample logged by a geo-environmental engineer.
- 4.4.2 Samples for determination of the concentration of chemical contaminants are taken from samples obtained in the disposable tubes as sub-samples using stainless steel sampling equipment, which is cleaned with de-ionised water.
- 4.4.3 The driven tube sampler obtains samples under category A allowing laboratory test quality classes 3 to 5 as described in BS EN ISO 22475-1:2006.
- 4.4.4 A pocket penetrometer was used in the cohesive soils retrieved from the boreholes. This tool is deemed to measure the apparent ultimate bearing capacity of the soil under test. The pocket penetrometer is calibrated in  $\text{kg}/\text{cm}^2$ . The reading can be approximately converted to an equivalent undrained shear strength by multiplying the results by a factor of 50. The results are reported on borehole records. The pocket penetrometer is not covered by British Standards.

4.4.5 A summary of pocket penetrometer results obtained from the cohesive soils retrieved from the boreholes are presented in graphical format on Drawing 04.

4.4.6 Records of boreholes formed using driven tube sampling techniques are presented in Appendix D.

## **4.5 Dynamic cone penetration testing**

4.5.1 Dynamic Cone Penetration (DCP) testing was carried out in three locations. Dynamic Cone Penetration testing consists of driving a 50mm diameter, 90° cone into the ground, via an anvil and extension rods with successive blows of a freefall hammer. The number of blows required to drive the cone each successive 100mm (N100) is recorded.

4.5.2 Dynamic Cone Penetration testing was carried out following BS EN ISO 22476-2:2005 and the apparatus used was categorised as 'super heavy' (DPSH-B) in accordance with the standard.

4.5.3 Dynamic cone penetration test data is presented in graphical format on Drawing 06.

## **4.6 Sampling strategies**

### **4.6.1 Geotechnical**

4.6.1.1 In general we adopted a judgemental sampling strategy in relation to geotechnical aspects of the investigation. The location and frequency of sampling was carried out in consideration of the following:

- i) Topography
- ii) Geology (including Made Ground)
- iii) Nature of development proposals

### **4.6.2 Environmental**

4.6.2.1 Details of sampling with respect to contamination issues are described in Section 8.

### **4.6.3 Sample retention**

4.6.3.1 Samples are stored for a period of one month following issue of this report unless otherwise required.

## 5 Ground conditions encountered

---

5.1	Soils
5.2	Topsoil
5.3	Groundwater

---

### 5.1 Soils

- 5.1.1 Each exploratory excavation encountered a similar profile of soils considered to be Topsoil overlying Devensian Till deposits. Locally, deposits of Made Ground were also encountered.
- 5.1.2 **Topsoil** deposits were generally encountered as soft dark brown slightly sandy to sandy slightly gravelly organic clay with frequent rootlets. The gravel comprised quartzite and sandstone. Topsoil was encountered to depths between 0.2m and 0.4m below existing surface levels.
- 5.1.3 **Made Ground** deposits were encountered in four locations (TP101, TP103, TP108 and TP125) and generally comprised firm high strength brown sandy gravelly clay and medium dense grey and brown clayey sand. The gravel included sandstone, quartzite, brick with localised timber, ceramic, wire and glass. The Made Ground extended to depths between 1.2m and 2.0m.
- 5.1.4 **Devensian Till** deposits were encountered as cohesive soils across the site, comprising low to very high strength brown mottled grey and orange brown/grey, slightly silty to silty, slightly sandy, slightly gravelly clay in the initial 1m-1.5m below surface level. Below such depths deposits generally exhibit an increase in shear strength and trend towards a brown mottled grey, dark brown and reddish brown colour with varying amounts of silt, sand and gravel.

### 5.2 Topsoil

- 5.2.1 As a practice we have adopted the following policy for description of Topsoil. If surface soils exhibit a visually significant organic content and darker colour than the soils it overlies (which are considered to be naturally deposited) then we will describe the soil as Topsoil. In some cases it is difficult to visually distinguish the interface between Topsoil and subsoils below, which may also exhibit an organic content, and in such cases we will adopt an estimate of the interface but may also use the terms 'grading into' with some defining depths.
- 5.2.2 If 'Topsoil' deposits include materials such as ash, brick and other man made materials, or the Topsoil overlies Made Ground deposits we will term the material 'Made Ground', even though it may still be able to support vegetable growth, and potentially be reused as Topsoil.

- 5.2.3 Topsoil can be classified following a number of test procedures as described in BS3882: 2007 '*Specification for Topsoil and Requirements for use*', to allow its uses to be determined. We do not carry out such testing unless specifically instructed to do so.

## **5.3 Groundwater**

- 5.3.1 A slight seepage of groundwater was observed within TP121 at a depth between 0.5-0.7m. No groundwater inflows were observed in any of the other excavations.

## 6 Laboratory testing

6.1	Classification testing
6.2	Chemical testing

### 6.1 Classification testing

6.1.1 Laboratory testing was carried out in accordance with BS1377: 1990 “*Methods of Test for Soils for Civil Engineering Purposes*” and limited to determination of:

- i) the liquid limit (one point cone penetrometer, method 4.4)
- ii) the plastic limit and plasticity index (method 5)

6.1.2 Laboratory testing was carried out by an independent specialist testing house, which operates a quality assurance scheme. Copies of laboratory test result certificates are presented in Appendix F.

### 6.2 Chemical testing

6.2.1 Laboratory testing was carried out as deemed necessary and carried out using the following techniques:

- Using inductively coupled plasma mass spectrometry (ICP-MS), determination of concentration of metals, semi-metals and soluble sulphate.
- Using gas chromatography flame ionisation detection methods (GC-FID), determination of concentration of petroleum hydrocarbons (TPH).
- Using gas chromatography flame ionisation detection methods (GC-FID), determination of concentration of polycyclic aromatic hydrocarbons (PAH).
- Using gas chromatography mass spectrometry (GS-MS), determination of the concentration of Volatile Organic Compounds (VOCs) and Semi-Volatile Organic Compounds (SVOCs).
- Using electromagnetic measurement, determination of pH.

6.2.2 Laboratory testing was carried out by an independent specialist testing house, which operates a quality assurance scheme. Copies of laboratory test result certificates are presented in Appendix G.

## 7 Engineering assessment

7.1	General description of the development
7.2	Building foundation design and construction
7.3	Influence of trees and other major vegetation
7.4	Ground floor construction
7.5	Foundation and Service trench excavations
7.6	Infiltration potential
7.7	Pavement foundations

### 7.1 General description of the development

7.1.1 The following assessments are made on the investigatory data presented in the preceding sections of this report and are made with reference to the specific nature of the development. Should scheme proposals change then it may be necessary to review the investigation and report.

7.1.2 We understand the scheme in its entirety will comprise the construction of up to 363 dwellings within what is termed Phases 1 and 2 (refer to Drawing 02a for details), with associated landscaping, gardens, hardstanding and access roads. The following assessment refers to the Phase 2 area only, in which 245 dwellings are proposed.

### 7.2 Building foundation, design and construction

7.2.1 Definitions of geotechnical terms used in the following paragraphs are provided in Appendix A.

#### 7.2.2 Ground conditions

7.2.2.1 A detailed summary of ground conditions is provided in Section 5. Essentially ground conditions comprised of Topsoil overlaying cohesive Devensian Till deposits, the latter comprising low to very high strength brown mottled grey and orange brown/grey clay in the initial 1m-1.5m below surface level. Below such depths deposits generally exhibit an increase in shear strength and trend towards a brown mottled grey, dark brown and reddish brown colour with varying amounts of silt, sand and gravel.

7.2.2.2 A more consistent positive trend was noted in shear strength data derived using the shear vane within the Phase 2 development area compared to those taken within the Phase 1 area (refer to Drawing 05), with strength generally increasing with depth and only very localised softening of soils noted below depths of 2.0m. It is also noted that shear strength data derived using the pocket penetrometer is much more varied, with inconsistent strength measurements recorded with depth. The shear vane is, however, considered to provide much more reliable insitu shear strength results and we have adopted such values in the following foundation assessments.

7.2.2.3 Made Ground was encountered in trial pits TP101, TP103, TP108 and TP125 and extended to depths between 1.2m and 2.0m. The reason for the presence of such Made Ground is unclear as the site has never been developed, however, it is possibly due to infilling of localised depressions or associated with the construction of the underground services identified, particularly in the areas of TP101 and TP103. It is also acknowledged that historic ponds are also recorded adjacent to some areas of Made Ground onsite (TP103 and TP108) and Drawing 02b indicates the positions of these, which could have been backfilled. Foundations will need to extend through any Made Ground into the underlying Devensian Till at depth.

### 7.2.3 Foundation solution

7.2.3.1 In our opinion naturally deposited Devensian Till will adequately support proposed buildings on concrete strip/trench fill foundations. Based on laboratory determination of plasticity and following National House Building Council (NHBC) Standards Chapter 4.2, we recommend foundations extend to a minimum depth of 0.9m below existing or proposed ground levels whichever gives the deeper founding level. In all cases we recommend foundation excavations fully penetrate any Made Ground and extend into the Devensian Till by a minimum of 0.3m into the naturally deposited soils, subject to an overall minimum foundation depth of 0.9m. Based on such, in areas of the site where Made Ground deposits have been encountered, foundations are likely to be located between depths of 1.5m and 2.3m. It should be noted that there are a number of trees and major vegetation along field boundaries which may require foundation depths exceeding the minimum depth defined above. Further guidance on this is provided in the following report paragraphs.

7.2.3.2 Laboratory testing indicates the Devensian Till deposits are plastic, thus our assessment of bearing values are based on the assumption that these soils predominantly exhibit cohesion. Calculations, based on a conservative undrained shear strength of say 60kN/m<sup>2</sup> (derived from measured insitu shear strengths taken below proposed founding levels – refer Drawing 05) indicates the following bearing values for strip/trench fill foundations:

Table of bearing values for traditional strip/trench fill foundations			
Width (m)	Ultimate bearing value kN/m <sup>2</sup>	Presumed bearing value kN/m <sup>2</sup>	Allowable bearing pressure kN/m <sup>2</sup>
0.45	445	160	150
0.6	420	150	140
0.75	405	145	135
1.0	390	140	130
Table 7.2.3.2			

7.2.3.3 The presumed bearing value has been derived from the ultimate bearing value by applying a factor of safety of 3, and the allowable bearing pressure derived to limit total settlement.



- 7.2.3.4 It is difficult to accurately predict the amount of total and differential movement caused by consolidation of the foundation supporting subsoils, however, providing the foundation loads do not exceed the allowable bearing pressure provided in the preceding paragraph, we suggest total settlement will be small, and probably less than 25mm. Differential settlements are totally dependent on the variation of foundation loads and consistency of the supporting ground. Assuming the foundation loads are reasonably uniform, we suggest differential settlement is unlikely to exceed say 15mm. It is likely settlement will be fully achieved within 20 years of construction.
- 7.2.3.5 The Devensian Till deposits encountered in exploratory excavations are generally consistent and will provide uniform support to foundations. In the event foundation excavations encounter a soft area, we recommend foundation excavations continue to locate stiffer soils (see below) or reinforcement introduced into foundation concrete to span the soft area. Whilst not extensively evident within the Phase 2 development area, caution should be taken as such soils were recorded within localised areas of the Phase 1 development site and likely to be largely associated with prolonged and extensive water logging of the site. If there is any doubt when trenches are being excavated, a suitably qualified Geo-Environmental Engineer could attend site with the purpose of providing an indication when suitable founding strata has been reached.

## **7.3 Influence of trees and other major vegetation**

### **7.3.1 Soil classification and new foundation design**

- 7.3.1.1 The results of plastic and liquid limit determinations performed on samples of the Devensian Till indicate the deposits are soils of medium volume change potential when classified in accordance with National House Building Council (NHBC) Standards, Chapter 4.2. Foundations taken down onto a depth of 0.9m will penetrate the zone of shrinkage and swelling caused by seasonal wetting and drying. Trees and other major vegetation extend this zone and will require deeper foundations. A good guide to this subject is provided in NHBC Standards, Chapter 4.2.

### **7.3.2 New planting**

- 7.3.2.1 Any planting schemes should also take into account the effect that new trees could have on foundations when they reach maturity. Again a good guide to this subject is provided in NHBC Standards, Chapter 4.2.

### **7.3.3 Tree species identification**

- 7.3.3.1 There are a number of trees and other major vegetation located along field boundaries at the site. We recommend a qualified Arboriculturist (listed in the Arboricultural Association Directory of Consultants – [www.trees.org.uk](http://www.trees.org.uk)) be appointed to determine the location, height (and mature height) and water demand of all trees/major hedgerows at the site, information, which will be necessary to design foundations in accordance with NHBC Standards, Chapter 4.2.

#### **7.3.4 Agricultural crops**

- 7.3.4.1 It is important to note that the site at the time of our investigations comprised fields surfaced in rough grass and used for grazing livestock. Based on our site reconnaissance, anecdotal information and fieldwork observations, the likelihood that near surface soils have recently supported a crop is considered low.

### **7.4 Ground Floor Construction**

- 7.4.1 Ground bearing floor slabs can be adopted at this site where buildings are remote from trees and where Topsoil and Made Ground deposits are fully removed within the footprint of the building. We recommend a blanket of good quality compacted granular material be placed prior to construction of the floor slabs.
- 7.4.2 In areas close to existing major vegetation at the site, where significant Made Ground deposits are present (or where ground floors are elevated requiring in excess of 600mm of fills) then we recommend the use of a suspended ground floor with a sub floor void determined following NHBC Standards, Chapter 4.2.

### **7.5 Foundation and Service Trench Excavations**

- 7.5.1 Generally the sides of foundation/service trench excavations will remain stable and we anticipate no significant groundwater inflows will be encountered in any of the excavations. Some minor overbreak and instability could be encountered within trenches formed through deposits of Made Ground. The silty nature of the near surface Devensian Till deposits will render them moisture susceptible with small increases in moisture content promoting rapid deterioration. We recommend, therefore, that as soon as foundation trench excavations are opened foundation concrete be poured as quickly as practically possible.
- 7.5.2 We recommend any trench excavation requiring human entry is shored as necessary to conform with current best practice, and accepted by the Health and Safety Executive (HSE) and in particular, following guidance provided in the HSE publication 'Health and Safety in Construction (HSG 150)' ([www.hse.gov.uk](http://www.hse.gov.uk)).

### **7.6 Infiltration Potential**

- 7.6.1 Based on infiltration testing undertaken in trial pits TP107 and TP114 at the site (refer to Appendix E), the Devensian Till deposits are considered to be effectively impermeable and would not be able to dispose of water using soakaway systems. Alternative means of storm water disposal will be required. Disposal into Higgin Brook could be an option and we understand that the surface water sewer, which cuts across the south-western part of the site, outfalls into the brook and serves a similar residential development adjacent to the south-east.

## 7.7 Pavement Foundations

7.7.1 It is anticipated that the proposed access road and associated hardstanding areas will be located at or about existing ground levels with formation located on Devensian Till deposits and locally Made Ground.

7.7.2 Equilibrium CBR (California Bearing Ratio) values (with reference to Transport and Road Research Laboratory (TRRL) Report LR1132 '*Structural design of Bituminous Roads*') are derived from knowledge of soil classification data (plasticity index for soils exhibiting cohesion (clay type) and particle size distribution for granular soils), the location of the water table pavement thickness, and weather conditions at the time of construction. It is anticipated that excavations to formation levels will generally encounter cohesive soils. Assuming an average plasticity index of say 30 for cohesive soils, a low water table, a 'thin' pavement the following equilibrium CBR values are derived for varying construction conditions.

Equilibrium CBR values for differing construction conditions		
Poor	Average	Good
CBR = 3%	CBR = 4%	CBR = 4%

Table 7.7.2

7.7.3 It is also possible to derive the 'insitu' CBR value at formation from undrained shear strength data by applying a conversion factor of 23 (refer TRRL laboratory report LR889). Thus adopting pessimistic undrained shear strength of say 50kN/m<sup>2</sup> at formation level (based on insitu shear strength measurements) then an equivalent CBR value can be obtained i.e.

$$\text{Insitu CBR} = \text{undrained shear strength} \frac{50}{23} = 2.17\%$$

7.7.4 The 'insitu' CBR derived above, is susceptible to change dependent upon weather conditions during construction. The equilibrium CBR value derived in paragraph 7.7.2 above is an estimate of the CBR value, which will predominate during the life of the pavement. We recommend the insitu CBR of 2.17% derived from shear strength data be utilised for design purposes and reassessed during construction. The fact that the clay subgrade soils are likely to be deemed frost susceptible will probably be the overriding criteria for pavement foundation design purposes. It should also be noted that the thickness of the pavement foundation also relates to the amount and loading from construction traffic, which is discussed in detail in the Transport and Road Research Laboratory (TRRL) Report LR1132 '*Structural design of Bituminous Roads*'.

7.7.5 The CBR value based on insitu shear strength test data is relatively low and subsequently pavement formation thicknesses will be need to be increased accordingly. Undertaking further insitu testing (maybe using a TRL DCP probe) along proposed access roads and hardstanding may yield an increase in this value, potentially above 3% which would decrease the required formation thickness and provide associated cost savings.

- 7.7.6 Once formation levels have been established it is recommended that the formation be trimmed and rolled following current requirements of the Highways Agency Specification for Highways Works (clause 616) (refer [www.dft.gov.uk/ha/standards/mchw/vol1](http://www.dft.gov.uk/ha/standards/mchw/vol1)). Such a process will identify any soft areas, which we recommend be either excavated out and backfilled with a suitable well compacted material similar to those exposed in the sides of the resulting excavation, or large cobbles of a good quality stone rolled into the formation to stabilise the 'soft' area.
- 7.7.7 The silty nature of the Devensian Till will render them moisture susceptible with small increases in moisture content giving rise to a rapid loss of support to construction plant. We therefore recommend, as soon as formation is trimmed and rolled, that sub-base is laid in order to avoid deterioration of the subgrade in wet or frosty conditions.

## 8 Chemical contamination

8.1	Contaminated land, regulations and liabilities
8.2	Objectives and procedures
8.3	Development characterisation and identified receptors
8.4	Identification of pathways
8.5	Assessment of sources of contamination
8.6	Initial conceptual model
8.7	Laboratory testing
8.8	Updated conceptual model
8.9	Actions
8.10	Risk assessment summary and recommendations
8.11	Statement with respect to National Planning Policy Framework
8.12	On site monitoring

### 8.1 Contaminated land, regulation and liabilities

#### 8.1.1 Statute

8.1.1.1 Part IIA of the Environment Protection Act 1990 became statute in April 2000. The principal feature of this legislation is that the hazards associated with contaminated land should be evaluated in the context of a site-specific risk based framework. More specifically contaminated land is defined as:

*“any land which appears to the Local Authority in whose area it is situated to be in such a condition, by reasons of substances in, on or under the land, that:*

- a) Significant harm is being caused or there is a significant possibility of such harm being caused; or*
- b) Pollution of controlled waters is being or is likely to be caused”.*

8.1.1.2 Central to the investigation of contaminated land and the assessment of risks posed by this land is that:

- i) There must be contaminant(s) at concentrations capable of causing health effects (*Sources*).
- ii) There must be a human or environmental receptor present, or one which makes use of the site periodically (*Receptor*); and
- iii) There must be an exposure pathway by which the receptor comes into contact with the environmental contaminant (*Pathway*).

8.1.1.3 In most cases the Act is regulated by Borough or District Councils and their role is as follows:

- i) Inspect their area to identify contaminated land
- ii) Establish responsibilities for remediation of the land
- iii) See that appropriate remediation takes place through agreement with those responsible, or if not possible:
  - by serving a remediation notice, or
  - in certain cases carrying out the works themselves, or
  - in certain cases by other powers
- iv) keep a public register detailing the regulatory action which they have taken

8.1.1.4 For “special” sites the Environment Agency will take over from the Council as regulator. Special sites typically include:

- Contaminated land which affects controlled water and their quality
- Oil refineries
- Nuclear sites
- Waste management sites

## **8.1.2 Liabilities under the Act**

8.1.2.1 Liability for remediation of contaminated land would be assigned to persons, organisations or businesses if they caused, or knowingly permitted contamination, or if they own or occupy contaminated land in a case where no polluter can be found.

## **8.1.3 Relevance to predevelopment conditions**

8.1.3.1 For current use, Part IIA of the Environmental Protection Act 1990 provides the regulatory regime. The presence of harmful chemicals could provide a ‘source’ in a ‘pollutant linkage’ allowing the regulator (Local Authority or Environment Agency) to determine if there is a significant possibility of harm being caused to humans, buildings or the environment. Under such circumstances the regulator would determine the land as ‘contaminated’ under the provision of the Act requiring the remediation process to be implemented.

## **8.1.4 Relevance to planned development**

8.1.4.1 The developer is responsible for determining whether land is suitable for a particular development or can be made so by remedial action. In particular, the developer should carry out an adequate investigation to inform a risk assessment to determine:

- a) Whether the land in question is already affected by contamination through source – pathway – receptor pollutant linkages and how those linkages are represented in a conceptual model.
- b) Whether the development proposed will create new linkages e.g. new pathways by which existing contaminants might reach existing or proposed receptors and whether it will introduce new vulnerable receptors, and

- c) What action is needed to break those linkages and avoid new ones, deal with any unacceptable risks and enable safe development and future occupancy of the site and neighbouring land?

8.1.4.2 Building control bodies enforce compliance with the Building Regulations. Practical guidance is provided in Approved documents, one of which is Part C, '*Site preparation and resistance to contaminants and moisture*' which seeks to protect the health, safety and welfare of people in and around buildings, and includes requirements for protection against harm from chemical contaminants.

#### **8.1.5 Pollution of controlled waters**

8.1.5.1 Part IIA of the Environment Protection Act 1990, defines pollution of controlled waters as

*'The entry into controlled waters of any poisonous, noxious or polluting matter or any solid waste matter'*

8.1.5.2 Paragraphs A36 and A39 of statutory guidance (DETR 2000) further define the basis on which land may be determined to be contaminated land on the basis of pollution of controlled waters.

*'Before determining that pollution of controlled waters is being, or likely to be, caused, the Local Authority should be satisfied that a substance is continuing to enter controlled waters, or is likely to enter controlled waters. For this purpose, the local authority should regard something as being likely when they judge it more likely than not to occur'*

*'Land should not be designated as contaminated land where:*

- a) *A substance is already present in controlled waters:*
- b) *Entry into controlled waters of that substance from the land has ceased, and*
- c) *It is not likely that further entry will take place.*

*Substances should be regarded as having entered controlled waters where:*

- a) *They are dissolved or suspended in those waters; or*
- b) *If they are immiscible with water, they have direct contact with those waters, or beneath the surface of the waters'*

8.1.5.3 Controlled waters are defined in statute to be:

*'territorial waters which extend seawards for 3 miles, coastal waters, inland freshwaters, that is to say, the waters in any relevant lake or pond or of so much of any relevant river or watercourse as is above the freshwater limit, and groundwaters, that is to say, any waters contained in underground strata.'*



## **8.1.6 Further information**

- 8.1.6.1 The above provides a brief outline as regards current statute and planning controls. Further information can be obtained from the Department for the Environment, Food and Rural Affairs (DEFRA) and their website [www.defra.gov.uk](http://www.defra.gov.uk).

## **8.2 Objectives and procedures**

### **8.2.1 Objectives**

- 8.2.1.1 This report section discusses investigations carried out with respect to chemical contamination issues relating to the site. The investigations were carried out to determine if there are any liabilities with respect to Part IIA of the Environment Protection Act. As stated in Section 2.4.2, the investigation process followed the principles of BS10175: 2011 '*Investigation of potentially contaminated sites – Code of Practice*', with the investigation combining a desk study (preliminary investigation) together with the exploratory and main investigations (refer BS10175: 2011 for an explanation).
- 8.2.1.2 This section of the report produces '*Conceptual models*' based on investigatory data obtained to date. The conceptual model is constructed by identification of *contaminants* and establishment of feasible *pathways* and *receptors*. The conceptual model allows a *risk assessment* to be derived. Depending upon the outcome of the risk assessment it may be necessary to carry out remediation and/or further investigations with a view to eliminating, reducing or refining the risk of harm being caused to identified receptors. If appropriate, our report will provide recommendations in this respect.
- 8.2.1.3 Clearly we must consider the current pre-development condition, establishing risks which may require action to render the site safe to all relevant (current) receptors meeting the requirements of current legislation (Part IIA of the Environmental Protection Act 1990).
- 8.2.1.4 Definition of terms used in the preceding paragraph and subsequent parts of this section of the report are presented in Appendix B.

## 8.2.2 Procedure to assess risks of chemical contamination

8.2.2.1 For the purposes of presenting this section of this report, we have adopted the following sequence in assessing risks associated with chemical contamination.

Table outlining sequence to assess risk associated with chemical contamination		
Conceptual model element	Contributory information	Outcome
Receptor	Development categorisation	Identification of receptors at risk of being harmed Method of analysing test data Criteria for risk assessment modelling
Pathways	Geology and ground conditions Development proposals	Identification of critical pathways from source to receptor
Source	Previous site history Desk study information Site reconnaissance Fieldwork observations	Testing regime Identification of a chemical source Analysis of test data and other evidence

**Table 8.2.2**

8.2.2.2 We have adopted, in general, the procedures described in CIRIA C552 '*Contaminated land risk assessment - a guide to good practice*' in deriving a risk assessment. Initially we have carried out a 'phase 1 assessment' based on desk study information and site reconnaissance, to produce an initial conceptual model and thus a preliminary risk assessment. This model/assessment is then used to target fieldwork activities and laboratory testing, with the results of this part of the investigation used to allow a phase 2 assessment to be produced by updating the conceptual model and refining the risk assessment.

## 8.3 Development characterisation and identified receptors

### 8.3.1 Site characterisation

8.3.1.1 The nature of the site has a significant influence the likely exposure pathways between potentially contaminated soils and potential receptors. The following table summarises elements which characterise the site based on site observations and desk study information.

Summary of site characteristics		
Element	Source/criteria	Characteristic
Current land use	Observations	Site currently in use as grazing land for livestock. Not accessible to the general public.
Future land use	Advice	Residential development which includes domestic gardens.
Site history	Desk study	Recorded as fields from earliest maps.
Geology	Desk study/Site investigation	Topsoil and localised Made Ground overlying >5m thickness of Devensian Till deposits with Bowland Shale Formation/Pendleside Sandstone Member at depth.
Ground water	Aquifer potential	Devensian Till deposits recorded as unproductive strata. Underlying Bowland Shale recorded as a Secondary A and secondary undifferentiated aquifers (r), with Pendleside Sandstone recorded as Secondary A.
	Abstractions	There are no potable water abstractions within 1000m of the site. There are two groundwater abstractions within 1000m of the site, the nearest associated with Mill Farm borehole located 890m south of the site.
	Source protection zone	Site not recorded in source protection zone (SPZ).
Surface waters	Location	The nearest surface water feature is a tertiary river (Higgin Brook) which flows in a north-easterly and north-westerly direction along the north-western boundary of the site.
	Abstractions	There is one surface water abstraction within 1000m of the site located 445m south-east associated with a field drain located in Lyndhurst, Longridge.

**Table 8.3.1**

### 8.3.2 Identified receptors

8.3.2.1 The principal receptors subject to harm caused by any contamination of the proposed development site are as follows.

Principle Receptor	Detail
Humans	Users of the current site
	End user of the developed site
	Construction operatives and other site investigators
Vegetation	Plants and trees, both before and after development
Controlled waters	Surface waters (Rivers, streams, ponds and above ground reservoirs)
	Ground waters (used for abstraction or feeding rivers/streams etc.)
Building materials	Materials in contact with the ground

**Table 8.3.2**

8.3.2.2 This section of the report assesses those receptors listed above. Section 10 provides a risk assessment in relation to building materials.

### **8.3.3 Human receptors**

8.3.3.1 The Contaminated Land Exposure Assessment (CLEA) model can be used to derive guideline values, against which land quality data can be compared to allow an assessment of the likely impacts of soil contamination on humans. The parameters used within the model can be chosen to allow guideline values to be derived for a variety of land uses and exposure pathways. For example, a construction worker is likely to be exposed in different ways and for different durations than an adult in a residential setting.

8.3.3.2 On the basis that the existing site is restricted to agricultural activities the adult is considered an appropriate current receptor. Following completion of the residential development the critical site user (receptor) is considered to be a child under the age of 6 years. These criteria have been used in the conceptual model for the current and future site use. Our assessment also considers construction operatives as adult receptors.

### **8.3.4 Vegetation receptors**

8.3.4.1 Soil contaminants can have an adverse effect on plants if they are present at sufficient concentrations. The effects of phytotoxic contaminations include growth inhibition, interference with natural processes within the plant and nutrient deficiencies.

8.3.4.2 Vegetation is currently present at the site and will remain so following development, in addition to further vegetation proposed as part of the new development. We have therefore considered vegetation a viable receptor.

### **8.3.5 Water receptors**

8.3.5.1 The near surface Devensian Till deposits are recorded as unproductive strata and extend to depths beyond 3.2m at the site. The underlying Bowland Shale Formation is recorded as a Secondary A aquifer. The site is not recorded in a source protection zone. Based on the above, given the thickness of Devensian Till groundwater is not considered a viable receptor. The nearest watercourse to the site is Higgin Brook, which flows along the north-western site boundary. On this basis, surface water is considered to be a viable receptor.

### 8.3.6 Summary of identified receptors

8.3.6.1 Based on the above assessments, the following table summarises identified and critical receptors.

Table summarising identified (viable) receptors				
Principle Receptor	Detail	Viable and critical receptors		
		Viability and justification		Critical receptor
Humans	Users of the current site	Yes	Grazing land	Adult
	End user of the developed site	Yes	Residential	Child
	Construction operatives and other site investigators	Yes		Adult
Vegetation	Current site	Yes	Trees on site	Vegetation
	Developed site	Yes	Trees to remain	Vegetation
Controlled waters	Surface waters (Rivers, streams, ponds and above ground reservoirs)	Yes	Higgin Brook along site boundary	Surface waters
	Ground waters (used for abstraction or feeding rivers/ streams etc.)	No	Devensian Till at crop (impermeable)	Groundwater
Building materials	Materials in contact with the ground	Yes	Assessed in report Section 10	Building materials
Table 8.3.6				

## 8.4 Identification of pathways

### 8.4.1 Pathways to human receptors

8.4.1.1 Guidance published by the Environment Agency in Science Report SC050021/SR3 'Updated technical background to the CLEA model' provides a detailed assessment of pathways and assessment and human exposure rates to source contaminants. In summary, there are three principal pathway groups for a human receptor:

Table summarising likely pathways	
Principal pathways	Detail
Ingestion through the mouth	Ingestion of air-borne dusts
	Ingestion of soil
	Ingestion of soil attached to vegetables
	Ingestion of home grown vegetables
Inhalation through the nose and mouth.	Inhalation of air-borne dusts
	Inhalation of vapours
Absorption through the skin.	Dermal contact with dust
	Dermal contact with soil
Table 8.4.1	

8.4.1.2 The site currently comprises open fields surfaced in grass and used for grazing livestock. It is understood that this has been the principal site use for much of the sites history, if not all. Based on such we have considered all the above pathways would be present for current users with the exception of those associated with the consumption of vegetables.

8.4.1.3 Following redevelopment the site will be occupied by housing with associated gardens and landscaping. Based on such all of the above pathways will be considered viable for proposed site users. A summary of our pathway assessment is presented in Section 8.4.4.

#### **8.4.2 Pathways to vegetation**

8.4.2.1 Guidance published by the Environment Agency in Science Report SC050021/SR (Evaluation of models for predicting plant uptake of chemicals from soil) provides a detailed assessment of plant uptake pathways. In summary, plants are exposed to contaminants in soils by the following pathways:

- Passive and active uptake by roots.
- Gaseous and particulate deposition to above ground shoots.
- Direct contact between soils and plant tissue.

8.4.2.2 All of the above routes of exposure are considered to be present for vegetation.

#### **8.4.3 Pathways to controlled waters**

8.4.3.1 A number of pathways exist for the transport of soil contamination to controlled waters. A summary of these pathways is presented below:

- Percolation of water through contaminated soils
- Near-surface water run-off through contaminated soils
- Saturation of contaminated soils by flood waters

8.4.3.2 Near surface soils comprised cohesive Devensian Till deposits which are considered impermeable and extend to depths beyond 3.2m at the site. The clay soils will severely restrict the percolation of surface water into the underlying aquifer of the Bowland Shale Formation, therefore, pathways associated with percolation of surface water will not be considered further.

8.4.3.3 Based on the permeability of near surface Devensian Till deposits, in our opinion such soils are considered amenable to promoting significant amounts of near surface water run off through contaminated soils.

8.4.3.4 The site is not recorded within a fluvial flood plain and as such saturation of contaminated soils by flood waters is unlikely to occur.

#### 8.4.4 Summary of identified likely pathways

8.4.4.1 Based on the above assessments, the following table summarises likely pathways of potential chemical contaminants at the site to identified receptors.

Table of likely pathways		
Receptor group	Critical receptor	Pathway
Proposed site users	Child	Ingestion of air-borne dusts
		Ingestion of soil
		Ingestion of soil attached to vegetables
		Ingestion of home grown vegetables
		Inhalation air-borne dusts
		Inhalation of vapours
		Dermal contact with dust
		Dermal contact with soil
Current site users and construction operatives	Adult	Ingestion of air-borne dusts
		Ingestion of soil
		Inhalation of air-borne dusts
		Inhalation of vapours
		Dermal contact with dust
Vegetation		Dermal contact with soil
		Root uptake, deposition to shoots and foliage contact
Groundwater	Surface water	Near-surface water run-off through contaminated soils
Table 8.4.4		

## 8.5 Assessment of sources of chemical contamination

### 8.5.1 Introduction

8.5.1.1 Initially, potential sources of contamination are assessed using the following elements of the investigation process.

- History of the site
- Desk study information
- Site reconnaissance
- Geology
- Fieldwork

8.5.1.2 These elements will dictate a relevant soil/water testing regime to quantify possible risks of any identified contaminative sources which may harm identified receptors.

### 8.5.2 Source assessment – History of the site

8.5.2.1 The history of the site and its immediate surroundings based on published Ordnance Survey maps is described in Section 3.



8.5.2.2 Based on published historical maps, there is no evidence to indicate the site has been subject to activities which could produce a source of chemical contamination; however, records indicate that a dairy was located on the adjacent site to the south-west, which could be a potential source of contamination. In addition, a mill, unclassified works and a garage were located to the south and west of the site, although all are in excess of 90m from the subject site. Due to the distance from the site and the relatively impermeable geology, in our opinion, there is unlikely to be a significant risk of contamination migrating from these potential sources to the subject site.

### 8.5.3 Source assessment – Desk study information

8.5.3.1 Envirocheck presents a detailed database of environmental information in relation to the site including;

- Pollution incidents
- Landfill sites
- Trading activities

#### 8.5.3.2 *Pollution incidents*

8.5.3.2.1 Envirocheck report a number of pollution incidents to controlled waters within 2000m of the site, the closest of which are recorded some 60m to the south and 160m to the west. The incident to the south is associated with the release of waste milk into Higgin Brook and is classified as a Category 2 significant incident. The incidents to the west are dated June 1997 and are associated with the release of paint/dyes and inert suspended solids and oils, again into Higgin Brook and are classified as Category 3 minor incidents. Given the distance from the site and the type and severity of the incidents they are considered unlikely to have impacted the site.

#### 8.5.3.3 *Landfill sites*

8.5.3.3.1 Envirocheck reports there are two registered landfill sites within 1km of the site. One is located approximately 520m to the east and was in receipt of inert, non-hazardous and industrial wastes. The second is recorded 800m to the south of the site, with records indicating the site is now dormant and was in receipt of demolition material and uncontaminated soils.

8.5.3.3.2 In addition, we have reviewed old Ordnance Survey maps and there are a small number of quarries recorded between 500m and 1000m from the subject site, predominantly to the east, exploiting the underlying clays and grits.

8.5.3.3.3 Based on the above, due to the distance, the risk of any chemical contamination associated with landfill sites and restored mineral sites in the area, migrating and impacting identified receptors at the site, is considered low.

**8.5.3.4**      *Trading activities*

8.5.3.4.1      Envirocheck reports the closest active trade entry is located 90m to the south-west of the site, associated with a garage (Irelands Ltd). It should also be noted that the site is recoded as a fuel station entry, however, records indicate this is now obsolete. Such activities utilise chemicals which could harm identified receptors, however, due to the distance from the site and the relatively impermeable geology, in our opinion, there is unlikely to be a significant risk of contamination migrating from the garage to the subject site.

**8.5.4**      **Source assessment – Site reconnaissance**

8.5.4.1      A full description of the site and observed adjacent land uses is provided in Section 3 of this report. A plan summarising observations made on site during our site reconnaissance visit is presented on Drawing 02b.

8.5.4.2      We did not observe any obvious evidence of any current or recent activities on site which provide a potential source of chemical contamination.

**8.5.5**      **Source assessment – Geology**

8.5.5.1      The geological map of the area indicates the topography local to the site is formed in deposits of Devensian Till, Bowland Shale Formation and Pendleside Sandstone Formation. Typically, and in our experience, such deposits do not exhibit any abnormal concentrations of naturally occurring chemical contaminants.

**8.5.6**      **Source assessment - Fieldwork observations**

8.5.5.1      Made Ground was encountered in four locations and contained anthropogenic material, which could potentially provide a source of contamination.

**8.5.7**      **Source assessment - summary**

8.5.7.1      Based on the paragraphs above, we have identified the following potential sources of contamination:

Table summarising results of source assessment				
Source	Origin of information	Possible contaminant	Probability of risk occurring	Likely extent of contamination
<b>On site</b>				
Made Ground	Fieldwork	Inorganics & organics	Likely	Localised
Table reference 8.5.7				

## **8.6 Initial Conceptual Model**

- 8.6.1 Based on our assessment of potential contaminative sources, identified receptors and viable pathways to receptors described in preceding paragraphs, we have produced an initial conceptual model in the form of a table which is presented in Appendix I.
- 8.6.2 Based on the conceptual model there are risks which exceed the low category which in our opinion are unacceptable, and require either remedial action or further investigation by laboratory testing of soil/water samples to refine the risk assessment.

## **8.7 Laboratory testing**

### **8.7.1 Testing regime – Human receptors**

- 8.7.1.1 Based on our source assessment (and our initial conceptual model) we have no evidence to identify any past or recent uses of the site which may have generated specific contamination. However, Made Ground was encountered on site in localised areas. Three samples, targeting areas considered to be at risk of potential contaminative sources were scheduled to measure concentrations of Total Petroleum Hydrocarbons (TPH), in addition to BTEX and MTBE, VOCs and SVOCs. Such samples were taken from exploratory excavations local to site boundaries where potential contaminants could have the potential to migrate onto site or from localised Made Ground.
- 8.7.1.2 In addition, twenty one samples of near surface Topsoil/Made Ground were submitted for measurement of organic and inorganic contaminants. The results of laboratory determination of concentration of chemical contaminants are presented in Appendix G.

### **8.7.2 Testing regime – Water receptors**

- 8.7.2.1 We have identified sources of chemical contamination onsite associated with Made Ground and as a result have selected four samples of Topsoil/Made Ground for the measurement of commonly occurring leachable inorganic and organic contaminants where they are considered a risk to water resources. This in our opinion is an absolute minimum to assist in the risk assessment. Further laboratory testing would increase the accuracy of the risk assessment.

### **8.7.3 Testing regime – Vegetation**

- 8.7.3.1 As described in 8.7.1 above we have scheduled testing for a suite of commonly occurring inorganic and organic contaminants. With reference to '*BPG Note 5 - Best Practice Guidance for Land Regeneration*' testing will include copper and zinc.

## 8.7.4 Scheduled testing

8.7.4.1 The following table summarises the scheduled testing, in relation to soil types and identified receptors under consideration of the conceptual model.

Table summarising scheduled testing						
Sample origin	Sample type	Strata	Targeted sampling	Non targeted sampling	Scheduled testing	Critical receptor
TP101 0.3m TP103 0.1m TP103 0.3m TP108 0.1m TP108 0.5m TP125 0.1m	Soil	Made Ground		✓	Inorganic & organics	All human receptors
TP102 0.2m TP104 0.1m TP106 0.1m TP107 0.1m TP110 0.1m TP112 0.1m TP113 0.1m TP114 0.1m TP116 0.1m TP117 0.1m TP118 0.1m TP119 0.1m TP120 0.1m TP123 0.1m TP124 0.1m	Soil	Topsoil		✓	Inorganic & organics	
TP101 0.9m TP108 0.5m TP125 0.5m	Soil	Made Ground	✓		TPH, VOCs and SVOCs	
TP102 0.2m TP110 0.1m TP114 0.1m	Soil	Topsoil		✓	Leachable inorganics & organics	Water receptors
TP125 0.5m	Soil	Made Ground		✓		

**Table 8.7.4.1**

## 8.7.5 Criteria for assessment of test data – Human receptors

8.7.5.1 Assessment of laboratory test data has been carried out with reference to current nationally recognised documents listed in the final page of Appendix B. Due to changes in guidance on contaminated land, items 6-8 and item 10 in the document listing above have been withdrawn. In the absence of alternative guidance however we have used these documents. Where new guidance is available, this has been followed in preference to superseded guidance.

- 8.7.5.2 Sutable 4 Use Levels (S4ULs) are used as a screening tool to assess the risks posed to health of humans from exposure to soil contamination in relation to land uses. Where published S4ULs are not available, we have adopted Category 4 Screening Levels (C4SLs) where appropriate, derived by DEFRA, and Soil Screening Values (SSV) derived by Soiltechnics and by Atkins (SSV<sup>ATK</sup>). The S4ULs have been derived by Land Quality Management (LQM) and the Chartered Institute of Environmental Health (CIEH) and presented in *'The LQM/CIEH S4ULs for Human Health Risk Assessment'*. They are derived in accordance with UK legislation, national as well as Environment Agency (EA) Policy using a modified version of the EA CLEA model and other available guidance. The S4ULs have been prepared for a number of metals and polycyclic aromatic hydrocarbons (PAH) and are used in preference to C4SLs and values produced by Soiltechnics and Atkins. The CLEA model has been used with toxicology data presented by the EA, LQM/CIEH and Atkins (in that order of preference) to derive SSVs by Soiltechnics. SSVs produced by Atkins are presented on their ATRISK<sup>SOIL</sup> website.
- 8.7.5.3 S4ULs, C4SLs, SSVs and SSV<sup>ATK</sup>s represent 'intervention values'; indications to an assessor that soil concentrations above these levels might present an unacceptable risk to the health of site users. These soil guideline values have been produced using conceptual exposure models, which use assumptions and are applied to differing end uses of land. If the values are exceeded, it does not necessarily imply there is an actual risk to health and site-specific circumstances should be taken into account. Conversely, where a critical pathway or chemical form of the contaminant has not been evaluated, a risk may be present even if the guideline has not been exceeded.
- 8.7.5.4 For evaluation of test data in relation to polycyclic aromatic hydrocarbon (PAH) contamination, we have compared measured concentrations with corresponding S4ULs. The S4ULs for PAHs are dependent on the Soil Organic Matter (SOM) content of the soils.
- 8.7.5.5 For evaluation of total petroleum hydrocarbon (TPH), BTEX and VOC/SVOC related contamination we have compared measured concentrations directly to the relevant S4ULs where available. Alternatively, we have assumed a possible risk if concentrations are above detectable limits.
- 8.7.5.6 We have followed procedures outlined by the CIEH to compare measured concentrations of metals and PAH contaminants against guideline values. TPH, SVOC and VOC related contaminants are compared directly with the relevant guideline values. The guidance presents an approach to data analysis and includes the examination of data for potential outliers, assessment of the normality of the test data and the calculation of a 95% Upper Confidence Limit (UCL). The UCL provides an estimate of the population mean, based on test data, with a 95% confidence that the actual mean does not exceed this value. The UCL is compared to the guideline value for the site.

8.7.5.7 We have adopted a conservative approach for current site users and compared measured concentrations of contaminants against guideline values for residential without plant uptake land use. For end users we have compared measured test data against guideline values presented for residential with plant uptake land use. In the absence of guidelines we have adopted commercial guideline values for assessment of construction operatives.

#### **8.7.6 Criteria for assessment of test data – Vegetation**

8.7.6.1 Guidance published by Forest Research in “BPG Note 5 - Best Practice Guidance for Land Regeneration” suggests that a residential without plant uptake or industrial/commercial CLEA model should be adopted for this receptor although specific guideline values are provided for copper and zinc at 130mg/kg and 300mg/kg respectively. As a practice we have adopted the industrial/commercial CLEA model for assessment of test data for vegetation.

8.7.6.2 It is difficult to quantify the phytotoxicity of a contaminant as large variations exist between plant tolerances, soil effects and synergistic/antagonistic reactions between chemicals. Due to the complexities of the effects of soil contamination on different plant species, we recommend that the test results presented in this report are passed to a landscape architect for the selection of suitable planting.

#### **8.7.7 Criteria for assessment of test data – Controlled waters**

8.7.7.1 For interpretation of test data in relation to water receptors we have directly compared measured values with the Environmental Quality Standards (EQS) and UK Drinking Water Standards (UKDWS). In the absence of EQS or UKDWS we have adopted World Health Organisation Drinking Water Guidelines (WHODWG).

8.7.7.2 EQS values are published by the Environment Agency in their publication, “Environment Agency technical advice to third parties on Pollution of Controlled Waters for Part 11A of the Environmental Protection Act 1990”. EQS values for most inorganic contaminants in freshwater are dictated by the hardness of the receiving watercourse. The hardness of water is a measure of the concentration of calcium carbonate in the water. Although we have not sampled water from nearby watercourses, we have reviewed information supplied by the Drinking Water Inspectorate website, which indicates a hardness of <100mg/l for drinking water in the local area. Although not an insitu groundwater measurement, such results are likely to be similar to those that would be measured in groundwater in the local area.

8.7.7.3 Using this information for List II substances (DOE Circular 7/89) we have compared the measured values with the EQS values relative to the hardness of the receiving watercourse assuming a worst case scenario of the watercourse supporting ‘sensitive’ aquatic life.

8.7.7.4 UKDWS are presented in the Water Supply (Water Quality) Regulations. We have adopted EQS values in preference to alternative guidelines where possible.

## 8.7.8 Evaluation of test data – Human receptors

8.7.8.1 Tables summarising and analysing test data are presented in Appendix H. The following table summarises the outcome of the analyses.

Table Summarising assessment of test data for Human receptors						
Analysis tables	Receptor group	Critical receptor	CLEA model	Inorganic contaminants	Organic contaminants (PAH)	Organic contaminants (TPH)
1 and 2	Current site users	Adult	Residential without plant uptake	No exceedances	No exceedances	n/a
3 and 4	Future site users	Child	Residential with plant uptake	No exceedances	No exceedances	n/a
5 and 6	Construction operatives	Adult	Industrial/commercial	No exceedances	No exceedances	n/a
8	Future site users	Child	Residential with plant uptake	n/a	n/a	No exceedances

**Table 8.7.8.1**

8.7.8.2 Based on the above, laboratory testing has not identified any measured concentrations of commonly occurring inorganic and organic contaminants which exceed current guideline values for human receptors. It should also be noted that all measured concentrations of VOCs and SVOCs have been recorded below detectable limits.

## 8.7.9 Evaluation of test data – Vegetation

8.7.9.1 Comparison of test data with guideline values is presented on Tables 6 and 7 in Appendix H. None of the measured concentrations exceed the adopted guideline values. On this basis, we are of the opinion that measured concentrations are unlikely to exhibit significant contamination with respect to vegetation.

8.7.9.2 It is difficult to quantify the phytotoxicity of a contaminant as large variations exist between plant tolerances, soil effects and synergistic/antagonistic reactions between chemicals. Due to the complexities of the effects of soil contamination on different plant species, we recommend that the test results presented in this report are passed to a landscape architect for the selection of suitable planting.

## 8.7.10 Evaluation of test data – Controlled waters

### 8.7.10.1 Inorganic contaminants

8.7.10.1.1 The measured values of inorganic contaminants fall well below relevant guideline values with the exception of copper. Out of the four samples of Topsoil/Made Ground tested across the site three exceed the EQS value of 6µg/l, with concentrations measured at concentrations of 6µg/l and 13µg/l and recorded in Topsoil deposits only. The remaining sample of Made Ground exhibited a concentration of 5.7 µg/l, which is marginally below the guideline value.

8.7.10.2 *Organic contaminants (polycyclic aromatic hydrocarbons)*

8.7.10.2.1 For the analysis of PAH contamination, the sum of the following contaminants has been compared to a UKDWS.

- Benzo(b)fluoranthene
- Benzo(k)fluoranthene
- Benzo(ghi)perylene
- Indeno(1,2,3-cd)pyrene

8.7.10.2.2 The summed concentration of the PAH 'suite' for each samples do not exceed the UKDWS. In addition the leachable concentration of benzo(a)pyrene and naphthalene do not exceed their respective guideline values.

## 8.8 Updated conceptual model

### 8.8.1 Human receptors

8.8.1.1 Based on the above, laboratory testing has not identified any measured concentrations of chemical contaminants which exceed current guideline values for identified human receptors. Based on the above evaluation, the concentrations of contaminants measured on soil samples taken from the site are considered unlikely to exhibit significant contamination from a perspective of human receptors.

### 8.8.2 Water receptors

8.8.2.1 Laboratory testing has identified elevated concentrations of leachable copper in three samples of Topsoil tested across the site. Given the limited historic use of the site (fields from earliest historical maps until present) it is most likely to be attributed to the use of copper based fertilisers in agriculture. However, if this is the source total concentrations of copper would also be expected to be present to some extent in Topsoil across the site and certainly at higher concentrations than those measured to date. The underlying naturally deposited Devensian Till has not been noted to contain gravels which could provide a potential source of copper.

8.8.2.2 The EQS values used in the assessment are largely dictated by the hardness of the receiving watercourse and we have been fairly conservative with the hardness value adopted for the site based on readily available groundwater data. It is likely that if water was tested within Higgin Brook (receiving surface watercourse) that hardness values would be higher than those adopted (>200mg/l rather than <100mg/l) which would have the effect of increasing the EQS value of copper from 6µg/l to 28µg/l. If this were to be the case then the concentrations of leachable copper measured in Topsoil deposits would fall below the guideline value for the site.

8.8.2.3 Based on the above, we are of the opinion that the concentrations of leachable copper recorded in Topsoil at the site are unlikely to have an adverse effect on surface waters in the area. However, as a precaution we recommend that surface waters within Higgin Brook are tested to determine site specific values of hardness which will enable a more detailed risk assessment to be completed.



### **8.8.3 Summary**

8.8.3.1 Having now completed analysis of laboratory testing, we can now update our conceptual model which is presented in Appendix I.

8.8.3.2 Based on the updated conceptual model, with the exception of determining hardness values of surface waters in Higgin Brook in relation to further assessment of leachable copper, none of the assessed risks exceed the low category and on this basis remedial action is not considered necessary at this stage to render the site fit for purpose. Sources that have not been identified by laboratory testing have been removed from the conceptual model.

## **8.9 Actions**

8.9.1 Based on the above our sole recommendations are as follows:-

- Hardness values within surface waters of Higgin Brook are determined to enable a more detailed risk assessment to be completed in relation to concentrations of leachable copper affecting water receptors
- Construction operatives adopt adequate hygiene precautions

## **8.10 Risk assessment summary and recommendations**

8.10.1 Based on our assessments described above, we can provide the following summary and recommendations for each identified receptor.

### **8.10.2 Current and proposed site users**

8.10.2.1 As no source of significant chemical contamination has been identified on site, we are of the opinion that the site represents a very low risk of causing harm to the health of identified current users of the site.

### **8.10.3 Construction operatives and other site investigators**

8.10.3.1 The risk of damage to health of construction operatives and other site investigators is, in our opinion, low. As a precautionary approach, however, we recommend adequate hygiene precautions are adopted on site. Such precautions would be:-

- Wearing protective clothing particularly gloves to minimise ingestion from soil contaminated hands.
- Avoiding dust by dampening the soils during the works.
- Wearing masks if processing produce dust.

8.10.3.2 Guidance on safe working practices can be obtained from the following documents

- The Health and Safety Executive Publication *“Protection of Workers and the General Public during the Development of Contaminated Land”* (HMSO) and
- *“A Guide to Safer Working on Contaminated Sites”* (CIRIA Report 132).

8.10.3.3 In addition, reference should be made to the Health and Safety Executive. In all cases work shall be undertaken following the requirements of the Health and Safety at Work Act 1974 and regulations made under the Act including the COSHH regulations.

#### **8.10.4 Controlled waters**

8.10.4.1 Based on the risk assessment outlined in Section 8.8.2 above, we are of the opinion that the site currently represents a low-moderate risk of causing harm to water receptors, and as a precaution we recommend that values of hardness are determined in surface waters of Higgin Brook to enable a more detailed risk assessment to be undertaken in relation to concentrations of leachable copper.

#### **8.10.5 Vegetation**

8.10.5.1 As no source of significant chemical contamination has been identified on site, we are of the opinion that the site represents a low risk of causing harm to vegetation.

### **8.11 Statement with respect to National Planning Policy Framework**

8.11.1 Based on investigations completed to date with respect to chemical contamination, we are of the opinion the proposed development will be safe and suitable for use for the purpose for which it is intended (without the need for any remedial action) thus meeting the requirements of the National Planning Policy Framework section 121, and compliant with the Building Regulations Part C, *'Site preparation and resistance to contaminants and moisture'*.

### **8.12 On Site Monitoring**

8.12.1 We have attempted to identify the potential for chemical contamination on the site, however, areas, which have not been investigated at this stage, may exhibit higher levels of contamination. If such areas are exposed at any time during construction we will be pleased to re-attend site to assess what action is required to allow the development of safely proceed.

## 9 Gaseous contamination

9.1	Legislative framework
9.2	General
9.3	Assessment of source of gases
9.4	Gas migration
9.5	Conclusion
9.6	Statement with respect to National Planning Policy Framework

### 9.1 Legislative framework

- 9.1.1 There is currently a complex mix of documentation relating to legislative and regulatory procedures on the issue of contamination and it is not considered a purpose of this report to discuss the detail of these regulations. Essentially, Government Policy is based on '*suitable for use approach*', which is relevant to both the current and proposed future use of land. For current use Part IIA of the Environmental Protection Act 1990 provides the regulatory regime (see Section 8.1). The presence of harmful soil gases could provide a 'source' in a 'pollutant linkage' allowing the regulator (Local Authority) to determine if there is a significant possibility of harm being caused to humans, buildings or the environment. Under such circumstances the regulator would determine the land as 'contaminated' under the provision of the Act requiring the remediation process to be implemented with the Environment Agency responsible for enforcement.
- 9.1.2 The Town and Country Planning (General Development Procedure) Order 1995, requires the planning authority to consult with the Environment Agency before granting planning permission for development on land within 250 metres of land which is being used for deposit of waste, (or has been at any time in the last 30 years) or has been notified to the planning authority for the purposes of that provision.
- 9.1.3 Building control bodies enforce compliance with the Building Regulations. Practical guidance is provided in Approved documents, one of which is Part C, '*Site preparation and resistance to contaminants and moisture*' which seeks to protect the health, safety and welfare of people in and around buildings and includes requirements for protection against harm from soil gas.

### 9.2 General

- 9.2.1 The following assessment relates to the potential for, and the effects of, gases generated by biodegradable matter. The potential for the development to be affected by radon gas is considered in Section 3. The principal ground gases are carbon dioxide (CO<sub>2</sub>) and methane (CH<sub>4</sub>). The following table provides a summary of the effects of these gases when mixed with air.

Significant gas concentrations in air		
Gas	Concentration by volume	Consequence
Methane	0.25%	Ventilation required in confined spaces
	5 - 15%	Potentially explosive when mixed with air
	30%	Asphyxiation
	75%	Death after 10 minutes
Carbon Dioxide	0.5%	8 hour long term exposure limit (LTTEL) (HSE workplace limit)
	1.5%	15 min short term exposure limit (STEL) (HSE workplace limit)
	>3%	Breathing difficulties
	6 – 11%	Visual distortion, headaches, loss of consciousness, possible death
	>22%	Death likely to occur

**Table 9.2.1**

9.2.2 Following the current Building Regulations Approved Document C1, Section 2 'Resistance to Contaminants' (2004 incorporating 2010 and 2013 amendments) a risk assessment approach is required in relation to gaseous contamination based on the source-pathway-receptor conceptual model procedure. We have adopted procedures described in the following reference documents for investigation and assessments of risk of the development being affected by landfill type gases (permanent gases) and if appropriate the identification of mitigation measures.

- BS10175:2011 'Investigation of potentially contaminated sites- Code of Practice'
- BS8576:2013 'Guidance on investigations for ground gas – Permanent gases and Volatile Organic Compounds (VOCs)'
- BS8485:2015 'Code of practice for the design of protective measures for methane and carbon dioxide ground gases for new buildings'
- CIRIA Report C665 'Assessing risks posed by hazardous ground gases to buildings' (2007)
- NHBC report No 10627-R01(04) 'Guidance on development proposals on sites where methane and carbon dioxide are present' (January 2007)
- CL:AIRE Research Bulletin RB17 'A pragmatic approach to ground gas risk assessment' (November 2012)

9.2.3 Whilst we have followed the guidance and recommendations of BS8576, we have used BS8485:2015 to derive recommendations for protective works, and where considered necessary supplemented by NHBC report No 10627-R01(04).

9.2.4 An assessment of the risk of the site being affected by ground gases is based on the following aspects:

- a) Source of the gas
- b) Investigation information
- c) Migration feasibility
- d) Sensitivity of the development and its location relative to the source

## 9.3 Assessment of source of gases

### 9.3.1 General sources

9.3.1.1 The following table summarises the common sources of ground gases and parameters affecting the generation of ground gases:

Source and control of gases	
Type	Parameters affecting the rate of gassing
Landfills	Portion of biodegradable material, rate reduces with time
Mineworkings	Flooding reduces rate of gassing
Dock silt	Portion of organic matter
Carbonate deposits	Ground/rainwater (acidic) reacts with some carbonates to produce carbon dioxide.
Made Ground	Thickness of Made Ground and proportion of degradable organic matter.
Naturally deposited soils/rocks	Thickness of Made Ground and proportion of degradable organic matter.
Table 9.3.1	

9.3.1.2 The rate of decomposition in gas production is also related to atmospheric conditions, pH, temperature, and water content/infiltration.

9.3.1.3 As the site is not within a dockland environment or an area affected by mineworkings, and near surface soils do not exhibit high carbonate content, then potential gas sources are limited to landfills and/or soils with a high proportion of organic matter.

### 9.3.2 Landfill and infilled ground sources

9.3.2.1 Waste Management Paper 27 (1991) produced by the Department of the Environment 'Control of Landfill Gases' contains the recommendation to avoid building within 50m of a landfill site actively producing large quantities of landfill type gases and to carry out site investigations within a zone 250m beyond the boundary of a landfill site. No distinction is made between sites of differing ground conditions, but the paper does not advocate the site is safe beyond the 250m zone, dependant, of course, upon the type of landfill and potential for migration of landfill gases.

9.3.2.2 Within a 2km radius of the site, there are no BGS recorded or historical landfill sites; however, there are two registered landfill sites. Lords Delph (Forty Acre Lane, Longridge) is located approximately 520m to the east of the site and has been accepting non-biodegradable waste since at least 1982. Chapel Hill Quarry is located approximately 800m to the south of the site and accepted non-biodegradable waste; in 1992, the site was recorded as dormant.

9.3.2.3 In addition, we have reviewed old Ordnance Survey maps there are a small number of quarries recorded between 500m and 1000m from the subject site, predominantly to the east. The geological map of the area indicates areas of infilled ground which approximately coincide with such areas.

- 9.3.2.4 Due to the distance of the sites from the subject site and the nature of the waste, in our opinion they are considered very unlikely to represent potential sources of ground gases which could affect the subject site. Furthermore, a series of small ponds are noted to have been recorded onsite and possibly filled in recent years. However, given the limited size of the water features it is considered unlikely that any gases associated with organic/putrescible material contained within would have the potential to affect identified receptors.

### 9.3.3 Soil conditions

- 9.3.3.1 None of the soils observed in exploratory excavations, in our opinion, exhibit significant concentrations of organic matter which are likely to produce elevated quantities of carbon dioxide and / or methane gas.
- 9.3.3.2 Based on an assessment of 'deep' geological conditions we are of the opinion that it is unlikely that the subject site would be affected by significant quantities of carbon dioxide and methane generated by soils/rocks at depth.
- 9.3.3.3 Based on the presence of extensive deposits of cohesive and impermeable Devensian Till in the local area, any potential migration of landfill type gases which may be generated at the sources outlined in Section 9.3.2 would also be severely restricted and unlikely to feasibly migrate to the subject site. We can confirm that we have consulted with Ribble Valley Borough Council with regards to this matter and they have agreed with such assessments. A copy of their correspondence is presented in Appendix K.

### 9.3.4 Source assessment summary

- 9.3.4.1 The following table summarises the possibility of a source of landfill type gases.

Source assessment summary		
Potential source origin	Viability of source	Evidence
Landfills	Unlikely	Desk study information
Mineworkings	Unlikely	Desk Study information Geological conditions not amenable
Dock silt	Unlikely	Site remote from dockland environment
Carbonate deposits	Unlikely	Recorded and observed soil conditions do not indicate high concentrations of carbonates
Made Ground	Unlikely	None present at thicknesses and compositions which would give cause for concern
Naturally deposited soils/rocks	Unlikely	Soils exposed in exploratory excavations do not exhibit high concentrations of organic matter
Table 9.3.4		

## **9.4 Conclusion**

- 9.4.1 Based on the above there is no evidence to demonstrate that there is a potential source rendering the site at a significant risk of being affected by ground gases (carbon dioxide / methane) sufficient to cause significant harm to human end users of the site, construction operatives or indeed buildings. On this basis, it is not considered necessary to consider possible pathways for migration of ground gases, and indeed implementation of further investigations to measure concentrations of ground gases. Again on the basis of evidence provided above, mitigation measures against ingress of ground gases into the proposed development are not considered necessary.

## **9.5 Statement with respect to National Planning Policy Framework**

- 9.5.1 Based on investigations completed to date with respect to gaseous contamination, we are of the opinion the proposed development will be safe and suitable for use for the purpose for which it is intended (without the need for any remedial action) thus meeting the requirements of the National Planning Policy Framework section 121, and compliant with the Building Regulations Part C, '*Site preparation and resistance to contaminants and moisture*'.

## 10 Effects of ground conditions on building materials

10.1	General
10.2	Reference documents
10.3	Hazard identification and assessment
10.4	Provision of test data to specifiers/manufacturers/installers
10.5	Risk assessments for individual building materials
10.6	Concrete – general mechanisms of attack
10.7	Concrete – sulphate attack
10.8	Concrete – chloride attack
10.9	Concrete – acid attack
10.10	Concrete – magnesium attack
10.11	Concrete – ammonium attack
10.12	Concrete blocks
10.13	Clay bricks/pipes
10.14	Mortar
10.15	Metals – general
10.16	Metals – cast iron
10.17	Metals – steel piles
10.18	Metals – stainless steel
10.19	Metals – galvanised steel
10.20	Metals – copper
10.21	Metals – lead
10.22	Plastics – general
10.23	Plastic membranes and geotextiles
10.24	Plastic pipes
10.25	Electrical cables
10.26	Rubbers

### 10.1 General

10.1.1 Building materials are often subjected to aggressive environments which cause them to undergo chemical or physical changes. These changes may result in loss of strength or other properties that may put at risk their structural integrity or ability to perform to design requirements. Aggressive conditions include:

- Severe climates
- Coastal conditions
- Polluted atmospheres
- Aggressive ground conditions

10.1.2 This report section only considers aggressive ground conditions, with other items considered outside our brief and scope of investigations.



10.1.3 In aggressive ground conditions, the potential for contaminant attack depends on the following:

- The presence of water as a carrier of chemical contaminants, (except free phase organic contamination).
- The availability of the contaminant in terms of solubility, concentration and replenishment rate.
- Contact between the contaminant and the building material.
- The nature of the building materials and its capability of being attacked by contaminants.

10.1.4 In general the thicker the building material the less likelihood there is for contaminant attack to cause damage to the integrity of the structure.

## 10.2 Reference documents

10.2.1 Following the Environment Agency publication '*Model Procedures for the Management of Land Contamination*' (Contaminated Land Report 11) the following documents have been referred to in production of the following report paragraphs.

- '*Performance of Building Materials in Contaminated Land*' report BR255 (Building Research Establishment 1994).
- '*Risks of Contaminated Land to Buildings, Building Materials and Services. A Literature Review*' - Technical Report P331 (Environment Agency 2000).
- '*Guidance on assessing and managing risks to buildings from land contamination*' - Technical Report P5 035/TR/01).
- Building Regulations Approved document C - site preparation and resistance to contaminants and moisture (Office of the Deputy Prime Minister, 2004).
- '*Concrete in aggressive ground*' Special Digest 1: 2005 (Building Research Establishment).

## 10.3 Hazard identification and assessment

10.3.1 The identification of hazards is based on the findings of this investigation primarily relating to former land uses (potential for chemical contamination, and likely type of contamination) and laboratory determination of concentration of chemical contaminants. Clearly, the scope of laboratory testing is determined with respect to former land uses, contaminants which may cause harm to human health and water resources.

10.3.2 Based on the above, the scope of our testing regime is described in Section 8. We have utilised this test data in production of the following risk assessments in relation to building materials, in conjunction with test data targeting the effects of chemical attack on concrete in contact with the ground, as described in BRE Special Digest 1.

10.3.3 The identification of hazards from contamination and subsequent assessment of risks is based on the following:

- The contaminants present on site.
- The nature of the contaminant (i.e. calcium sulphate is much less soluble than sodium or magnesium sulphate and is, therefore, less of a concern with regards sulphate attack).
- The concentration of contaminants - in general the higher the concentration the greater the hazard.
- The solubility of the contaminants - contaminants which are not soluble will not generally react with materials.
- The permeability of the soils - i.e. ease by which fluids can transport contaminants to the building.

10.3.4 The process of risk assessment for building materials is concerned with identification of the hazard (contaminants at the site - a source) and subsequently how the contaminants can reach the building (pathway) and how they can react with the building (receptor). Thus the risk assessment is produced based on the source - pathway - receptor model.

## **10.4 Provision of test data to specifiers/manufacturer/installer**

10.4.1 The following risk assessments are based on current published data. We strongly recommend, however, that information gained from this investigation are provided to specifiers/manufacturers/installers of building materials/service ducts/apparatus who may have more up to date research to confirm the ability of the product to resist the effects of chemical contaminants at the site for the desired lifespan of the product.

## **10.5 Risks assessments for individual building materials**

10.5.1 The following/typical sections contain risk assessments for various building materials likely to be incorporated in developments. Other materials which we are not aware of may also be used in developments and in contact with the ground and, therefore, recommend the suppliers are consulted with respect to ground conditions at this site and their opinion sought as to the ability of the product to resist chemical conditions determined at the site.

## **10.6 Concrete - General mechanisms of attack**

10.6.1 There are a number of mechanisms by which contaminants attack concrete including the following:

- Hydrolysis of the hardened concrete.
- Degradation as a result of exchange reactions between calcium in calcium hydroxide (free lime hydrate) and ions in aggressive solutions.
- Expansive reactions as a result of chemical reaction or salt crystallisation.

## 10.7 Concrete - Sulphate attack

### 10.7.1 Hazard

10.7.1.1 Sulphate attack on concrete is characterised by expansion, leading to loss of strength, cracking, spalling and eventual disintegration. There are three principal forms of sulphate attack, as follows:

- Formation of gypsum through reaction of calcium hydroxide and sulphate ions.
- Ettringite formation through reaction of tricalcium alluminate and sulphite ions.
- Thaumasite formation as a result of reactions between calcium silicate hydrates, carbonate ions (from aggregates) and sulphate ions.

### 10.7.2 Assessment

10.7.2.1 The hazard of sulphide attack is addressed by reference to procedures described in Building Research Establishment (BRE) Special Digest 1: 2005 '*Concrete in Aggressive Ground*' to establish a design sulphate class (DS) and the '*Aggressive Chemical Environment for Concrete*' (ACEC). These procedures have been followed during our investigation and are described in the following paragraphs.

### 10.7.3 Desk Study Information

10.7.3.1 The first step in the procedure is to consider specific elements of the desk study. These are tabulated below.

Summary of desk study information			
Element	Interrogation	Outcome	SD1: 2005 reference
Geology	Likelihood of soils containing pyrites	<b>Unlikely</b>	Box C6
Past industrial uses	Brownfield site?	<b>No</b>	C2.1.2

**Table 10.7.3**

10.7.3.2 A brownfield site is defined in SD1: 2005 as a site, or part of a site which has been subject to industrial development, storage of chemicals (including for agricultural use) or deposition of waste, and which may contain aggressive chemicals in residual surface materials, or in ground penetrated by leachates. Where the history of the site is not known, it should be treated as brownfield until there is evidence to classify it as natural.

10.7.3.3 Based on the above it is necessary to follow the procedures described in figure C4 ('*natural ground sites except where soils may contain pyrite*').

#### **10.7.4 Assessment of Design Sulphate Class**

10.7.4.1 The sulphate concentration in a 2:1 water/soil extract was measured in one sample of Made Ground and seven samples of Devensian Till. The mean of the two highest values has been calculated as the characteristic value (refer to table 10.7.7) for Till, with the measured test result used for Made Ground.

#### **10.7.5 Assessment of groundwater mobility**

10.7.5.1 With reference to SD1: 2005, Section C3.1, we are of the opinion that soils at the site generally have a low permeability and thus 'static' groundwater conditions are considered characteristic of the site.

#### **10.7.6 Assessment of pH**

10.7.6.1 Following SD1: 2005, Section C5.1.1 (step 4) the characteristic value for pH within Devensian Till is 7.75, derived by taking the mean of the lowest 2 of the pH results. The characteristic value for pH within Made Ground relates to the measured value of 6.5.

#### **10.7.7 Assessment of aggressive chemical environment for concrete (ACEC)**

10.7.7.1 Based on the design sulphate class, characteristic value of pH and assessment of groundwater mobility, and with reference to table C1 of SDI: 2005, the ACEC class for each soil type is presented in Table 10.7.2 below.

<b>Summary of concrete classification</b>						
<b>Soil type</b>	<b>No. of samples</b>	<b>Characteristic pH</b>	<b>Groundwater mobility</b>	<b>Characteristic sulphate (mg/l)</b>	<b>DS class</b>	<b>ACEC class</b>
Made Ground	1	6.5	Static	10	DS-1	AC-1s
Devensian Till	7	7.75	Static	10	DS-1	AC-1s

**Table reference 10.7.7**

### **10.8 Concrete - Chloride attack**

#### **10.8.1 Hazards**

10.8.1.1 There are a number of ways in which chlorides can react with hydrated cement compounds in concrete. These are as follows:

- Chlorides react with calcium hydroxide in the cement binder to form soluble calcium chloride. This reaction increases the permeability of the concrete reducing its durability.
- Calcium and magnesium chlorides can react with calcium aluminate hydrates to form chloroaluminates which result in low to medium expansion of the concrete.

- If concrete is subject to wetting and drying cycles caused by groundwater fluctuations, salt crystallisation can form in concrete pores. If pressure produced by crystal growth is greater than the tensile strength of the concrete, the concrete will crack and eventually disintegrate.

#### **10.8.2 Risk assessment**

10.8.2.1 Chlorides of sodium, potassium, and calcium are generally regarded as being non-aggressive towards mass concrete; indeed brine containers used in salt mines have been known to be serviceable after 20 years' service. Depending upon the type of concrete, and the cement used up to 0.4% chloride is allowed in BS8110: Part 1.

10.8.2.2 In view of the past use of the site we consider the likelihood of elevated concentrations of chlorides in the ground to be low and on this basis have not specifically measured concentrations of chlorides.

### **10.9 Concrete - Acid attack**

#### **10.9.1 Hazards**

10.9.1.1 Concrete being an alkaline material is vulnerable to attack by acids. Prolonged exposure of concrete structures to acidic solutions can result in complete disintegration.

#### **10.9.2 Risk assessment**

10.9.2.1 The rate of acid attack on concrete depends upon the following:

- The type of acid
- The acid concentration (pH)
- The composition of the concrete (cement/aggregate)
- The soil permeability
- Groundwater movement

10.9.2.2 British Standard BS8110: Part 1 classifies extreme environment as one where concrete is exposed to flowing groundwater that has a pH<4.5. The standard also warns that Portland Cement is not suitable for acidic conditions with a pH of 5.5 or lower.

10.9.2.3 The pH of the soil/groundwater was measured exceeding 5.5 and on this basis the risk of concrete being affected by acidic conditions is considered low.

## **10.10 Concrete - Magnesium attack**

### **10.10.1 Hazards**

- 10.10.1.1 Magnesium salts (excepting magnesium hydrogen carbonate) are destructive to concrete. Corrosion of concrete occurs from cation exchange reactions where calcium in the cement paste hydrates and is replaced with magnesium. The cement loses binding power and eventually the concrete disintegrates.

### **10.10.2 Risk assessment**

- 10.10.2.1 In practise 'high' concentrations of magnesium will be found in the UK only in ground having industrial residues. Following BRE Special Digest 1:2005, measurement of the concentration of magnesium is recommended if sulphate concentrations in water extract or groundwater exceed 3000mg/l. Once measured the concentration of magnesium is considered further in BRE Special Digest in establishing the concrete mix to resist chemical attack.
- 10.10.2.2 We are not aware the site has been subject to any manufacturing processes which would have included magnesium containing compounds, and in addition sulphate concentrations did not exceed 3000mg/l, on this basis we have not measured the concentration of magnesium in soils at the site, and would consider the risk of soils at the site promoting attack on concrete to be low.
- 10.10.2.3 BS EN 206-1:2000 '*Concrete - Part 1: Specification, performance, production and conformity*' does, however, provide exposure classes for concrete in contact with water, with varying concentrations of magnesium for the design/specification for concrete mixes. No groundwater was encountered by the investigation and we would consider the risk of magnesium requiring special consideration with respect to enhancement of exposure class for this contaminant in isolation to be low.

## **10.11 Concrete - Ammonium attack**

### **10.11.1 Hazards**

- 10.11.1.1 Ammonium salts, like magnesium salts act as weak acids and attack hardened concrete paste resulting in softening and gradual decrease in strength of the concrete.

### **10.11.2 Risk assessment**

- 10.11.2.1 UK guidance is not available on the concentration of ammonium which may affect concrete. BS EN 206-1: 2000 '*Concrete - Part 1: Specification, performance, production and conformity*' does, however, provide exposure classes for concrete in contact with water with varying concentrations of ammonia for the design/specification for concrete mixes.

- 10.11.2.2 As no groundwater was encountered by the investigation, we have not been able to obtain water samples for measurement of concentration of ammonia. In addition the site has no history which provides evidence of the uses of ammonia on site, and in overall conclusion the risk of concrete being affected by ammonia is considered low.

## **10.12 Concrete blocks**

### **10.12.1 Hazards**

- 10.12.1.1 Precast aggregate concrete blocks and autoclaved aerated concrete blocks are commonly used in the construction of shallow foundations. Concrete blocks are potentially attacked by the same contaminants and ground conditions which affect dense concrete.

### **10.12.2 Risk Assessment**

- 10.12.2.1 In general, the mechanism of attack on concrete blocks is the same for hardened concrete. We recommend parameters for ground conditions for concrete described in the preceding paragraphs for concrete blockwork in contact with the ground/groundwater and the blockwork manufacturers confirmation sought for applicability of their product.

## **10.13 Clay Bricks/Pipes**

- 10.13.1 Clay Bricks are highly durable materials which have been used in buildings for many centuries. Fire clay pipe material can also be considered similarly resistant to contaminants.

### **10.13.2 Hazards**

- 10.13.2.1 Dissolution of clay brick is a potentially serious cause of deterioration. The extent of dissolution depends upon the solubility of the glassy material (produced by firing of the clay) contained in the brick. The acidic nature of the glass phase will produce low solubility in a neutral and acidic environment, but can be soluble in a basic environment.
- 10.13.2.2 A potentially more serious hazard for brickwork is the crystallisation of soluble salts within the brick pore structure. Salts are transported by water to the interior of the brick originating from the external environment or by rehydration, however, are only likely to occur when there is a gradient from a wet interior to a drying surface. The potential, therefore, for salt crystallisation in the ground is, therefore, low.

### **10.13.3 Risk Assessment**

- 10.13.3.1 There seems to be little published information as regards the resistance to clay bricks/pipes in aggressive ground conditions, however, clay bricks are generally considered very durable. As no significant concentrations of chemical contaminants have been identified at this site in combination with near neutral pH conditions it is considered unlikely that ground conditions are sufficiently aggressive to cause damage to brickwork/clay pipes.
- 10.13.3.2 Some basic guidance is provided in BS5628-3: 2005 '*Code of Practice for the Use of Masonry - Part 3: Materials and components, design and workmanship*' with regards to resistance of masonry to resist the effects of sulphate attack.

### **10.14 Mortar**

- 10.14.1 Mortars are based on building sands mixed with cement and/or lime as a binder. In the UK Portland cements and masonry cement are commonly used. Masonry cements are a mixture of Portland Cements and fine mineral filler (i.e. Limestone) with an air entraining agent.

#### **10.14.2 Hazards**

- 10.14.2.1 Mortar is subject to the same agents for deterioration as concrete with the major cause of deterioration being sulphate attack.

#### **10.14.3 Risk assessment**

- 10.14.3.1 Sulphates can originate from soils/groundwater or from the bricks themselves. Calcium, magnesium, sodium and potassium sulphates are present in almost all fired-clay bricks. Water can dissolve a fraction of these sulphates and transport them to the mortar.
- 10.14.3.2 Currently, we are not aware of any guidance on the resistance of mortars to sulphate attack. The Building Research Establishment report that the sulphate resistance of mortar was improved by the use of sulphate resisting Portland cements and lime. Some guidance is also provided in BS5628-3: 2005 '*Code of Practice for the use of Masonry - Part 3: Materials and components, design and workmanship*'.
- 10.14.3.3 Based on ground conditions determined at the site the risk of significant sulphate attack on mortars (Based on testing/analysis of sulphates in relation to concrete - refer Section 10.7) is considered low.



## **10.15 Metals - general**

10.15.1 There are a number of metals which are used in buildings either as piles, services, non-structural and, indeed, structural components. The most common metals used in buildings are steel, stainless steel, copper, lead, zinc, aluminium and cast iron. All these metals can deteriorate through corrosion process. Corrosion can affect metals in a variety of ways depending upon the nature of the metal and the environment to which it is subjected. In most common forms of corrosion are:-

- Electrochemical - the most common form of corrosion in an aqueous solution
- Chemical corrosion - occurs when there is a direct charge transfer between the metal and the attacking medium (examples are oxidation, attack by acids, alkalis and organic solvents)
- Microbial induced corrosion

## **10.16 Metals - Cast iron**

10.16.1 Cast iron is a term to describe ferrous metals containing more than 1.7% carbon and is used extensively in the manufacture of pipes.

### **10.16.2 Hazards**

10.16.2.1 Generally, cast iron has a good resistance to corrosion by soils, however, corrosion can occur due to the following mechanisms:-

- 1) Generation of large scale galvanic cells caused by differences in salt concentrations, oxygen availability or presence of stray electrical currents.
- 2) Hydrochloric acid will cause corrosion at any concentration and temperature. Dilute sulphuric, nitric and phosphoric acids are also aggressive as also are well aerated organic acids.

### **10.16.3 Risk assessment**

10.16.3.1 Testing can be carried out on site to measure the resistivity and redox potential of soils which can assist in deriving recommendations for protection of cast iron components using coatings, burial trenches, or isolation techniques. Currently, however, there is no specific guidance and we recommend advice is sought from manufacturers.

10.16.3.2 Guidelines produced by the Water Research Centre (WRc) on the use of ductile iron pipes, state that highly acidic soils (pH <5) are corrosive to cast iron pipe even when protected by a zinc coating or polythene sleeving. WRc also indicate that groundwater containing >300ppm chloride may corrode even protected cast iron pipes.

- 10.16.3.3 On the basis that the pH of soils at the site are not less than 5, and groundwater is unlikely to be in contact with cast iron elements, then the risk of ductile cast iron pipes being affected by acid/chloride attack is considered low. We have not carried out any redox/resistivity testing (considered outside our brief) and thus we cannot comment further with regards to the risks of galvanic action.

## **10.17 Metals - Steel piles**

### **10.17.1 Hazards**

- 10.17.1.1 The corrosion of steel requires the presence of both oxygen and water. In undisturbed natural soils the amount of corrosion of driven steel piles is generally small. In disturbed soils (made ground) however, corrosion rates can be high and normally twice as high as those for undisturbed natural soils.

### **10.17.2 Risk Assessment**

- 10.17.2.1 Guidance on the use of steel piles in different environments is provided in British Steel's piling handbook which includes calculating the effective life of steel piles. There is no specific guidance, however, for contaminated soils in this publication. Coatings can be provided to the pile surface but experience has shown that some coatings can be damaged during driving, particularly in ground which can contain hard materials such as brick/concrete/stone.

## **10.18 Metals - Stainless steel**

### **10.18.1 Hazards**

- 10.18.1.1 Stainless steel is used in a number of building components including services, pipework, reinforcement bars and wall ties. There is little knowledge, however, of the performance of stainless steel in aggressive environments.

### **10.18.2 Risk assessment**

- 10.18.2.1 Stainless steel can withstand pH of 6.5 to 8.5, but the chlorine content of a soil increases the risk of corrosion. At concentrations of 200mg/l type 304 stainless steel can be used, but for concentrations of 200mg/l to 1000mg/l type 316 should be used in preference to type 304, but for concentrations greater than 1000mg/l type 316 should always be used.
- 10.18.2.2 At this site the pH of the natural soils was recorded within the range of 5.0 to 8.2, and whilst groundwater will not be in contact with stainless steel components, we recommend that manufacturer's advice is sought to the affects of soils on stainless steel at the site.

## **10.19 Metals - Galvanised steel**

### **10.19.1 Hazards**

- 10.19.1.1 Galvanising steel is a means of protecting steel from aggressive environments; however, zinc galvanising can be corroded by salts and acids.

### **10.19.2 Risk assessment/remedial action**

- 10.19.2.1 There is no current specific guidance on the effects of aggressive ground conditions on galvanised steel, however, some research indicates zinc alloys are generally more resistant than pure zinc coatings in aggressive conditions.

## **10.20 Metals - Copper**

### **10.20.1 Hazards**

- 10.20.1.1 Copper is commonly used for gas and water supplies. Copper is generally resistant to corrosion in most natural environments, but in contaminated ground copper can be subject to corrosion by acids, sulphates, chlorides and ground containing cinders/ash. Wet peat (pH 4.6) and acid clays (pH 4.2) are considered aggressive conditions to promote corrosion to copper.

### **10.20.2 Risk assessment**

- 10.20.2.1 There is no specific published guidance on what constitutes aggressive conditions to copper except very acid/peaty conditions.
- 10.20.2.2 There are no significantly acidic or peaty conditions in near surface soils at the site or, indeed, significant concentrations of ash/cinders. On this basis the risk of significant corrosion to copper in contact with the ground is considered low.

## **10.21 Metals - Lead**

### **10.21.1 Hazards**

- 10.21.1.1 Lead is used in tanking, flashings, damp proof courses, etc. Lead is a durable material which is resistant to corrosion in most environments. Lead damp proof courses can be subject to attack from the lime released by Portland Cement based mortar and concrete. In the presence of moisture, a slow corrosive attack is initiated on lead sheet. In such cases a thick coat of bitumen should be used to protect the lead damp proof course.

### **10.21.2 Risk assessment**

- 10.21.2.1 There is no current guidance on the performance of lead in contact with contaminated soils, however, acids and alkalis (lime) could be aggressive towards lead.

- 10.21.2.2 At the site pH conditions are not considered significantly extreme and this it is considered unlikely that ground conditions at the site would significantly affect lead.

## **10.22 Plastics - General**

- 10.22.1 The range of plastics in construction is wide and increasing. The deterioration of plastics varies with the individual material and the environment to which it is exposed. In general, plastics deteriorate through degradation of their polymer constituent, but loss of plasticizer and other additives can render plastics ultimately unserviceable.

## **10.23 Plastic membranes and geotextiles**

- 10.23.1 Plastic membranes and textiles are used in the construction industry as damp proof courses, gas resistant membranes, cover systems and liners. They are typically used to restrict the movement of gas or water into buildings, building materials or components or to separate differing soil types. Typically materials used for membranes are polyethylene (PE) and poly vinyl chloride (PVC).

### **10.23.2 Hazards**

- 10.23.2.1 Membranes of PE and PVC are attacked by a variety of acids and solvents. PE has a poor corrosion resistance to oxidising acids (nitric and sulphuric) at high concentrations. Hydrochloric acid (HCl) does not chemically attack PE but can have a detrimental effect on its mechanical properties. Alkalies, basic salts, ammonia solutions and bleaching chemicals such as chlorine will cause deterioration of PE. PE is resistant to non-oxidising salt solutions.

- 10.23.2.2 PVC is degraded by the action of oxidising acids. Nitric acid is particularly aggressive towards PVC. PVC does not deteriorate under the action of neutral or alkaline solutions.

### **10.23.3 Risk assessment**

- 10.23.3.1 There is no published guidance on quantitative assessment of the risks to PE or PVC although there is a lot of advice on how contaminants react with these plastics. In general, the more concentrated the contamination the greater the risk to plastic membranes/geotextiles.

- 10.23.3.2 Based on the investigatory data obtained to date, and in consideration of the hazards described above, there is no evidence of significant concentrations of acids or alkalies, indicating the risks of ground conditions at the site affecting PE and PVC materials are considered low.

## **10.24 Plastic Pipes**

### **10.24.1 Hazards**

10.24.1.1 Plastic pipes are predominantly manufactured from PVC and PE but other materials can be used. In general they perform well but it is known that chemical attack and permeation of contaminants through the pipes can result from use in contaminated land. A published review on plastic pipes reports the following:

- Polyethylene (PE) - good resistance to solvents, acids and alkalis
- Poly vinyl chloride (PVC) - most common form of pipe. Good general resistance to chemical attack but can be attacked by solvents such as ketones, chlorinated hydrocarbons and aromatics
- Polypropylene (PP) - chemically resistant to acids, alkalis and organic solvents but not recommended for use with storing oxidising acids, chlorinated hydrocarbons and aromatics.
- Poly vinylidene fluoride (PVDF) - inert to most solvents, acids and alkalis as well as chlorine, bromide and other halogens
- Polytetrafluoroethylene (PTFE) - one of the most inert thermoplastics available. PTFE has good chemical resistance to solvents, acids and alkalis

10.24.1.2 A survey carried out by the Water Research Centre (WRc) on reported incidents of permeation (more than 25), only two involved PVC with these incidents relating to spillages of fuel.

### **10.24.2 Assessment**

10.24.2.1 A survey carried out by the Water Research Centre (WRc) on reported incidents of permeation (more than 25), only two involved PVC with these incidents relating to spillages of fuel.

10.24.2.2 The UK Water Industry research (UKWIR) have published a document entitled '*Guidance for the selection of Water supply pipes to be used in Brownfield sites*'. The publication defines brownfield sites as

*'Land or premises that have been used or developed. They may also be vacant, or derelict. However they are not necessarily contaminated'*

10.24.2.3 The subject site has not previously been developed and is not considered to be a brownfield site as defined by the UKWIR publication. In addition laboratory test data for polycyclic aromatic hydrocarbons (PAHs) produced no or very limited concentrations above detectable limits. Based on this evidence we are of the opinion that special precautions are unlikely to be required for water supply pipe. We recommend United Utilities is however consulted on this to gain their opinion and requirements.

## **10.25 Electrical cables**

### **10.25.1 Hazards**

- 10.25.1.1 Electrical cables are generally protected by plastic sleeves. These sleeves are potentially subject to chemical and permeation in similar modes as plastic pipes. Medium and low voltage cables are often laid directly into the ground and are thus at risk of attack by contaminants. High voltage cables tend to be laid in trenches backfilled with 'clean' materials.

### **10.25.2 Risk assessment/remedial action**

- 10.25.2.1 The selection of appropriate sheathing material is important to provide resistance to ground conditions at the site and recommend manufacturers' advices are sought.

## **10.26 Rubbers**

### **10.26.1 Hazards**

- 10.26.1.1 Rubbers are crosslinked polymeric materials containing a number of additives such as carbon black, fillers, antioxidant and vulcanising agents. The corrosion resistance of rubber is dependent upon the polymeric constituent. The mechanisms by which rubbers deteriorate when placed in aggressive chemical environments are similar to those described for plastics. Oxidation is the principal form of degradation. Whilst rubbers are resistant to strong acids and alkalis, they are rapidly attacked by oxidising agents such as nitric acid and oxidising salts such as copper, manganese and iron.

- 10.26.1.2 Rubber is also susceptible to attack by certain hydrocarbons and oils. The absorption of these liquids causes the rubber to swell.

### **10.26.2 Risk assessment/remedial action**

- 10.26.2.1 Information on the effect of a range of chemicals on the physical properties of various rubbers has been produced by the Rubber and Plastics Research Association. This was based on observations carried out following immersion tests using undiluted chemicals, but this has limitations such as the effects of combined chemicals and the effects of dilution.

- 10.26.2.2 We recommend manufacturers of the rubber materials likely to be in contact with the ground at the site are consulted to confirm, or otherwise, the applicability of their product.

## 11 Landfill issues

---

11.1	Disposal of soils off site
11.2	Landfill tax
11.3	Reuse of soils – Materials Management Plans

---

### 11.1 Disposal of soils off site

- 11.1.1 Disposal of waste soils must comply with the Landfill Directive and amendments to the '*Landfill (England and Wales) Regulations*'. Essentially, this requires the 'waste producer' to classify soils for off-site disposal to an appropriately licensed landfill facility. Laboratory testing on soils from the site would be required to allow such classification in accordance with current Environment Agency waste acceptance criteria and procedures. We can carry such testing and an assessment of soil classification for disposal on further instructions.

### 11.2 Landfill tax

- 11.2.1 Disposal of soils to landfill sites is normally subject to landfill tax with rates varying from year to year based on government policy. Current information on rates of landfill tax can be obtained from the HM Revenue and Customs website ([www.hmrc.gov.uk](http://www.hmrc.gov.uk)).

### 11.3 Reuse of Soils - Materials Management Plans

- 11.3.1 Where soils are to be moved and reused onsite, or are to be imported to the site, a Waste Exemption or an Environmental Permit is required.
- 11.3.2 An alternative is the use of a Materials Management Plan (MMP) to determine where soils are and are not considered to be a waste. By following '*The Definition of Waste: Development Industry Code of Practice*' published by CL:AIRE (produced in 2008 and revised in March 2011), soils that are suitable for reuse without the need for remediation (either chemical or geotechnical) and have a certainty of use, are not considered to be waste and therefore do not fall under waste regulations. In addition, following this guidance may present an opportunity to transfer suitable material between sites, without the need for Waste Exemptions or Environmental Permits.
- 11.3.3 MMPs offering numerous benefits, including maximising the use of soils onsite, minimising soils going to landfill and reducing costs and time involved in liaising with waste regulators.
- 11.3.4 We can provide further advice on this and provide fees for producing a Materials Management Plan on further instructions.

## **12 Further investigations**

12.1 Although we have endeavoured to provide a comprehensive investigation for the proposed development within budgetary constraints there are areas, which we recommend further investigations be carried out. These are as follows:

- Further insitu CBR testing using a TRL DCP probe along proposed access roads and hardstanding may yield a value above 3% which would decrease the required formation thickness and provide associated cost savings.
- Precautionary testing to determine hardness values within surface waters of Higgin Brook onsite which will enable a more detailed risk assessment to be completed in relation to water receptors.

12.2 We would be pleased to carry out any of the supplementary investigations described above and provide proposals with costings on further instructions.



## **13 Remediation strategy and specification**

- 13.1.1 We have not identified any significant chemical contamination at the subject site, therefore, remediation is not considered necessary. It is recommended, however, that hardness values within surface waters of Higgin Brook are determined to enable a more detailed risk assessment to be completed in relation to water receptors.

Residential development  
Phase 2, Chipping Lane, Longridge

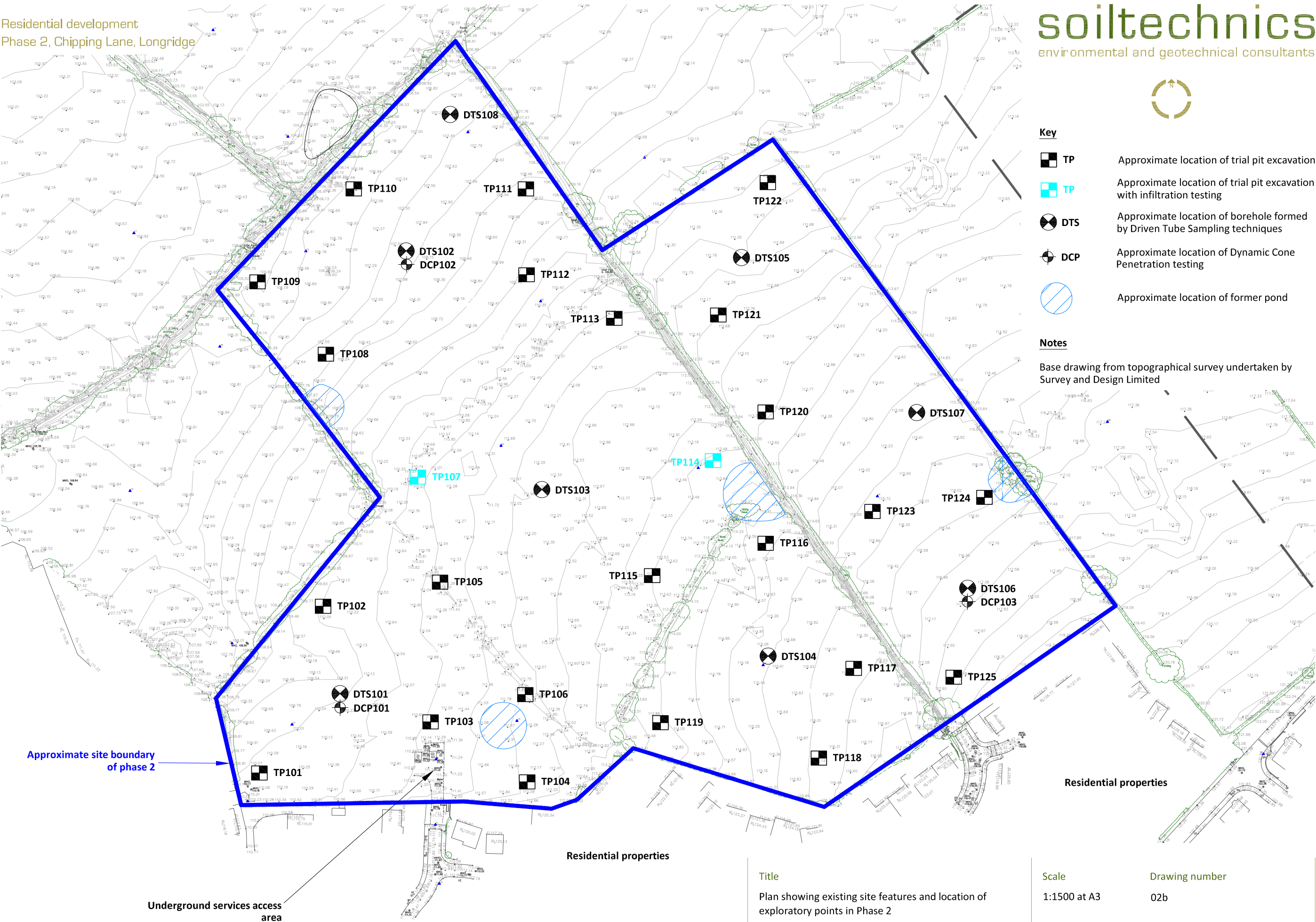


Key

- TP Approximate location of trial pit excavation
- TP Approximate location of trial pit excavation with infiltration testing
- DTS Approximate location of borehole formed by Driven Tube Sampling techniques
- DCP Approximate location of Dynamic Cone Penetration testing
- Approximate location of former pond

Notes

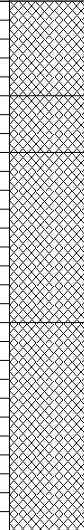
Base drawing from topographical survey undertaken by Survey and Design Limited



Title  
Plan showing existing site features and location of  
exploratory points in Phase 2

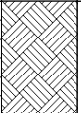
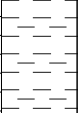
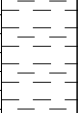
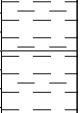
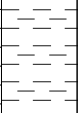
Scale  
1:1500 at A3

Drawing number  
02b

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft medium strength brown gravelly very sandy CLAY with frequent rootlets. Gravel consists of fine to medium sandstone and quartzite. MADE GROUND		0.25		HVP 0.10	52	0.10		D
...masonry slab 0.2x0.3m in size at 0.2m depth.								
Medium dense brown gravelly very clayey SAND. Gravel consists of fine to medium sandstone, quartzite and brick. MADE GROUND		0.40		HVP 0.50	139	0.50		D
Firm high strength gravelly very sandy CLAY. Gravel consists of fine to medium sandstone, quartzite and brick. MADE GROUND								
...0.1m diameter ceramic land drain running east to west at 0.6m depth.								
		0.85					0.90 0.90	D ES
Medium dense grey clayey very gravelly SAND. Sand is fine to medium. Gravel consists of sandstone and occasional quartzite. MADE GROUND								
...from 1m depth, becoming very clayey.								
		1.40		HVP 1.50 PP 1.50	116 83	1.50		D
Firm high becoming very high strength brown mottled grey slightly silty slightly gravelly CLAY. Gravel consists of medium sandstone and mudstone. DEVENSIAN TILL								
...from 1.7m depth, becoming stiff.								
		HVP 1.90 PP 1.90	201 197	1.90		D		
...from 2m depth, becoming friable.								
...from 2.4m depth, becoming very stiff.								
		HVP 2.60 PP 2.60	213 217	2.60		D		
TRIAL PIT TERMINATED AT 2.80m		2.80						

**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b>	<b>Surface breaking</b>
		Trial pit record	No
<b>Groundwater observations</b>	<b>Dimensions (W x L)</b>	<b>Date of excavation (range if applicable)</b>	<b>Appendix</b>
No groundwater encountered.	0.60m x 2.50m	16/02/2016	C
	<b>Method of excavation</b>	<b>Location plan on drawing number</b>	<b>TP101</b>
	JCB 3CX	02b	

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto medium dense brown gravelly very clayey organic SAND with frequent rootlets. Gravel consists of medium sandstone and mudstone. TOPSOIL		0.30		HVP 0.50 PP 0.50	116 103	0.20		D
Firm high strength orangish brown slightly silty gravelly CLAY. Gravel consists of fine to coarse sandstone and mudstone. DEVENSIAN TILL								
...from 0.5m depth, becoming slightly gravelly.								
		1.20		HVP 1.00 PP 1.00	103 88	1.00		D
Stiff very high strength orangish brown mottled grey slightly gravelly friable CLAY. Gravel consists of fine to coarse sandstone and mudstone. DEVENSIAN TILL								
		1.90		HVP 1.30 PP 1.30	179 192	1.90		D
Stiff very high strength orangish brown mottled grey slightly gravelly friable CLAY. Gravel consists of fine to coarse sandstone and mudstone. DEVENSIAN TILL								
		2.50		HVP 1.90 PP 1.90	266 225	2.70		D
Stiff very high strength brown silty CLAY with occasional gravels of medium sandstone and occasional cobbles of sandstone. DEVENSIAN TILL								
		3.10		HVP 2.70 PP 2.70	160 135			
Stiff very high strength brown silty CLAY with occasional gravels of medium sandstone and occasional cobbles of sandstone. DEVENSIAN TILL								
TRIAL PIT TERMINATED AT 3.10m								

**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 16/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP102</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft very low strength brown sandy CLAY with frequent rootlets and occasional gravels of sandstone, mudstone and ceramic. MADE GROUND		0.15		PP 0.10	8	0.10		D
Firm high strength brown slightly silty slightly sandy CLAY with occasional gravels of medium sandstone and brick. MADE GROUND		0.45		HVP 0.30 PP 0.30	82 75	0.30		D
Loose dark brown clayey SAND with occasional gravels of medium sandstone. MADE GROUND		0.70				0.70		D
...from 1.3m depth, becoming gravelly.		1.40				1.40		D
Soft to firm medium strength slightly gravelly sandy CLAY. Gravel consists of fine to medium sandstone and limestone. MADE GROUND		1.50		HVP 1.60 PP 1.60	58 40	1.70		D
...0.1m diameter wet ceramic land drain running north to south at 1.8m depth.		2.00						
Stiff high and very high strength brown silty CLAY with occasional gravels of medium sandstone and mudstone and occasional cobbles of sandstone. DEVENSIAN TILL		2.60		HVP 2.20 PP 2.20	134 188	2.60		D
TRIAL PIT TERMINATED AT 3.10m		3.10		HVP 3.10 PP 3.10	122 207			

**Notes:** Trial pit sides remained upright and stable upon completion.

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Trial pit record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Dimensions (W x L)**

0.60m x 2.50m

**Date of excavation (range if applicable)**

16/02/2016

**Appendix**

C



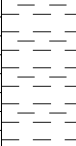
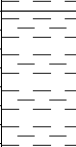
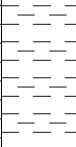
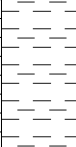
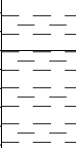

**Method of excavation**

JCB 3CX

**Location plan on drawing number**

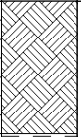
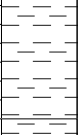
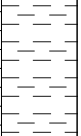
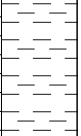
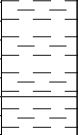
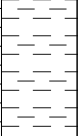
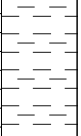
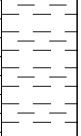
02b

**TP103**

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft very low strength dark brown sandy organic CLAY with frequent rootlets. TOPSOIL		0.25				0.10		D
Soft to firm low strength brown sandy CLAY. DEVENSIAN TILL		0.60		HVP 0.40 PP 0.40	33 110	0.40		D
Firm high strength orangish brown sandy CLAY. DEVENSIAN TILL		1.05		HVP 0.70 PP 0.70	79 90	0.70		D
Stiff high becoming very high strength dark brown sandy CLAY. DEVENSIAN TILL				HVP 1.10 PP 1.10	125 110			
...from 1.7m depth, becoming very stiff.				HVP 1.50 PP 1.50	152 128	1.50		D
				HVP 2.10 PP 2.10	258 217			
Very stiff very high strength brown slightly silty CLAY. DEVENSIAN TILL		2.40		HVP 2.60 PP 2.60	228 225			
TRIAL PIT TERMINATED AT 2.80m		2.80				2.80		D

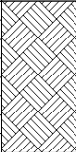
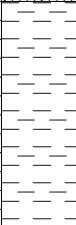

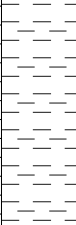
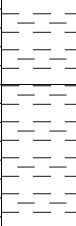
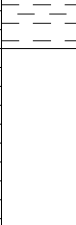
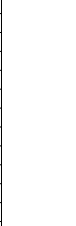
**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 16/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP104</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown very sandy organic CLAY. TOPSOIL						0.10		B
Soft medium strength brown slightly silty slightly sandy CLAY. DEVENSIAN TILL		0.35		HVP 0.50 PP 0.50	52 60	0.50		B
Firm high strength brown slightly silty slightly sandy CLAY. DEVENSIAN TILL		0.70		HVP 0.90 PP 0.90	79 100	0.90		B
...from 1.3m depth, becoming stiff.				HVP 1.40 PP 1.40	99 105			
Stiff very high strength brown mottled grey CLAY with occasional gravels of fine to medium sandstone. DEVENSIAN TILL		1.80		HVP 2.00 PP 2.00	149 183	2.00		B
Stiff brown slightly silty CLAY with occasional cobbles of sandstone. DEVENSIAN TILL		2.30				2.20		B
...from 2.8m depth, becoming very stiff.								
TRIAL PIT TERMINATED AT 3.20m		3.20						

**Notes:** Trial pit sides remained upright and stable upon completion.

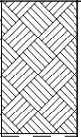
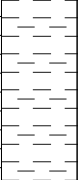
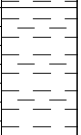


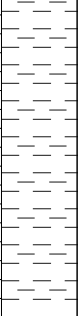


<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 16/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP105</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown sandy organic CLAY with frequent rootlets. TOPSOIL		0.40				0.10		D
Firm to stiff medium and high strength grey brown slightly silty slightly sandy slightly gravelly CLAY. Gravel consists of medium to coarse sandstone and mudstone. DEVENSIAN TILL				HVP 0.50 PP 0.50	85 47	0.50		D
Stiff very high strength brown mottled bluish grey CLAY. DEVENSIAN TILL		1.00		HVP 0.80 PP 0.80	82 87			
				HVP 1.20 PP 1.20	167 175	1.10		D
<i>...from 1.5m depth, becoming very stiff and friable.</i>								
				HVP 2.10	167	2.10		D
Stiff high strength brown slightly silty CLAY with occasional gravels of fine mudstone. DEVENSIAN TILL		2.40						
				HVP 2.90 PP 2.90	122 138	2.90		D
TRIAL PIT TERMINATED AT 2.90m		2.90						

**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 16/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP106</b>



DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown sandy CLAY with frequent rootlets and occasional gravels of medium sandstone. TOPSOIL		0.35		HVP 0.50 PP 0.50	78 82	0.10		D
Firm high strength orangish brown slightly sandy slightly gravelly silty CLAY. Gravel consists of fine to medium sandstone. DEVENSIAN TILL								
Firm high strength dark brown mottled bluish grey CLAY with occasional gravels of medium to coarse sandstone and mudstone. DEVENSIAN TILL		0.85		HVP 0.90 PP 0.90	103 110			
...from 1.2m depth, becoming stiff.								
...boulder of mudstone in southern end of pit at 1.4m depth.				HVP 1.40 PP 1.40	116 142	1.40		D
								
		2.50		HVP 2.60 PP 2.60	125 160			
Stiff high strength brown slightly silty CLAY. DEVENSIAN TILL		2.80						
TRIAL PIT TERMINATED AT 2.80m								

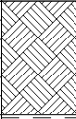
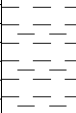
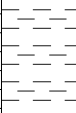

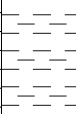
**Notes:** Trial pit sides remained upright and stable upon completion. Infiltration testing performed.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 17/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP107</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING			
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE	
Grass onto soft medium strength brown sandy gravelly CLAY with frequent rootlets. Gravel consists of medium to coarse brick, timber and sandstone. MADE GROUND		0.20		HVP 0.10	15	0.10		D	
				PP 0.10	60				
Soft medium strength brown sandy gravelly CLAY. Gravel consists of fine to coarse brick, timber, ceramic, sandstone and occasional half brick. MADE GROUND				HVP 0.50	64	0.50		B	
<u>...cut sandstone blocks up to 0.1x0.4x0.2m in size at 0.3m depth.</u>				PP 0.50	98				
				HVP 1.00	46				
<u>...cut sandstone blocks up to 0.2x0.2x0.2m in size at 1.1m depth.</u>				PP 1.00	60				
Firm to stiff high and very high strength reddish brown slightly sandy slightly gravelly CLAY. Gravel consists of medium mudstone. DEVENSIAN TILL		1.20		HVP 1.40	134	1.80 2.00		D	
<u>...from 1.6m depth, becoming stiff.</u>									
<u>...from 2.5m depth, becoming very stiff.</u>									
TRIAL PIT TERMINATED AT 2.80m		2.80		HVP 2.80	134				

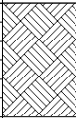
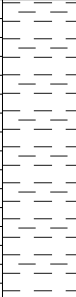
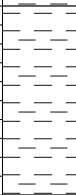
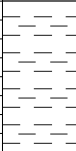

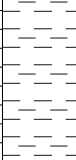
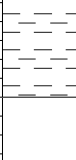
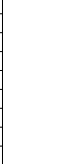


**Notes:** Some collapse of trial pit sides between 0.2m and 1.0m depth.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 17/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP108</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown very sandy organic CLAY with frequent rootlets and occasional roots. TOPSOIL		0.30				0.10		D
Firm medium strength brown mottled grey silty sandy CLAY. DEVENSIAN TILL				HVP 0.40	97	0.50		D
...from 0.6m depth, becoming soft with occasional roots.				HVP 0.70	46			
				HVP 1.00	59			
				HVP 1.30	90	1.30		D
firm high strength brown mottled bluish grey CLAY with occasional gravels of medium mudstone and quartzite. DEVENSIAN TILL		1.20		HVP 1.60	190			
...from 1.5m depth, becoming stiff.						1.90		D
				HVP 2.30	155			
Stiff high and very high strength brown slightly silty CLAY with occasional gravels of medium mudstone. DEVENSIAN TILL		2.20		HVP 3.10	140			
TRIAL PIT TERMINATED AT 3.30m		3.30						

**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 17/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP109</b>

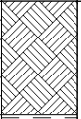
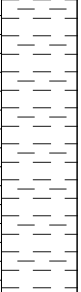
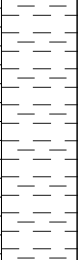
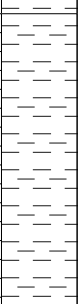
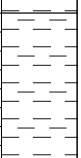
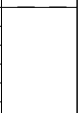
DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto firm low strength dark brown sandy organic CLAY with frequent rootlets. TOPSOIL		0.30				0.10		D
Firm medium becoming high strength orangish brown slightly sandy silty CLAY. DEVENSIAN TILL						HVP 0.50 PP 0.50		
		1.10					1.10	D
Stiff medium strength brown mottled bluish grey CLAY. DEVENSIAN TILL								
		1.60						D
Stiff high strength brown mottled bluish grey slightly sandy gravelly CLAY. Gravel consists of fine to medium mudstone. DEVENSIAN TILL								
		2.00						D
Very stiff high strength brown occasionally sandy occasionally gravelly CLAY. Gravel consists of medium sandstone and mudstone. DEVENSIAN TILL								
<u>...from 2m depth, becoming very stiff.</u>		3.10					3.00	D
								
TRIAL PIT TERMINATED AT 3.10m								

**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 17/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP110</b>

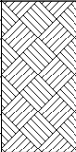
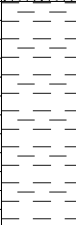

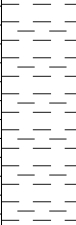

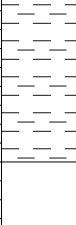
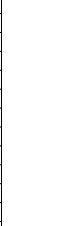



**Notes:** Trial pit sides remained upright and stable upon completion.

**Report ref:** STN3505NM-G02

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown sandy organic CLAY with occasional gravels of medium sandstone and frequent rootlets. TOPSOIL		0.30				0.10		D
Firm medium strength slightly silty sandy orangish brown CLAY with occasional gravels of sandstone. DEVENSIAN TILL				HVP 0.50	73	0.50		D
...from 1.1m depth, becoming very sandy.		1.80		HVP 1.20	157			
Very stiff very high strength reddish brown mottled grey CLAY with occasional gravels and cobbles of medium to coarse sandstone and mudstone. DEVENSIAN TILL				HVP 2.00	198	2.00		D
...from 2.2m depth, becoming friable.		2.60						
Stiff very high strength brown slightly silty CLAY. DEVENSIAN TILL				HVP 2.80	198			
TRIAL PIT TERMINATED AT 3.00m		3.00						

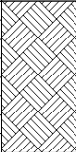
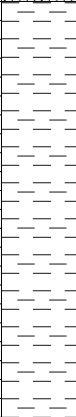







**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 17/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP112</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark sandy organic CLAY with frequent rootlets. TOPSOIL		0.40				0.10		D
Firm medium strength orangish brown slightly sandy slightly gravelly silty CLAY. Gravel consists of fine to medium mudstone. DEVENSIAN TILL				HVP 0.50 PP 0.50	64 47	0.60		D
Stiff medium strength becoming high and very high strength reddish brown mottled bluish grey CLAY with occasional gravels of medium quartzite and sandstone. DEVENSIAN TILL		1.00		HVP 1.10 PP 1.10	91 60	1.10		D
...from 1.5m depth, becoming very stiff.								
								
								
								
Stiff very high strength brown slightly silty CLAY. DEVENSIAN TILL		2.50		HVP 2.40	144	2.40		D
						3.00		D
TRIAL PIT TERMINATED AT 3.20m		3.20		HVP 3.20	155			

**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 17/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP113</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown very sandy organic CLAY with frequent rootlets. TOPSOIL		0.40				0.10		D
Soft becoming firm medium strength orangish brown slightly silty slightly sandy CLAY. DEVENSIAN TILL				HVP 0.50 PP 0.50	65 88	0.50		D
				HVP 0.90 PP 0.90	58 70			
				HVP 1.30 PP 1.30	65 68	1.30		D
...from 1.5m depth, becoming stiff and friable.		1.70						
Firm high strength reddish brown CLAY with occasional gravels of medium mudstone. DEVENSIAN TILL				HVP 1.70	127	1.70		D
				HVP 2.30	120			
...no longer friable and occasional gravels of mudstone from 2.4m depth.		2.70						
TRIAL PIT TERMINATED AT 2.70m								

**Notes:** Trial pit sides remained upright and stable upon completion. Infiltration testing performed.

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Trial pit record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Dimensions (W x L)**

0.60m x 3.20m

**Date of excavation (range if applicable)**

17/02/2016

**Appendix**

C

**Method of excavation**

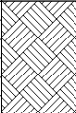
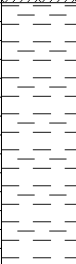

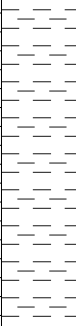
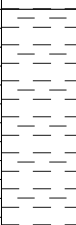
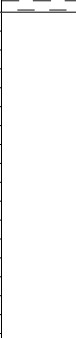
JCB 3CX

**Location plan on drawing number**

02b

**TP114**



DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown very sandy organic CLAY with frequent rootlets. TOPSOIL		0.30				0.10		B
Firm medium becoming high strength brown slightly gravelly silty CLAY with occasional cobbles of sandstone. Gravel consists of medium to coarse sandstone and quartzite. DEVENSIAN TILL				HVP 0.40	73	0.50		B
				HVP 0.80	88			
Firm to stiff medium becoming high strength reddish brown mottled bluish grey CLAY with occasional gravels of fine sandstone and mudstone. DEVENSIAN TILL		1.00		HVP 1.20	76	1.10		B
...from 1.6m depth, becoming stiff.				HVP 1.80	119			
Stiff high strength reddish brown CLAY with occasional cobbles of mudstone. DEVENSIAN TILL		2.50		HVP 2.70	140	2.70		B
TRIAL PIT TERMINATED AT 3.10m		3.10						

**Notes:** Trial pit sides remained upright and stable upon completion.

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Trial pit record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Dimensions (W x L)**

0.60m x 2.50m

**Date of excavation (range if applicable)**

17/02/2016

**Appendix**

C

**Method of excavation**

JCB 3CX

**Location plan on drawing number**


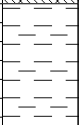
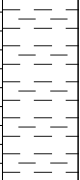
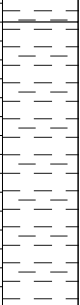
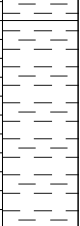
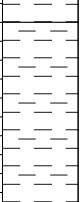

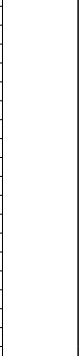
02b

**TP115**

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Firm dark brown very sandy CLAY with occasional gravels of fine sandstone and frequent rootlets. TOPSOIL						0.10		D
Firm high strength dark brown gravelly CLAY. Gravel consists of fine to coarse sandstone and mudstone. DEVANSIAN TILL		0.30		HVP 0.40	94	0.40		D
Firm to stiff high strength orangish brown slightly silty CLAY with occasional gravels of medium sandstone. DEVANSIAN TILL		0.60		HVP 0.80	76	0.70		D
				HVP 1.20	99			
Firm medium strength brown mottled bluish grey CLAY. DEVANSIAN TILL		1.30		HVP 1.50	67	1.40		D
Stiff high becoming very high strength brown mottled bluish grey friable CLAY. DEVANSIAN TILL		1.60		HVP 1.80	140			
				HVP 2.60	198			
Stiff high strength reddish brown slightly silty CLAY with occasional gravels of medium mudstone. DEVANSIAN TILL		3.00		HVP 3.00	146			
		3.10						
TRIAL PIT TERMINATED AT 3.10m								




















Notes: Trial pit sides remained upright and stable upon completion.

Ground level (mAOD)	Co-ordinates	Title Trial pit record	Surface breaking No
Groundwater observations No groundwater encountered.	Dimensions (W x L) 0.60m x 2.50m	Date of excavation (range if applicable) 17/02/2016	Appendix C
	Method of excavation JCB 3CX	Location plan on drawing number 02b	TP116

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown sandy organic CLAY with frequent rootlets. TOPSOIL		0.25		HVP 0.40	76	0.10		D
Firm medium strength orangish brown slightly silty CLAY with occasional gravels of coarse sandstone. DEVENSIAN TILL						0.50		
<u>...from 0.6m depth, becoming mottled grey in colour.</u>								
Firm high strength orange mottled grey CLAY. DEVENSIAN TILL		1.10		HVP 1.20	84	1.20		D
				HVP 1.60	120			
Very stiff very high strength orange mottled grey friable CLAY. DEVONSIAN TILL		1.90		HVP 2.00	169	2.00		D
		2.50						
Stiff very high strength brown slightly silty CLAY with occasional gravels and cobbles of coarse mudstone. DEVENSIAN TILL		3.00		HVP 3.00	182			
TRIAL PIT TERMINATED AT 3.00m								
								

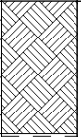
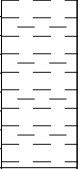
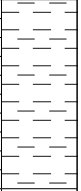
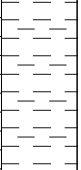
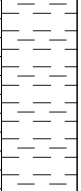
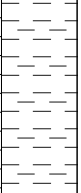
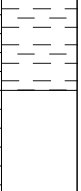
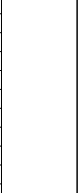
**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 17/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP117</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown very sandy organic CLAY with frequent rootlets TOPSOIL						0.10		D
Firm medium strength orangish brown slightly silty slightly sandy CLAY with occasional gravels of medium sandstone. DEVENSIAN TILL		0.20						
Stiff high strength brown mottled bluish grey slightly sandy CLAY with occasional gravels of medium mudstone. DEVENSIAN TILL		0.40		HVP 0.50 PP 0.50	100 90	0.50		D
				HVP 1.00 PP 1.00	109 100	1.10		D
				HVP 1.40	96			
<i>...from 1.6m depth, becoming very stiff and friable.</i>								
				HVP 1.90	119	1.80		D
								
Very stiff high strength brown slightly silty CLAY with occasional cobbles of sandstone. DEVENSIAN TILL		2.40		HVP 2.40	137			
						2.80		D
TRIAL PIT TERMINATED AT 3.00m		3.00						
								
								
								
								
								
								
								
								

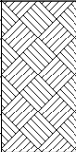
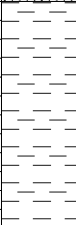
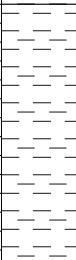
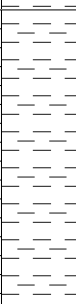
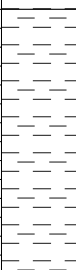
**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 18/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP118</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown very sandy organic CLAY with frequent rootlets. TOPSOIL						0.10		D
Firm orangish brown slightly silty slightly sandy CLAY with occasional gravels of medium sandstone. DEVENSIAN TILL		0.35		HVP 0.60	79	0.50		D
Firm high strength greyish brown slightly gravelly CLAY. Gravel consists of medium sandstone. DEVENSIAN TILL		0.80		HVP 0.90	79	1.00		D
<i>...boulder sized pockets of sand at south end of pit between 0.8m and 1.0m depth.</i>				HVP 1.10	144			
Stiff very high becoming high strength reddish brown mottled grey CLAY with occasional gravels of coarse sandstone. DEVENSIAN TILL		1.30		HVP 1.50	195	1.40		D
				HVP 2.10	175			
				HVP 2.80	109	2.80		D
TRIAL PIT TERMINATED AT 3.00m		3.00						

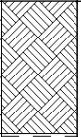
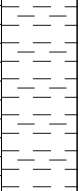
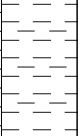
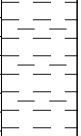
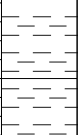
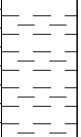
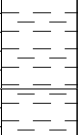
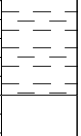
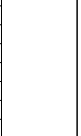
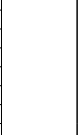
**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 18/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP119</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown very sandy organic CLAY with frequent roots and occasional cobbles of sandstone. TOPSOIL		0.40				0.10		D
Firm medium strength orangish brown slightly silty sandy CLAY with occasional gravels and cobbles of medium sandstone. DEVENSIAN TILL				HVP 0.50 PP 0.50	70 65	0.50		D
Firm to stiff medium to high strength orangish brown mottled bluish grey sandy CLAY. DEVENSIAN TILL		1.00		HVP 1.10 PP 1.10	79 75	1.10		D
				HVP 1.50	94			
Stiff high strength brown mottled bluish grey slightly sandy friable CLAY with occasional coarse gravels and cobbles of mudstone and quartzite. DEVENSIAN TILL		1.70		HVP 1.90	137	1.90		D
						2.00		D
				HVP 2.40	143			
Very stiff very high strength brown mottled bluish grey slightly silty slightly sandy friable CLAY with occasional coarse gravels and cobbles of mudstone and quartzite. DEVENSIAN TILL		2.50		HVP 2.90	167			
				HVP 3.20	213			
----- TRIAL PIT TERMINATED AT 3.20m -----		3.20						


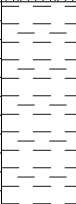
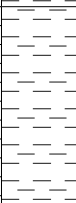


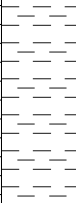
**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 18/02/2016	<b>Appendix</b> -
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP120</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown very sandy organic CLAY with frequent rootlets and occasional gravels of medium sandstone. TOPSOIL						0.10		B
Firm low to medium strength orangish brown slightly sandy CLAY with occasional gravels and cobbles of coarse sandstone. DEVENSIAN TILL <i>...pockets of grey sandy at north end of pit between 0.5m and 0.9m depth.</i>		0.35		HVP 0.40	38	0.50		B
Firm medium strength brown mottled grey sandy CLAY DEVENSIAN TILL		0.90		HVP 0.80	61			
				HVP 1.10	58	1.00		B
Stiff high strength brown mottled grey and bluish grey CLAY. DEVENSIAN TILL		1.30		HVP 1.40	94	1.50		B
				HVP 1.80	134			
Very stiff very high strength brown mottled grey and bluish grey friable CLAY. DEVENSIAN TILL		1.90		HVP 2.20	160			
				HVP 2.90	140			
Stiff high strength brown slightly silty CLAY with occasional gravels of medium mudstone. DEVENSIAN TILL		2.70						
TRIAL PIT TERMINATED AT 3.10m		3.10				3.10		B

**Notes:** Trial pit sides remained upright and stable upon completion.


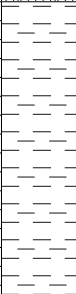
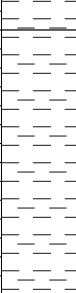
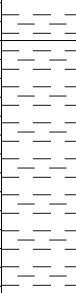
<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> Slight seepage in sand between 0.5m and 0.7m depth.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 18/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP121</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto high strength dark brown very sandy organic CLAY with occasional gravels of medium sandstone and quartzite and frequent rootlets. TOPSOIL		0.25				0.10		D
Firm high strength brown mottled grey slightly silty slightly gravelly sandy CLAY. Gravel consists of medium to coarse sandstone and quartzite. DEVENSIAN TILL				HVP 0.50 PP 0.50	75 100	0.50		D
Stiff high strength brown mottled bluish grey slightly sandy slightly gravelly CLAY. Gravel consists of sandstone and quartzite. DEVENSIAN TILL		0.80		HVP 0.90 PP 0.90	114 130	0.90		D
Very stiff very high strength brown mottled bluish grey slightly gravelly sandy CLAY. Gravel consists of sandstone and quartzite. DEVENSIAN TILL		1.40		HVP 1.50 PP 1.50	186 193	1.80		D
...from 2.1m depth, becoming slightly silty.				HVP 2.20	170			
TRIAL PIT TERMINATED AT 3.10m		3.10		HVP 3.10	179			

**Notes:** Trial pit sides remained upright and stable upon completion.

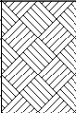










<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 18/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP122</b>



DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft dark brown sandy organic CLAY with frequent rootlets. TOPSOIL		0.25				0.10		D
Firm medium strength orangish brown slightly silty slightly sandy CLAY with occasional gravels of fine to medium sandstone. DEVENSIAN TILL				HVP 0.50	71	0.50		D
Firm to stiff medium strength orangish brown mottled grey slightly silty CLAY with occasional gravels of medium sandstone and quartzite. DEVENSIAN TILL		1.10		HVP 1.00	61			
				HVP 1.40	56	1.20		D
				HVP 1.70	52			
Stiff very high strength reddish brown mottled bluish grey slightly sandy CLAY with occasional gravels of sandstone and medium quartzite. DEVENSIAN TILL		1.90		HVP 2.10	172	2.10		D
				HVP 2.20	155			
<u>...from 2.6m depth, becoming stiff and friable.</u>				HVP 2.60	179			
TRIAL PIT TERMINATED AT 3.10m		3.10						

**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 18/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP123</b>

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
Grass onto soft high strength dark brown sandy organic CLAY with frequent rootlets. TOPSOIL		0.30				0.10		D
Firm medium strength orangish brown mottled grey slightly silty slightly sandy CLAY with occasional gravels of medium to coarse sandstone and quartzite. DEVENSIAN TILL				HVP 0.40 PP 0.40	88 75	0.50		D
		1.60		HVP 0.80 PP 0.80	73 70			
...boulder of grey mudstone at 1.1m depth..				HVP 1.10 PP 1.10	68 50	1.10		D
		1.80		HVP 1.50 PP 1.50	100 90			
Firm orangish brown mottled grey CLAY with occasional gravels of medium sandstone and mudstone. DEVENSIAN TILL						1.90		D
Stiff high becoming very high strength brown mottled bluish grey slightly sandy friable CLAY with occasional gravels of medium sandstone. DEVENSIAN TILL		3.00		HVP 2.00 PP 2.00	135 155			
				HVP 2.40 PP 2.40	213 225			
...from 2.4m depth, becoming very stiff.		3.00				2.90		D
				HVP 3.00 PP 3.00	213 225			
TRIAL PIT TERMINATED AT 3.00m								

**Notes:** Trial pit sides remained upright and stable upon completion.

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Trial pit record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Dimensions (W x L)**

0.60m x 2.50m

**Date of excavation (range if applicable)**

18/02/2016

**Appendix**

C

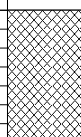
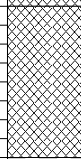
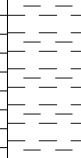
**Method of excavation**

JCB 3CX

**Location plan on drawing number**


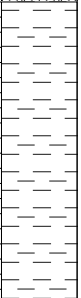
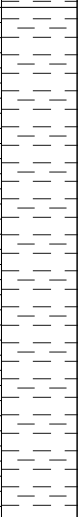
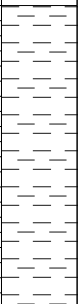
02b

**TP124**

DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING			
				TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE	
Grass onto soft medium strength dark brown slightly gravelly very sandy CLAY. Gravel consists of sandstone, limestone and timber. MADE GROUND		0.35		HVP 0.10	67	0.10		D	
Soft medium strength reddish brown slightly sandy gravelly CLAY. Gravel consists of fine to coarse brick, sandstone, loose wire and glass. MADE GROUND				HVP 0.50	58	0.50 0.51		D ES	
Firm high strength brown CLAY with occasional gravels of sandstone and occasional rootlets. MADE GROUND				1.00	HVP 1.10	122	1.20		D
Stiff high strength orangish brown slightly silty CLAY. DEVENSIAN TILL				1.40	HVP 1.50	94	1.70		B
...from 1.8m depth, becoming very stiff.		2.20		HVP 1.90	141				
Stiff very high strength reddish brown mottled grey occasionally sandy CLAY with occasional gravels of medium sandstone. DEVENSIAN TILL				HVP 2.40	190				
TRIAL PIT TERMINATED AT 3.10m		3.10		HVP 3.10	198	3.10		B	

**Notes:** Trial pit sides remained upright and stable upon completion.

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Trial pit record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.	<b>Dimensions (W x L)</b> 0.60m x 2.50m	<b>Date of excavation (range if applicable)</b> 18/02/2016	<b>Appendix</b> C
	<b>Method of excavation</b> JCB 3CX	<b>Location plan on drawing number</b> 02b	<b>TP125</b>

WELL	DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
					TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
	Grass onto soft dark brown organic CLAY with frequent rootlets and occasional sub-angular gravels of medium sandstone. TOPSOIL		0.30				0.10		D
	Firm high strength brown slightly silty slightly sandy CLAY with occasional gravels of medium sandstone. DEVENSIAN TILL				PP 0.50	80	0.50		D
					PP 0.90	75			
	Stiff very high becoming high strength brown slightly gravelly CLAY. Gravel consists of medium to coarse sandstone and mudstone. DEVENSIAN TILL		1.30		HVP 1.40 PP 1.40	140 165	1.50		D
	<i>...from 1.6m depth, becoming very stiff.</i>				HVP 1.80 PP 1.80	213 220			
					HVP 2.35 PP 2.35	94 113			
					PP 2.70	120	2.70		D
	Firm medium strength light reddish brown CLAY with occasional gravels of medium mudstone. DEVENSIAN TILL		3.00		PP 3.10	73			
					PP 3.50	75			
					HVP 3.70	64	3.80		D
					PP 3.90	63			
	BOREHOLE TERMINATED AT 4.00m		4.00						

**Notes:** For Dynamic Cone Penetration testing, refer to DCP101.

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Driven tube sampler borehole record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Date of excavation (range if applicable)**

16/02/2016
















**Appendix**

D

**Location plan on drawing number**

02b

**DTS101**

WELL	DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
					TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
	Grass onto soft dark brown sandy organic CLAY with frequent rootlets. TOPSOIL								
	Firm medium strength orangish brown slightly sandy silty CLAY DEVENSIAN TILL		0.25				0.20		D
	<i>...from 0.5m depth, becoming soft.</i>						0.40		D
	Firm brown slightly sandy slightly gravelly CLAY. Gravel consists of medium to coarse sandstone and mudstone. DEVENSIAN TILL		0.80		HVP 0.60	77		0.90	D
	Stiff high strength brown CLAY. DEVENSIAN TILL		1.30		PP 1.35	137			
					HVP 1.50	128	1.40		D
					PP 1.70	113			
					PP 2.10	123			
					PP 2.50	97			
	Soft to firm medium strength reddish brown slightly silty CLAY. DEVENSIAN TILL		2.60		HVP 2.80	65			
					PP 2.80	48			
					PP 3.20	62			
					PP 3.60	52	3.50		D
					PP 4.00	38			
	BOREHOLE TERMINATED AT 4.00m		4.00						

**Notes:** For Dynamic Cone Penetration testing, refer to DCP102.

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Driven tube sampler borehole record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Date of excavation (range if applicable)**

16/02/2016







**Appendix**

D

**Location plan on drawing number**

02b

**DTS102**

WELL	DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
					TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
	Grass onto soft dark brown slightly sandy slightly gravelly organic CLAY with frequent rootlets. Gravel consists of medium mudstone. TOPSOIL		0.30				0.10		D
	Firm medium and high strength brown slightly gravelly CLAY. Gravel consists of medium sandstone and mudstone. DEVENSIAN TILL <i>...cobble of sandstone at 0.4m depth.</i>				PP 0.40	85			
					HVP 0.60	63		0.70	D
					PP 0.80	55			
	Firm medium strength orangish brown silty CLAY. DEVENSIAN TILL		1.10		PP 1.20	72			
					HVP 1.40	94			
					PP 1.60	68		1.50	D
	<i>...from 2m depth, becoming dark brown mottled bluish grey in colour.</i>				PP 2.10	63			
					HVP 2.50	56			
	Stiff high strength reddish brown slightly silty CLAY. DEVENSIAN TILL		2.60		PP 2.50	48			
					PP 2.80	112			
					HVP 2.90	122		3.00	D
					PP 3.20	185			
					HVP 3.30	119			
					PP 3.60	158			
					PP 3.90	115			
	BOREHOLE TERMINATED AT 4.00m		4.00						

**Notes:**

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Driven tube sampler borehole record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Date of excavation (range if applicable)**

16/02/2016


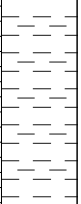
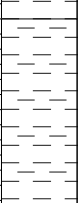
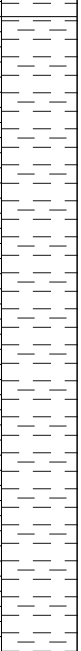
**Appendix**

D

**Location plan on drawing number**


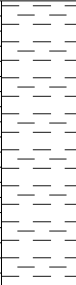


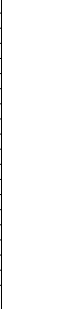
02b

**DTS103**

WELL	DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
					TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
	Grass onto soft dark brown sandy organic CLAY with occasional gravels of fine sandstone and frequent rootlets. TOPSOIL		0.30				0.20		D
	Firm medium becoming high strength orangish brown silty CLAY. DEVENSIAN TILL				PP 0.40	45			
					HVP 0.80	87	0.70		D
					PP 0.80	92			
	Stiff high strength brown mottled bluish grey occasionally sandy friable CLAY. DEVENSIAN TILL		1.10		PP 1.20	75			
					HVP 1.50	213	1.40		D
					PP 1.60	137			
	Very stiff very high strength becoming medium strength at 4.0m depth reddish brown slightly silty CLAY. DEVENSIAN TILL		1.80		PP 2.00	200			
					PP 2.40	197			
					PP 2.80	192			
					HVP 2.90	106	3.00		D
					PP 3.20	188			
					PP 3.60	143			
					PP 4.00	67			
	BOREHOLE TERMINATED AT 4.00m		4.00						

**Notes:**

<b>Ground level (mAOD)</b>	<b>Co-ordinates</b>	<b>Title</b> Driven tube sampler borehole record	<b>Surface breaking</b> No
<b>Groundwater observations</b> No groundwater encountered.		<b>Date of excavation (range if applicable)</b> 16/02/2016	<b>Appendix</b> D
		<b>Location plan on drawing number</b> 02b	<b>DTS104</b>

WELL	DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
					TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
	Grass onto soft dark brown slightly sandy organic CLAY with frequent rootlets. TOPSOIL		0.25		PP 0.30	83	0.15		D
	Firm medium and high strength orangish brown slightly silty slightly sandy CLAY. DEVENSIAN TILL				PP 0.70	63	0.70		D
					PP 1.10	77			
	Stiff very high becoming high strength brown mottled bluish grey slightly sandy CLAY. DEVENSIAN TILL		1.20		PP 1.40	192	1.40		D
					PP 1.80	220			
					PP 2.20	177			
					PP 2.60	157			
					PP 3.00	93	3.00		D
	Stiff high strength reddish brown slightly silty CLAY with occasional gravels of coarse sandstone. DEVENSIAN TILL		3.20		PP 3.40	107			
					PP 3.80	77			
	BOREHOLE TERMINATED AT 4.00m		4.00						

**Notes:**

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Driven tube sampler borehole record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Date of excavation (range if applicable)**

16/02/2016

**Appendix**


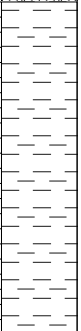
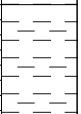
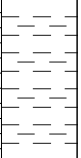
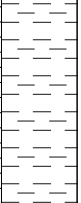
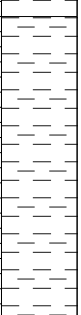
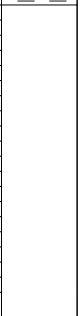
D

**Location plan on drawing number**

02b

**DTS105**



WELL	DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
					TYPE/DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
	Grass onto soft dark brown slightly sandy organic CLAY with frequent rootlets. TOPSOIL		0.30				0.10		D
	Firm to stiff medium and high strength orangish brown mottled dark brown slightly silty CLAY. DEVENSIAN TILL				PP 0.40	80			
					HVP 0.60	101			
					HVP 0.80	213	0.80		D
					PP 0.80	60			
					PP 1.20	70			
	Stiff very high strength brown mottled bluish grey CLAY. DEVENSIAN TILL		1.40		PP 1.60	180			
	<i>...from 1.8m depth, becoming very stiff.</i>				PP 2.00	217	1.80		D
	<i>...from 2.3m depth, becoming slightly gravelly with medium to coarse gravels of sandstone.</i>				PP 2.40	200			
					PP 2.80	170			
	Stiff high and very high strength reddish brown slightly silty CLAY. DEVENSIAN TILL		3.00		PP 3.20	138	3.20		D
					HVP 3.50	149			
					PP 3.60	170			
	BOREHOLE TERMINATED AT 4.00m		4.00		PP 4.00	123			

**Notes:** For Dynamic Cone Penetration testing, refer to DCP103.

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Driven tube sampler borehole record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Date of excavation (range if applicable)**

16/02/2016



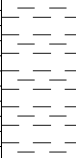


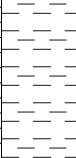
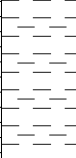

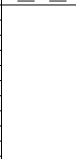
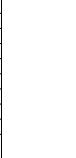

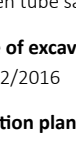
**Appendix**

D

**Location plan on drawing number**

02b

**DTS106**

WELL	DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
					TYPE/DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
	Grass onto soft dark brown sandy organic CLAY with frequent rootlets. TOPSOIL		0.25		PP 0.30	42	0.20		D
	Soft medium strength orangish brown silty CLAY. DEVENSIAN TILL				PP 0.70	42	0.70		D
	...from 0.8m depth, becoming firm.				PP 1.00	85			
					PP 1.20	47			
	Stiff very high strength brown friable CLAY. DEVENSIAN TILL		1.40						
					PP 2.00	198	1.70		D
					PP 2.40	212	2.50		D
					PP 2.80	192			
	...from 3m depth, becoming stiff.				PP 3.20	175			
					PP 3.60	100			
	Firm medium strength brown CLAY. DEVENSIAN TILL		3.80				3.90		D
	...from 3.8m depth, becoming firm.		4.00		PP 4.00	75			
	BOREHOLE TERMINATED AT 4.00m								

**Notes:**

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Driven tube sampler borehole record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Date of excavation (range if applicable)**

16/02/2016


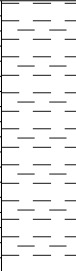
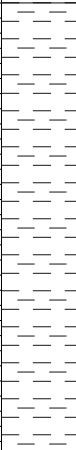
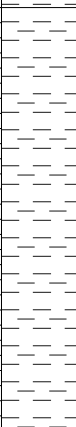
**Appendix**

D

**Location plan on drawing number**

02b

**DTS107**

WELL	DESCRIPTION	LEGEND	DEPTH (m)	WATER STRIKE	TEST RESULTS		SAMPLING		
					TYPE/ DEPTH (m)	RESULT	FROM (m)	TO (m)	TYPE
	Grass onto soft dark brown sandy organic CLAY with frequent rootlets. TOPSOIL		0.20				0.10		D
	Firm low becoming medium strength orangish brown slightly silty CLAY. DEVENSIAN TILL				PP 0.30	37			
					PP 0.70	63	0.70		D
					PP 1.00	138			
	Stiff medium and high strength brown mottled grey slightly gravelly friable CLAY. Gravel consists of medium mudstone. DEVENSIAN TILL		1.10		PP 1.30	120			
					PP 1.70	72	1.70		D
					PP 2.10	107			
					PP 2.50	97			
	Firm medium strength reddish brown slightly silty CLAY. DEVENSIAN TILL		2.60		PP 2.80	55			
							3.00		D
					PP 3.20	38			
					PP 3.60	47			
					PP 4.00	58			
	BOREHOLE TERMINATED AT 4.00m		4.00						

**Notes:**

**Ground level (mAOD)**

**Co-ordinates**

**Title**

Driven tube sampler borehole record

**Surface breaking**

No

**Groundwater observations**

No groundwater encountered.

**Date of excavation (range if applicable)**

16/02/2016

**Appendix**

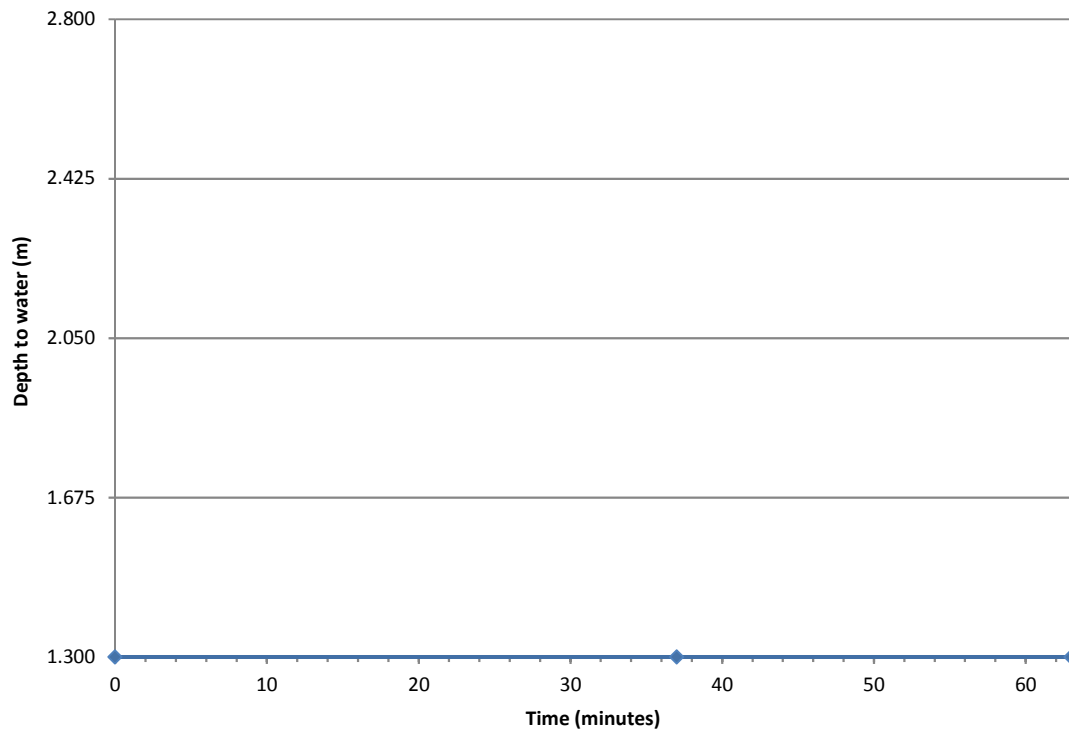
D

**Location plan on drawing number**

02b

**DTS108**

### Plot showing time against depth to water:



### Test observations:

TIME (mins)	DEPTH TO WATER (m)	TIME (mins)	DEPTH TO WATER (m)
0	1.3		
37	1.3		
63	1.3		

### Calculations:

No movement in water level over 63 minutes of monitoring therefore unable to calculate soil infiltration rate.

#### Groundwater observations

No groundwater encountered.

#### Trial pit dimensions (width x length)

0.6m x 2.5m

#### Depth of trial pit at start of test (m)

2.8

#### Ground level

N/A

#### Co-ordinates

-

#### Title

Soil infiltration test (following principles of the Building Research Establishment Digest 365 2007)

#### Location plan on drawing number

02

#### Trial pit number

TP107

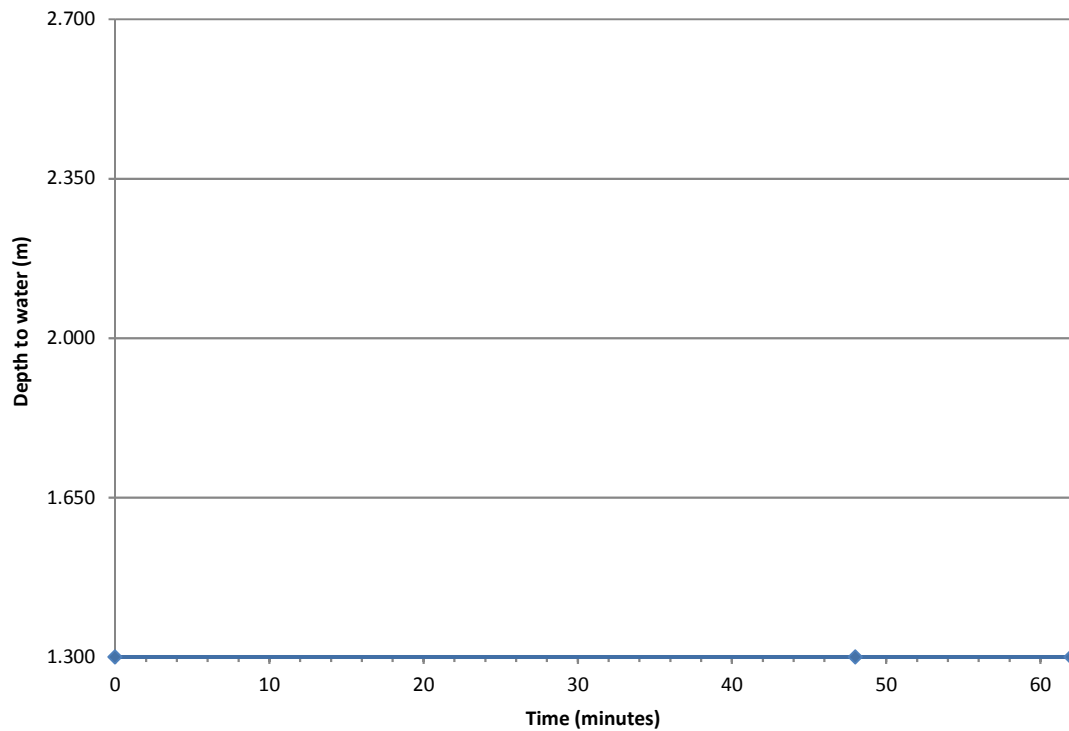
#### Cycle number

1

#### Date of excavation

17/02/2016

### Plot showing time against depth to water:



### Test observations:

TIME (mins)	DEPTH TO WATER (m)	TIME (mins)	DEPTH TO WATER (m)
0	1.3		
48	1.3		
62	1.3		

### Calculations:

No movement in water level over 62 minutes of monitoring therefore unable to calculate soil infiltration rate.

#### Groundwater observations

No groundwater encountered.

#### Trial pit dimensions (width x length)

0.6m x 3.2m

#### Depth of trial pit at start of test (m)

2.7

#### Ground level

N/A

#### Co-ordinates

-

#### Title

Soil infiltration test (following principles of the Building Research Establishment Digest 365 2007)

#### Location plan on drawing number

02

#### Trial pit number

TP114

#### Cycle number

1

#### Date of excavation

17/02/2016